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UPDATED DRAFT

**PAVEMENT ENGINEERING FOR PLANNING REPORT
AREA OF CONTINUED ANALYSIS
DETROIT RIVER INTERNATIONAL CROSSING
WINDSOR, ONTARIO**

Submitted to:

URS Canada Inc.
75 Commerce Valley Drive East
Markham, Ontario
L3T 7N9

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March 14, 2008

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March 14, 2008

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URS Canada Inc.
75 Commerce Valley Drive East
Markham, Ontario
L3T 7N9

Attention: Mr. M. Thompson, P. Eng.

**RE: PAVEMENT ENGINEERING FOR PLANNING REPORT
AREA OF CONTINUED ANALYSIS
DETROIT RIVER INTERNATIONAL CROSSING
WINDSOR, ONTARIO**

Dear Sirs:

This updated draft report presents the results of the review of the readily available geotechnical information for the pavement engineering component of the Detroit River International Crossing project together with the results of the analyses of the traffic data provided and planning level pavement design recommendations. The location of the Area of Continued Analysis for the project is shown on the Location Plan, Figure 1.

The purpose of this report is to provide pavement engineering input in support of the Detroit River International Crossing transportation corridor study. A group of consulting firms, lead by URS Canada Inc., is completing the work on behalf of the Ontario Ministry of Transportation (MTO). The MTO is one of the four principal members of the international partnership organized to study potential transportation corridors to permit more efficient transit of travelers and goods between the highway systems in the Windsor and Detroit areas.



We trust that this updated draft report provides all of the information that you presently require. If you require any additional information, or when you wish us to finalize this report, please contact this office.

Yours truly,

GOLDER ASSOCIATES LTD.

Michael E. Beadle, P. Eng.

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1.0 GENERAL DATA

1.1 Project Description

Current and projected cross-border traffic conditions in the greater Detroit and Windsor areas are such that a new crossing of the Detroit River is required. The pavement component of the project will generally consist of the upgrading of existing transportation routes and/or the construction of new roadways to connect Highway 401 in Ontario with the interstate highway system surrounding Detroit. The connection of the highway systems will require a new bridge crossing of the Detroit River and has the potential to include multiple overpass or underpass bridge structures and associated interchanges and/or grade separations along the connecting routes. Consideration may also be given to depressed pavement sections constructed between retaining structures to achieve grade separations. The Area of Continued Analysis (ACA) for the project is shown on the Key Plan, Figure 1.

At this time, six preliminary alternative transportation corridor configurations for the Ontario portion of the project have been established and are summarized as:

- Alternative 1a – one way service roads on either side of six lane freeway, at grade;
- Alternative 1b – one way service roads on either side of six lane freeway, below grade;
- Alternative 2a – six lane freeway at grade adjacent to Huron Church Road/Highway 3;
- Alternative 2b – six lane freeway below grade adjacent to Huron Church Road/Highway 3;
- Alternative 3 – cut and cover tunnel below reconstructed Huron Church Road/Highway 3 corridor; and
- Parkway Alternative - six lane freeway generally below grade between Highway 401 and Huron Church Road/E.C. Row Expressway interchange and at or above grade between Huron Church Road/E.C. Row Expressway interchange and Ojibway Parkway including 1.86 kilometres of short tunnels.

These alternatives are briefly discussed below:

1.1.1 Alternative 1A

Alternative 1A includes an at-grade freeway with below grade sections and one-way service roads on each side of the freeway. Interchanges are proposed at Highway 3, St. Clair College and Huron Church Road.

1.1.2 Alternative 1B

Alternative 1B includes a below grade freeway with one-way service roads on each side of the freeway. Interchanges are proposed at Highway 3, St. Clair College and Huron Church Road.

1.1.3 Alternative 2A

Alternative 2A includes an at-grade freeway with below grade sections which is predominantly aligned west of the existing Highway 3 and Huron Church Road. This section of Highway 3 and Huron Church Road would function as a service road to the freeway. Interchanges are proposed at Highway 3, Howard Avenue, Todd Lane/Cabana Road and Huron Church Road.

1.1.4 Alternative 2B

Alternative 2B includes a below grade freeway which is predominantly aligned west of the existing Highway 3 and Huron Church Road. This section of Highway 3 and Huron Church Road would function as a service road to the freeway. Interchanges are proposed at Highway 3, Howard Avenue, Todd Lane/Cabana Road and Huron Church Road.

1.1.5 Alternative 3

Alternative 3 is a cut and cover tunneled freeway with service roads (Highway 3 and Huron Church Road) constructed on top of the tunnel box. Interchanges are proposed at Highway 3, St. Clair College and Huron Church Road.

1.1.6 Parkway Alternative

The Parkway includes a below grade freeway with 1.86 kilometres of short tunnels. The freeway is predominantly aligned south of the existing Highway 3 and Huron Church Road. Existing Highway 3 and Huron Church Road would function as a service road to the freeway. Interchange ramps are proposed at Highway 3, Howard Avenue, St. Clair College, Todd Lane/Cabana Road and Huron Church Road.

1.2 Background Information

During the initial phases of the planning study, Golder Associates Ltd. (Golder Associates) compiled, reviewed and summarized the background geotechnical information available for the whole study area and the results were summarized in Golder Associates Draft "Interim Report on Pavements Engineering, Detroit River International Crossing, Windsor, Ontario" dated August 2005 which is attached in Appendix A.

The relevant information has been included in this report together with the pertinent information from Golder Associates Report No. 04-1111-060 entitled "Preliminary Foundation Investigation and Design Report, Detroit River International Crossing, Bridge Approach Corridor" Geocres No. 40J6-18 and dated June 2007.

1.3 Scope of Work

The scope of work to complete the Pavement Engineering Planning Report consists of:

- Concentrate the remainder of the Field Review in the Area of Continued Analysis (ACA).
- Assemble and compile borehole data, including information from the ongoing foundation investigations for the ACA.
- Analyze the traffic data provided by URS Canada Inc.
- Review the concept pavement designs, both rigid and flexible, provided by MTO in the Request for Proposal (RFP) for the project and suggest modifications, as appropriate. Emphasis has been placed on the Highway 401 extension with consideration also being given to the major City of Windsor streets.
- Carry out Life Cycle Costing of the pavement options.
- Prepare a Pavement Engineering Planning Report and submit in draft format for review and input.
- Finalize the report following receipt of comments from URS and MTO.

2.0 GEOTECHNICAL ENGINEERING CONDITIONS

2.1 Subsurface Data Review

Existing subsurface data was compiled and used to develop the soil classification maps attached in Appendix A for the whole study area. The information presented was gathered from Ministry of Transportation Ontario files (through the GEOCREs system), Golder Associates project files, Ontario Ministry of Natural Resources (MNR), and published papers and texts. These data were used to assist in constructing an electronic database of subsurface information and to map the soil classification throughout the Windsor area at the following approximate intervals of depth: 0.3, 1.5, 3.0, and 6.0 metres below existing ground surface at the borehole locations.

Subsurface data existing in Golder Associates and the Ministry of Transportation Ontario (MTO) GEOCREs library files for projects completed in Windsor and surrounding areas over the last 40 years were reviewed for this project using the following criteria:

- Available boreholes and test pits drilled or excavated from ground surface to a minimum depth of 3 metres, and
- One representative borehole or test pit per minimum area of 0.25 square kilometres, where available, within the City of Windsor and Towns of LaSalle and Tecumseh.

For the purposes of this report, the database has been adjusted to reflect the conditions within the Area of Continued Analysis. The database for the Area of Continued Analysis is provided in Appendix A.

2.2 Geology of the Windsor Area

The subsurface conditions in the Windsor area are characterized by regionally extensive, flat-lying soils including:

- Surface layers of miscellaneous fill materials in localized areas, generally associated with industrial and urban growth;
- Native deposits of sand and silt present at or near the surface in some locations, particularly in the west end of the City of Windsor and the Town of LaSalle;
- Beneath the sand, where present, extensive deposits of clayey silt to silty clay; and
- Bedrock throughout the study area is generally encountered at depths of 20 to 35 metres.

Figure 2 illustrates the Quaternary geology and outlines the general surficial sedimentary geology of the study area based on geologic interpretation of widely-spaced sample locations and an understanding of geomorphologic processes. It should be noted that only the natural sediments encountered closest to the surface are identified, and that other sediments may underlie these layers.

2.3 Sedimentary Geology

The study area is located in the physiographic region of Southwestern Ontario known as the St. Clair Clay Plains¹. Within this region, Essex County and the southwestern part of Kent County are normally discussed as a subregion known as the Essex Clay Plain. The clay plain was deposited during the retreat of the ice sheets during the late Pleistocene Era when a series of glacial lakes inundated the area. In general, the ice sheets deposited till in the area of Windsor and Detroit. Depending on the locations of the glacial ice sheets and depths of water in the ice-contact glacial lakes, the till may have been directly deposited at the contact between the ice sheet and the bedrock or, as the lake levels rose and the ice sheets retreated and floated, the soil and rock debris within and at the base of the ice may have been deposited through the lake water as lacustrine deposits. Glacial till, in its common usage, often indicates a very dense or hard condition resulting from consolidation and densification under the weight of the ice sheet. The mineral soil particles typically have a distribution of grain sizes ranging from cobbles to clay. However, in many areas of Windsor and Detroit, the soils described as “glacial till” were deposited through water and, as a result, have a softer consistency. A large end moraine of glacial till is mapped in the area of Windsor-Detroit, generally trending northwest to southeast near the outlet of Lake St. Clair as illustrated throughout Essex County near the terminus of Highway 401. In other areas, the lacustrine deposits overlie the hard glacial till. The major clay stratum, typically ranging in thickness from about 20 metres to 30 metres, exhibits a till-like structure exemplified by a random distribution of coarser particles within the primarily fine-grained silt and clay deposit.

Surficial layers or pockets of more typical layered lacustrine silty clay, silt or sand may be encountered overlying the extensive stratum of “till-like” silty clay. Silt and sand deposits, on the order of 2 to 4 metres thick, are often found near the ground surface in areas near the western side of Windsor and the southwestern limits of the Area of Continued Analysis.

¹ Chapman, L. J. and Putnam, D.F., The Physiography of Southern Ontario, 1984.

2.4 General Subsurface Conditions

The following sections describe the general subsurface conditions in the Area of Continued Analysis (ACA) at intervals of depth of approximately 0.3, 1.5, 3.0 and 6.0 metres below existing ground surface. The information was gathered from previous geotechnical investigations carried out by Golder Associates and others using the sources outlined in Section 2.1. The following descriptions of the subsurface conditions are generalized and should not be interpreted to be exact, nor used in the detail design of the pavement structure of the proposed corridor. Further detailed, site specific geotechnical investigation is required once the preferred corridor has been selected.

The subsurface database is provided in Appendix B and the locations of the previous boreholes summarized in the database are shown on Figure 3.

Beneath the existing pavement structure(s), topsoil and/or surficial fill materials, granular materials consisting of sand and gravel, sands and silty sands were identified at a depth of approximately 0.3 metres below existing ground surface in the E.C. Row Expressway/Ojibway Parkway area. More localized areas of sands and silty sands were also identified, predominantly adjacent to the E.C. Row Expressway between Ojibway Parkway and Huron Church Road. Elsewhere in the ACA, the subsurface conditions at approximately 0.3 metres below ground surface generally consist of silty clay to clayey silt with localized areas of surficial granular soils.

At approximately 1.5 metres depth, sands and sandy silts were identified in the E.C. Row Expressway/Ojibway Parkway area. Elsewhere within the ACA, the subsurface conditions consist of clayey silt to silty clay.

At approximately three metres depth, the subsurface conditions generally consist of clayey silt and/or silty clay in the ACA. Figure 5 illustrates the subsurface soil conditions at approximately three metres below existing ground surface in the ACA.

At approximately six metres depth, the subsurface conditions generally consist of clayey silt and/or silty clay throughout the ACA.

Groundwater levels are expected to be located about 2 to 3 metres below ground surface in the clayey silt and silty clay materials and will generally be defined by the location of the transition of the soil colour from brown to grey. It should be noted that one of the boreholes drilled for the foundations engineering component of the project encountered artesian groundwater pressures in the clayey silt. This borehole was drilled near the intersection of Ojibway Parkway and E.C. Row Expressway in the northwestern portion of the project.

2.5 Soil Characteristics Influencing Pavement Design

The subsurface conditions as described in this report have an important influence on the design of the pavement structures for the proposed transportation corridor. Two main factors affecting roadway pavement design are:

- frost susceptibility of the subgrade soil, which influences the design of the pavement structure component thicknesses; and
- soil erodibility, which influences the design of embankment surfacing and drainage ditches that may be proposed in the design of a proposed corridor.

2.6 Frost Susceptibility

The generalized subsurface conditions have been categorized as low, moderate or high frost susceptibility. At depths of about 1.2 to 1.5 metres, the subsurface conditions in the E.C. Row Expressway/Ojibway Parkway area indicate the presence of localized area of sands and silty sands, which are considered to have low to moderate frost susceptibility. This area also has areas of silty clay and clayey silt which are considered to have a low susceptibility to frost action. These are also the soils that occur at depth.

2.7 Soil Erodibility

It is anticipated that drainage ditches and embankments will be incorporated in the design of the proposed corridor. The subsurface conditions in the Area of Continued Analysis consist largely of clayey silt and silty clay with localized surficial areas of sand and gravel, sands and silty sand. The silty clay, clayey silt, sand and gravel and sands are considered to be slightly erodible and the silty sands are considered to be moderately erodible.

3.0 REVIEW OF AVAILABLE BACKGROUND INFORMATION

Following the review of the available geological mapping and information from our files to delineate, in a preliminary fashion, the general subsurface conditions in the area of the site, a detailed review of the files in our Windsor office was carried out to attempt to determine the characteristics of the existing pavements in the ACA. In addition, requests were made to the City of Windsor by URS and to the MTO by Golder Associates for any available background information regarding the existing pavement conditions in the study area. To date, no information has been provided by the City of Windsor.

3.1 Golder Associates Report 04-1111-060

Four deep boreholes, numbered 1, 7, 14 and 23, were advanced in conjunction with the foundation investigation portion of this project, and their locations are shown on the Location Plan, Figure 1. The following paragraphs summarize the subsurface conditions encountered in the upper portion of these boreholes as they relate to pavement design. The Records of Boreholes are provided in Appendix C.

3.1.1 Borehole Information

Borehole 1 – Highway 3 and Howard Avenue Area

About 0.3 metres of clayey topsoil was encountered at ground surface. The surficial topsoil had a natural moisture content of about 20 per cent.

Beneath the topsoil, a 3.6 metre thick layer of firm to very stiff silty clay was encountered. A single standard penetration test completed in the silty clay indicated an N value of 28 blows per 0.3 metres. The moisture content of the silty clay was about 15 to 18 per cent.

An extensive deposit of stiff clayey silt was encountered beneath the silty clay. The upper portion of the clayey silt had N values of 9 to 15 blows per 0.3 metres with moisture contents of about 16 to 18 per cent. In situ vane shear strength testing carried out in the upper portion of the clayey silt indicated undrained shear strengths of about 75 to 80 kilopascals (kPa) with vane sensitivities of 1.0 to 1.5. Below about 10 metres depth, the undrained shear strength was about 55 kPa with a vane sensitivity of 1.7. The clayey silt had corresponding average plastic and liquid limits of 13 and 25 per cent, respectively, based on two Atterberg limits determinations. The Atterberg limits data are shown on the Plasticity Chart, Figure 4.

Borehole 7 – Howard Avenue and Geraedts Drive Area

About 0.3 metres of clayey topsoil was encountered at ground surface. The surficial topsoil had an N value of 10 blows per 0.3 metres based on a standard penetration test partially completed in the topsoil with a natural moisture content of about 20 per cent.

Beneath the topsoil, a 3.6 metre thick layer of very stiff to hard clayey silt was encountered. The clayey silt had N values of 23 to 59 blows per 0.3 metres with moisture contents of about 13 per cent.

An extensive deposit of very stiff to firm clayey silt was encountered beneath the upper very stiff to hard clayey silt. The upper portion of the clayey silt had N values of 11 to 32 blows per 0.3 metres with moisture contents of about 14 to 19 per cent. In situ vane shear strength testing carried out in the upper portion of the clayey silt indicated undrained shear strengths of greater than 100 kPa above about 7 metres depth with a vane sensitivity of 1.3. Below about 7 metres depth, the undrained shear strength was about 55 to 70 kPa with vane sensitivities of 1.7 to 2.0. The clayey silt had corresponding average plastic and liquid limits of 12 and 25 per cent, respectively, based on three Atterberg limits determinations. The Atterberg limits data are shown on the Plasticity Chart, Figure 4.

Borehole 14 – Huron Church Road and Balmoral Drive Area

About 1.1 metres of silty clay fill was encountered at ground surface in borehole 14. The fill had an N value of 13 blows per 0.3 metres with an in situ moisture content of about 16 per cent. Beneath the silty clay fill, a 0.3 metre thick layer of silty sand and gravel fill was encountered.

A soft to very stiff layer of clayey silt about 4.1 metres thick was encountered beneath the fill. Silty clay layers were noted within the clayey silt. The clayey silt had N values of 4 to 14 blows per 0.3 metres with moisture contents of about 22 to 26 per cent. In situ vane testing in the clayey silt indicated undrained shear strengths of 115 to 120 kPa with vane sensitivities of 1.4 to 2.7. The clayey silt had corresponding plastic and liquid limits of 21 and 41 per cent, respectively, based on a single Atterberg limits determination. The Atterberg limits data are provided on Figure 4.

Beneath the upper soft to very stiff clayey silt, an extensive deposit of firm to stiff clayey silt was encountered at 5.5 metres depth or at about elevation 176.6 metres. The upper portion of the clayey silt had an N value of 7 blows per 0.3 metres with moisture contents of about 20 to 23 per cent. In situ vane shear strength testing carried out in the upper portion of the clayey silt indicated undrained shear strengths of 58 to 77 kPa with vane sensitivities of 1.7 to 3.2. The clayey silt had corresponding average plastic and liquid

limits of 15 and 31 per cent, respectively, based on two Atterberg limits determinations. The Atterberg limits data are shown on the Plasticity Chart, Figure 4.

Borehole 23 – E.C. Row Expressway and Ojibway Parkway Area

About 0.3 metres of silty sand fill was encountered at ground surface in borehole 23. Beneath the fill, about 0.3 metres of sandy topsoil and 1.1 metres of sandy silt was encountered.

A stiff to very stiff layer of clayey silt about 2.7 metres thick was encountered beneath the sandy silt. The clayey silt had N values of 6 to 15 blows per 0.3 metres with moisture contents of about 21 to 24 per cent. In situ vane testing in the clayey silt indicated undrained shear strengths of 105 to 120 kPa with vane sensitivities of 2.8 to 2.9. The clayey silt had corresponding plastic and liquid limits of 17 and 28 per cent, respectively, based on a single Atterberg limits determination. The Atterberg limits data are provided on Figure 4.

Beneath the upper stiff to very stiff clayey silt, an extensive deposit of soft to very stiff clayey silt was encountered at about 4.4 metres depth or about elevation 174.5 metres. This layer extended to about 20.7 metres depth or about elevation 158.2 metres. The upper portion of the clayey silt had N values of 1 to 5 blows per 0.3 metres with moisture contents of about 23 to 36 per cent. In situ vane shear strength testing carried out in the upper portion of the clayey silt indicated undrained shear strengths of 25 to 43 kPa with vane sensitivities of 1.8 to 3.5. The clayey silt had corresponding average plastic and liquid limits of 15 and 27 per cent, respectively, based on two Atterberg limits determinations. The Atterberg limits data are shown on the Plasticity Chart, Figure 4.

3.1.2 Field Vane and Piezocone Penetration Testing (CPT) Information

In addition to the above, rigorous in situ vane testing was carried out adjacent to each borehole and piezocone penetration testing (CPT) was carried out at regular intervals along the alignment. The locations of the CPTs and field vane testing are shown on the Plan, Figure 1. Summaries of the undrained shear strength measurements carried out during the investigation using field vane and CPT methods are provided on Figures 5, 6 and 7.

3.2 Ministry of Transportation Background Information

The MTO provided the available background information for sections of roadway under their jurisdiction in the form of Region Geotechnical Pavement Condition Reports. The relevant information from these are summarized in the following sections.

Highway 401 Eastbound Lanes - Walker Road to the terminus of Highway 401 (surveyed May 2005 and June 2007)

- Original grading carried out under Contract 55-138.
- Granular base and paving carried out under Contract 55-23 and consisted of 225 millimetres of plain jointed concrete pavement (JPCP) over 100 millimetres of Granular A and 125 millimetres of Granular C.
- Contract 73-172 consisted of an asphalt overlay consisting of 50 millimetres HL 4 modified, 40 millimetres HL 4 binder course and 40 millimetres of HL 1 surface course.
- Under Contract 90-41, four sections of pavement (total length of 700 metres) were resurfaced by milling 40 millimetres and paving 40 millimetres of HL 1 (reclaimed) in the driving lane. Prior to paving, all depressed transverse cracks were sawcut and reinstated with HL 4.
- Under Contract 2002-3002, the pavements were resurfaced between Station 9+995 and 14+000 by milling 50 millimetres and paving 50 millimetres of SuperPave 12.5 FC2.
- In 2004, microsurfacing was carried out.
- 2005 PCR = 95, 2007 PCR = 90

Highway 401 Westbound Lanes - Walker Road to the terminus of Highway 401 (surveyed May 2005 and June 2007)

- Original grading carried out under Contract 55-138.
- Granular base and paving carried out under Contract 55-23 and consisted of 225 millimetres of plain jointed concrete pavement (JPCP) over 100 millimetres of Granular A and 125 millimetres of Granular C.
- Contract 73-172 consisted of an asphalt overlay consisting of 50 millimetres HL 4 modified, 40 millimetres HL 4 binder course and 40 millimetres of HL 1 surface course.
- Under Contract 90-41, a 200 metre long section was resurfaced by milling 40 millimetres and paving 40 millimetres of HL 1 (reclaimed) in the driving lane. Prior to paving, all depressed transverse cracks were sawcut, the asphalt removed and the areas reinstated with HL 4.
- Under Contract 2002-3002, 5 kilometres of the pavements were resurfaced by milling 50 millimetres and paving 50 millimetres of SuperPave 12.5 FC2.

- In 2004, the western 0.6 kilometres was resurfaced with SMA and the remainder was microsurfaced.
- 2005 PCR = 93, 2007 PCR = 88

Highway 3 – West junction of Essex Road 34 to Highway 401 (surveyed September 2006 and May 2007)

- Existing pavement consisted of asphalt and/or concrete placed directly on subgrade. No details of the original construction were provided.
- The pavements were widened and resurfaced on the south side under Contract 76-115. The widening consisted of 150 millimetres of Granular A, 237 millimetres of HL 4 binder course and 40 millimetres of HL 1 surface course. The resurfacing consisted of HL 4 padding, HL 4 binder course and HL 1 surface course.
- The westerly 0.5 kilometres was resurfaced with 50 millimetres of DFC under Contract 98-19.
- Milling of 80 millimetres and paving 40 millimetres of HDBC and 40 millimetres of HL 1 was carried out at the Walker Road intersection in 1998 to address rutting.
- Dynapatching was carried out in 2002.
- Selective full depth repairs were carried out in the westbound lanes in 2004.
- Under Contract 2004-3009, the pavements were resurfaced by milling 50 millimetres and paving 60 millimetres of SuperPave 19.0 and 40 millimetres of SuperPave 12.5 FC2.
- Under Contract 2004-3039, intersection improvements were carried out at Outer Drive and Walker Road and the main line pavements were resurfaced by milling 50 millimetres and paving 60 millimetres of SuperPave 19.0 and 40 millimetres of SMA.
- 2006 PCR = 93, 2007 PCR = 89

Highway 3 (Huron Church Road) Eastbound Lanes – Highway 401 to Todd Lane (surveyed September 2006 and July 2007)

- No details of the original construction were provided.
- Resurfacing was carried out consisting of HL 5 padding to correct crossfall, 20 millimetres HL 2 padding and 40 millimetres of surface course.
- Widening consisted of 312 millimetres of Granular A in the shoulders, 350 millimetres of HL 5 and 40 millimetres of surface course.

- Under Contract 90-14, the westerly 0.7 kilometres was widened and resurfaced with 40 millimetres of HDDB and 40 millimetres of DFC.
- Under Contract 98-19, all but the westerly 0.7 kilometres was resurfaced with 50 millimetres of HDDB and 40 millimetres of DFC.
- Whitetopping carried out at Cousineau Road and Todd Lane under District Contracts 2002-3383 (driving lane) and 2002-3399 (passing lane).
- Under Contract 2003-3022, the pavements were resurfaced by milling 50 millimetres and paving 50 millimetres of SMA.
- 2006 PCR = 92, 2007 PCR = 80

***Highway 3 (Huron Church Road) Westbound Lanes – Highway 401 to Todd Lane
(surveyed September 2006 and July 2007)***

- No details of the original construction were provided.
- Resurfacing was carried out consisting of HL 5 padding to correct crossfall, 20 millimetres HL 2 padding and 40 millimetres of surface course.
- Widening consisted of 312 millimetres of Granular A in the shoulders, 350 millimetres of HL 5 and 40 millimetres of surface course.
- Under Contract 90-14, the westerly 0.7 kilometres was widened and resurfaced with 40 millimetres of HDDB and 40 millimetres of DFC. Widening plugs consisted of 200 millimetres of hot mix asphalt over 700 millimetres of Granular A.
- Under Contract 98-19, all but the westerly 0.7 kilometres was resurfaced with 50 millimetres of HDDB and 40 millimetres of DFC.
- Whitetopping carried out at Howard Avenue intersection District Contracts 32-99-05 to address rutting.
- Whitetopping carried out at Cousineau Road and Todd Lane under District Contracts 2002-3383 (driving lane) and 2002-3399 (passing lane).
- Under Contract 2003-3022, the pavements were resurfaced by milling 50 millimetres and paving 50 millimetres of SMA.
- Whitetopping repaired at Howard Avenue under District Contract 2003-3355.
- 2006 PCR = 93, 2007 PCR = 82

E.C. Row Expressway - Eastbound Lanes – Highway 3 (Huron Church Road) to Ojibway Parkway (surveyed September 1992)

- Pavement constructed under Contract 89-41 and consisted of 25 millimetres of OFC and 35 millimetres of HDDB over 225 millimetres of unreinforced concrete placed on 110 millimetres of OGD. This structure includes the partially paved shoulders.
- 0.7 kilometre long structure approach at Highway 3 constructed under Contract 87-23 and consisted of 65 millimetres of hot mix asphalt over 230 millimetres of plain concrete and asphalt stabilized OGD.
- 1992 PCR = 59

E.C. Row Expressway - Westbound Lanes – Highway 3 (Huron Church Road) to Ojibway Parkway (surveyed September 1992)

- Original pavement consisted of 40 millimetres of DFC over 200 millimetres of unreinforced concrete placed on 150 millimetres of cement treated Granular A.
- Structure approach at Highway 3 constructed under Contract 87-23 and consisted of 65 millimetres of hot mix asphalt over 230 millimetres of plain concrete and asphalt stabilized OGD.
- Rout and seal carried out between Matchette Road and Huron Church Line.
- Structure approaches patched full width at Malden Road and Matchette Road.
- Several slabs replaced under Contract 87-41.
- French drains installed in outer shoulder.
- 1992 PCR = 57

3.3 Review of Background Information From Golder Associates' Files

To supplement the background information for the pavements at the site, a review of the information in Golder Associates' files for the area of the site was carried out. The relevant information from that review is summarized in the following sections. The review included a summary of the recommended pavement design information for the various sections of roadway; however, these should not necessarily be construed to be the actual pavements constructed. Further, post report and construction modifications may have also been made.

No pavement information was provided by the City of Windsor.

Huron Church Road at Todd Lane and Todd Lane

Reference: Golder Associates Report No. 754139/13 entitled “Geotechnical Investigation, Contract No. 10, Proposed Provincial Sewage Works Program, Township of Sandwich West, County of Essex, Ontario” dated September 21, 1982.

The boreholes drilled for this investigation generally encountered silty clay to clayey silt till materials at subgrade level.

The recommendations provided in the report indicated that the pavements impacted by sewer construction could be restored using a pavement structure consisting of 40 millimetres of surface course, 50 millimetres of binder course and 400 millimetres of Granular A base.

Cousineau Road, East of Highway 3

Reference: Golder Associates Report No. 791-4096 entitled “Geotechnical Investigation, Proposed Heritage Estates Residential Development, Township of Sandwich West, County of Essex, Ontario” dated August 1979.

This report indicated that the Cousineau Road realignment could be constructed using a flexible pavement structure or a rigid pavement. The rigid pavement could consist of 200 millimetres of concrete and 230 millimetres of Granular A base. The flexible pavement could consist of 40 millimetres of surface course, 100 millimetres of binder course and either 600 millimetres of Granular A base or 150 millimetres of Granular A and 600 millimetres of Granular B subbase.

Labelle Street, East of Huron Church Road

Reference: Golder Associates Report No. 801-4009 entitled “Geotechnical Investigation, Proposed Phase II Bellewood Land Assembly, Windsor, Ontario” dated February 1980.

Two pavement design alternatives were provided for Labelle Street as summarized below:

<u>COMPONENT</u>	<u>THICKNESS (mm)</u>	
	<u>Alternative 1</u>	<u>Alternative 2</u>
Asphalt	100	100
Granular A Base	600	150
Granular B Subbase	-	660

It is not known which alternative was ultimately constructed.

Huron Church Road, Cabana Road to Grand Marais Drain and Pulford Street

Reference: Golder Associates Report No. 901-4256 entitled “Geotechnical Investigation, Proposed Phase 4 Reconstruction, Huron Church Road, Windsor, Ontario” dated January 1991.

Recommendations were provided in the above-noted report to reconstruct the subject section of Huron Church Road using a rigid pavement structure consisting of 275 millimetres of Portland cement concrete, 100 millimetres of drainage layer (19 millimetre clear stone or asphalt stabilized OGD) and 275 millimetres of Granular A base.

The report also noted that the existing Pulford Street intersection at Huron Church Road would be widened and realigned to the north. The pavement structure recommended for the new section of Pulford Street consisted of 230 millimetres of Portland cement concrete and 300 millimetres of Granular A base.

Howard Avenue, North Talbot Road to Highway 3

Reference: Golder Associates Report No. 921-4204 entitled “Geotechnical Investigation, Proposed Howard Avenue Sanitary Sewer, North Talbot to Highway No. 3, Windsor, Ontario” dated November 1992.

The above-noted report suggested that the new Howard Avenue pavement structure for the project consist of 150 millimetres of asphalt and 610 millimetres of Granular A base. In addition, it was suggested that continuous subdrains be provided at subgrade level to provide effective drainage of the pavement structure.

E.C. Row Expressway, Ojibway Parkway to Huron Church Road

Reference: John Emery Geotechnical Engineering Limited (JEGEL) Report No. 198258 entitled “Pavement Evaluation, E.C. Row Expressway, Ojibway Parkway to Huron Church Road and Huron Church Road to Midpoint of Dominion Boulevard and Dougall Avenue” dated April 5, 1999.

The background information provided in the above-noted report indicates that the original construction of this section of the E.C. Row Expressway was carried out between 1979 and 1981 and consisted of 38 millimetres of DFC, 200 millimetres of plain jointed concrete pavement and 150 millimetres of cement treated Granular A. The construction was carried out to accommodate two lanes of traffic.

In 1987, a grade separation was constructed at the Huron Church Road intersection from 900 metres west to 700 metres east of Huron Church Road. The pavements for the grade separation consisted of 25 millimetres of OFC, 30 millimetres of HL 4 binder course, 225 millimetres of plain jointed concrete and 110 millimetres of asphalt stabilized OGDL.

In 1989, two additional lanes, creating the existing eastbound lanes were constructed as well as interchanges at Matchette Road and Malden Road. The mainline pavements consisted of 25 millimetres of OFC, 30 millimetres of HL 4 binder course, 225 millimetres of plain jointed concrete and 110 millimetres of asphalt stabilized OGDL. The speed change lanes consisted of 40 millimetres of DFC, 225 millimetres of plain jointed concrete and 150 millimetres of OGDL. The ramps consisted of 35 millimetres of DFC, 35 millimetres of HL 4 binder course, 225 millimetres of plain jointed concrete and 110 millimetres of asphalt stabilized OGDL.

In 1997, a 50 millimetre HL 1 surface course overlay was provided between Ojibway Parkway and Matchette Road.

JEGEL provided several alternative pavement designs for the various sections of pavements to be reconstructed including two alternative flexible structures and a rigid structure as summarized in Table 6 of their Report No. 198258, as presented below:

TABLE 6
MINIMUM PAVEMENT STRUCTURE REQUIREMENTS
(All thickness in millimetres)

SECTION	PAVEMENT STRUCTURE ALTERNATIVES		
	RIGID	FLEXIBLE (THIN BASE)	FLEXIBLE (THICK BASE)
MAINLINES Ojibway Parkway to East Ramps of Huron Church Road, Section I	250 PCC* 100 OGDL 200 100% Crushed Granular A	40 SMA** 120 HDDB (2 lifts) 40 HL 8 100 OGDL 100 100% Crushed Granular A 300 100% Crushed Granular B	40 SMA 110 HDDB (2 lifts) 100 OGDL 100 100% Crushed Granular A 450 100% Crushed Granular B
East Ramps of Huron Church Road to East Ramps of Dominion Boulevard, Section II West	270 PCC 100 OGDL 200 100% Crushed Granular A	40 SMA 120 HDDB (2 lifts) 60 HL 8 100 OGDL 100 100% Crushed Granular A 300 100% Crushed Granular B	40 SMA 90 HDDB (2 lifts) 40 HL 8 100 OGDL 100 100% Crushed Granular A 450 100% Crushed Granular B

SECTION	PAVEMENT STRUCTURE ALTERNATIVES		
	RIGID	FLEXIBLE (THIN BASE)	FLEXIBLE (THICK BASE)
East Ramps of Dominion Boulevard to Midpoint of Dominion Boulevard and Dougall Road, Section II East	285 PCC 100 OGDL 200 100% Crushed Granular A	40 SMA 120 HDBC (2 lifts) 80 HL 8 100 OGDL 100 100% Crushed Granular A 300 100% Crushed Granular B	40 SMA 110 HDBC (2 lifts) 40 HL 8 100 OGDL 100 100% Crushed Granular A 450 100% Crushed Granular B
RAMPS Matchette Road Interchange	225 PCC 300 100% Crushed Granular A	40 SMA 90 HDBC 60 HL 8 100 100% Crushed Granular A 300 100% Crushed Granular B	40 SMA 90 HDBC 40 HL 8 100 100% Crushed Granular A 450 100% Crushed Granular B
Huron Church Road Interchange	270 PCC 300 100% Crushed Granular A	40 SMA 120 HDBC 80 HL 8 100 100% Crushed Granular A 300 100% Crushed Granular B	40 SMA 120 HDBC 40 HL 8 100 100% Crushed Granular A 450 100% Crushed Granular B
Dominion Boulevard Interchange	260 PCC 300 100% Crushed Granular A	40 SMA 120 HDBC 80 HL 8 100 100% Crushed Granular A 300 100% Crushed Granular B	40 SMA 120 HDBC 40 HL 8 100 100% Crushed Granular A 450 100% Crushed Granular B

* Portland Cement Concrete with a compressive strength of 35 MPa at 28 days.

** Stone Mastic Asphalt with Performance Graded Asphalt Cement.

Reproduced from JEGEL Report No. 198258

4.0 PAVEMENT CONDITION SURVEY

A pavement condition survey of the major pavements within the project was carried out by a senior pavement design engineer from our staff. At that time, the existing pavements were visually examined and digital photographs of characteristic and major distresses were obtained. Select site photographs are provided in Appendix D.

A summary of the items noted during the pavement condition survey are provided in the following table:

LOCATION	REMARKS
<p><i>Highway 401</i> - North Talbot Road to Huron Church Line</p>	<ul style="list-style-type: none"> - asphalt surfaced pavements - good condition - very slight to slight single and multiple transverse cracks throughout (see Photograph 1 in Appendix D) - multiple very slight centerline joint cracks throughout - short section of outer wheel path/pavement edge failure in curve beneath Talbot Road East in westbound lanes with severe rutting (see Photograph 2 in Appendix D)
<p><i>Huron Church Line (Highway 3)</i> - Highway 401 to Huron Church Road</p>	<ul style="list-style-type: none"> - asphalt surfaced pavements - good condition - intermittent slight transverse cracks in left turn lanes - intermittent slight longitudinal cracks in left turn lanes - slight to moderate wheel path rutting at intersections - the white topping placed at the Howard Avenue, Cousineau Road and Todd Lane intersections has been replaced with asphalt.
<p><i>Huron Church Road</i> - Huron Church Line to E.C. Row Expressway</p>	<ul style="list-style-type: none"> - asphalt surfaced from Highway 3 to just west of Todd Lane - good condition - no distresses noted in asphalt section - concrete pavements west of Todd Lane - good condition - few to intermittent, apparently full depth repairs in concrete - few moderate corner cracks, intermittent slight corner cracks
<p><i>Howard Avenue</i> - south of Huron Church Line</p>	<ul style="list-style-type: none"> - asphalt surfaced - recently resurfaced - excellent condition - see Photographs 3 and 4 in Appendix D
<p>- north of Huron Church Line</p>	<ul style="list-style-type: none"> - asphalt surfaced - severe midlane cracking in eastbound lanes throughout - moderate to severe transverse cracks throughout - moderate pavement edge/inner wheel path cracks in westbound lanes throughout - see Photographs 5 and 6 in Appendix D
<p><i>Cousineau Road</i></p>	<ul style="list-style-type: none"> - asphalt surfaced - good condition - slight transverse cracks - see Photograph 7 in Appendix D

LOCATION	REMARKS
<i>Sandwich Parkway</i>	<ul style="list-style-type: none"> - asphalt surfaced - fair to good condition - slight longitudinal cracks throughout - extensive slight transverse cracks - few moderate alligatored transverse cracks - see Photographs 8 and 9 in Appendix D
<i>Cabana Road</i>	<ul style="list-style-type: none"> - asphalt surfaced - excellent condition - recently resurfaced to about 25 metres from intersection - no distresses noted - see Photographs 10 and 11 in Appendix D
<i>Todd Lane</i>	<ul style="list-style-type: none"> - asphalt surfaced - fair condition - recently resurfaced at Huron Church Road to end of radius - slight alligatored pavement edge cracking throughout - few very slight transverse cracks - see Photographs 12 and 13 in Appendix D
<i>Pulford Street</i>	<ul style="list-style-type: none"> - concrete pavement - generally in good condition - see Photographs 14 and 15 in Appendix D - moderate to severe edge and corner cracks and joint distresses about 50 metres and 20 metres south of Northway Avenue (see Photographs 16 and 17 in Appendix D)
<i>Grand Marais Road West</i>	<ul style="list-style-type: none"> - concrete radius at Huron Church Road and asphalt surfaced beyond - good condition - intermittent slight transverse cracks - see Photographs 18 and 19 in Appendix D
<i>Lambton Street</i>	<ul style="list-style-type: none"> - concrete radius at Huron Church Road and asphalt surfaced beyond - fair condition - extensive moderate to severe alligatored transverse cracks - frequent moderate pavement edge cracks - few distresses patched with cold mix asphalt - map/random cracks at maintenance holes - see Photographs 20 and 21 in Appendix D
<i>Spring Garden Road/Bethlehem Avenue</i>	<ul style="list-style-type: none"> - concrete pavement to curve and asphalt surfaced beyond - concrete pavement in good condition - see Photographs 22 and 23 in Appendix D - few spalled concrete joints (see Photograph 24 in Appendix D)
<i>Labelle Street</i>	<ul style="list-style-type: none"> - concrete radius at Huron Church Road and asphalt surfaced beyond - pavement in excellent condition - see Photograph 25 in Appendix D

LOCATION	REMARKS
<p><i>Malden Road</i> - south of E.C. Row Expressway</p> <p>- north of E.C. Row Expressway</p>	<ul style="list-style-type: none"> - asphalt surfaced - good to excellent condition - frequent slight transverse cracks - see Photograph 26 in Appendix D - asphalt surfaced - pavements in poor condition - see Photographs 27 and 28 in Appendix D - severe to very severe single and alligatored transverse cracks throughout (see Photograph 29 in Appendix D) - very severe longitudinal/meander cracks throughout (some patched with cold mix –see Photograph 30 in Appendix D) - moderate pavement edge cracks (some patched with cold mix)
<p><i>Matchette Road</i> - north of E.C. Row Expressway</p> <p>- south of E.C. Row Expressway</p>	<ul style="list-style-type: none"> - asphalt surfaced - fair condition - see Photographs 31 and 32 in Appendix D - extensive slight transverse cracks - intermittent alligatored wheel path cracks and slight rutting in northbound lanes (see Photograph 33 in Appendix D) - asphalt surfaced - fair condition - see Photographs 34 and 35 in Appendix D - intermittent slight transverse cracks - intermittent slight wheel path ruts - intermittent slight wheel path cracks - frequent moderate longitudinal cracks (see Photograph 36 in Appendix D)
<p><i>E.C. Row Expressway</i> - Huron Church Road to Ojibway Parkway</p>	<ul style="list-style-type: none"> - asphalt surfaced pavements - excellent condition - intermittent slight transverse cracks
<p><i>Ojibway Parkway North</i> - E.C. Row Expressway to Prospect Avenue</p>	<ul style="list-style-type: none"> - asphalt surfaced to just north of rail crossing - asphalt in poor condition - moderate transverse cracks throughout - moderate longitudinal cracks throughout, some alligatored - frequent moderate map cracks - concrete pavements north of rail crossing - concrete pavements in poor condition - approximately 30 per cent of transverse joints failed (blow ups) with some concrete patches and poorly performing asphalt patches (see Photographs 37 and 38 in Appendix D) - few moderate longitudinal/crescent cracks
<p><i>Ojibway Parkway South</i> - E.C. Row Expressway to Weaver Road</p>	<ul style="list-style-type: none"> - asphalt surfaced pavements - poor condition in southbound lanes from E.C. Row Expressway to Broadway Street - severe depressed transverse cracks throughout - extensive moderate longitudinal cracks - northbound lanes and southbound lanes south of Broadway Street in fair condition - slight single and multiple transverse cracks throughout, blow ups - intermittent slight meander cracks - moderate centerline joint cracks throughout

5.0 TRAFFIC VOLUMES

The traffic data provided by URS Canada Inc. for the entire project are attached in Tables I-A to I-F following the text of this report. A summary of the projected 2015 traffic data for the proposed Highway 401 mainline extension/access route to the bridge for the various alternatives is provided below:

HIGHWAY 401 EXTENSION/ ACCESS ROUTE TO BRIDGE		ALT 1a		ALT 1b		ALT 2a		ALT 2b		ALT 3		PKWAY ALT	
FROM	TO	AA DT	% COMM.	AA DT	% COMM.	AA DT	% COMM.	AA DT	% COMM.	AA DT	% COMM.	AA DT	% COMM.
WESTBOUND LANES													
North Talbot Road	Howard Avenue	19,972	27.6	19,954	27.2	19,958	27.4	19,958	27.4	19,936	27.0	19,954	27.2
Howard Avenue	St. Clair College	19,026	27.4	19,664	27.6					19,359	26.8	14,993	27.6
St. Clair College	Pulford Street			16,522	25.2							9,858	29.5
Pulford Street	E.C. Row Expressway			22,550	19.8								
Howard Avenue	Todd Lane					17,735	24.4	17,735	24.4				
Todd Lane	E.C. Row Expressway					22,373	18.0	22,373	18.0				
St. Clair College	Highway 3/Huron Church Road	16,155	24.8									20,784	25.2
St. Clair College	Highway 3/Huron Church Road									16,845	23.0		
Highway 3/Huron Church Road	Malden Road	7,502	55.7										
E.C. Row Expressway	Malden Road			7,414	53.3	7,274	53.8	7,274	53.8	7,336	55.3		
Malden Road	Ojibway Parkway	10,678	43.5	10,591	41.8	10,449	41.7	10,449	41.7	10,498	42.9		
Ojibway Parkway	EC Row Expressway	7,521	59.5	7,296	58.0	7,243	57.8	7,243	57.8	7,381	58.7	7,577	58.0
EC Row Expressway	Canadian Plaza	10,032	58.7	9,987	57.9	9,981	57.8	9,981	57.8	10,009	58.1	9,987	57.9
EASTBOUND LANES													
Canadian Plaza	Ojibway Parkway	18,007	52.4	17,980	52.0	18,028	52.8	18,028	52.8	18,010	52.4	17,980	52.0
Ojibway Parkway	EC Row Expressway	31,533	27.6	31,533	27.6	31,610	28.2	31,610	28.2	31,565	28.1	27,348	27.6
EC Row Expressway	Malden Road	18,914	51.3	18,914	51.3	18,476	52.2	18,476	52.2	18,546	53.3	14,333	51.3
Malden Road	Highway 3/Huron Church Road or E.C. Row Expressway	14,382	58.0	14,231	55.2	13,766	56.4	13,766	56.4	13,841	57.7	14,333	51.3
Highway 3/Huron Church Road	St Clair College	24,917	32.5							25,086	33.7	30,697	27.3
E.C. Row Expressway	Pulford Street			30,422	27.3								
Pulford Street	St Clair College			24,232	34.5							22,166	27.3
E.C. Row Expressway	Todd Lane					29,083	27.3	29,083	27.3				
Todd Lane	Howard Avenue					23,313	30.1	23,313	30.1				
Howard Avenue	Highway 3					27,051	25.3	27,051	25.3				
St Clair College	Howard Avenue	21,477	35.0	21,700	34.9					22,105	34.7	22,874	34.9
Howard Avenue	North Talbot Road	21,529	36.2	21,530	36.3	29,133	27.1	29,133	27.1	21,550	36.3	27,843	36.3

6.0 GRANULAR MATERIALS

The general area of the project lacks crushable gravel. Sandy material may be available from pits in the vicinity of Leamington. The high quality hot mix asphalt aggregates would have to be imported from great distances, either by truck or by water/truck transportation. The availability of aggregates from the following licensed sources needs to be investigated by bidders at the time of tendering. Aggregates transported by water from quarries in the USA, or from Canadian sources, need to be stockpiled and sampled at dock sites.

6.1 Granular Base and Subbase

Aggregate materials for Granular B Type I may be obtained from pits in the sandy delta deposit at Leamington, such as:

- Sterling Acres Farms Limited, Lot 24, Concession 5, Gosfield South Township, 4 kilometres northwest of Leamington.
- Erie Sand and Gravel Limited, Lots 23 and 24, Concessions 3 and 4, Gosfield South Township, 4 kilometres northwest of Leamington.
- Erie Sand and Gravel Limited, Lots 1, 2 and 3, Concession 2, Mersea Township, 2.4 kilometres west of Leamington.

This type of aggregate might also be obtained from pits under wayside permits from the same deposit provided the requirements of the Aggregate Resources of Ontario Provincial Standards under the Aggregate Resources Act are met by the Contractor. Careful selection would be required, as some of the sands are too fine for Granular B Type I.

Granular B Type II aggregates may be available from the following quarries:

- Amherst Quarries (1969) Anderdon quarry, Lots 9 and 10, Concession 7, Anderdon Township, 4 kilometres northwest of McGregor. It should be noted that this quarry has not reportedly been active recently for aggregate production.
- Amherst Quarries (1969) Limited quarry, Lots 21 and 22, Concession 2, Malden Township, 1.5 kilometres east of Amherstberg.
- The 671173 Ontario Limited quarry, Lot 27, the Ben Koop quarry, Lot 13 and the 670028 Ontario Limited quarry, Lots 1, 2, 12 and 13 in Registered Plan 35, Pelee Township, on Pelee Island, approximately 1 kilometre east of the Scudder dock. Transportation for these quarries is through the Kingsville dock. In the past, the main products from these quarries were armor stone, subbase and base aggregates for Pelee Island.
- Lafarge Canada Inc. Manitoulin Dolomite quarry, Lots 41 to 45, Concession 8, Dawson Township, 9.4 kilometres west of Meldrum Bay on Manitoulin Island, or quarries from the USA, all via the docks at Kingsville or the docks in Windsor.

Possible Granular A sources for the project are:

- Amherst Quarries (1969) Anderdon quarry; however excess material passing the 75 micrometre sieve may be problematic.
- Huron Gravel Limited pit in a large kame deposit, Lots 16, 17 and 18, Concession 13 LES, 11.5 kilometres east of Chatham. A dragline is currently in operation. The stone quality of pit run material is borderline for Granular A due to the presence of Kettle Point shale boulders. To improve the product, these boulders should be removed before crushing. The addition of screenings or suitable sand is required due to a lack of fines in order to meet Ministry specifications and to facilitate compaction.
- Lafarge Canada Inc. Manitoulin Dolomite quarry or quarries in the USA.
- Quarries on Pelee Island through the Kingsville dock; however, excess fines passing the 75 micrometre sieve may be problematic.

In addition to the use of virgin materials, since the majority of the pavements at the site are designated for removal and full reconstruction, recycling of the existing pavement materials into new granular base and subbase should be fully explored.

6.2 Asphaltic Concrete

SuperPave mixtures are to be used throughout and SuperPave aggregates would likely be producible from quarries or pits that can currently produce HDDB coarse and fine aggregates, subject to the materials meeting the additional testing requirements. For the top 100 millimetres of SuperPave asphalt, the fine aggregates could be obtained from HDDB fine aggregate sources, if properly processed.

Heavy duty binder course (HDDB) aggregates may be obtained from the following sources:

- Amherst Quarries (1969) Anderdon quarry; only the top bench consisting of Anderdon limestone is suitable.
- Lafarge Canada Inc. Manitoulin Dolomite quarry.
- Quarries in the USA.
- Beachvilime Limited, two quarries in Lots 13, 14 and 15, Broken Front, and Lots 14, 15, 17 and 18, Concessions 2 and 3, Oxford on Thames Geographic Township, 2.5 and 3.5 kilometres, respectively, south of Beachville. Aggregate may break down during handling.
- Global Stone Ingersoll Limited quarry, Lots 13, 14 and 15, Concessions 2, 3 and 4, North Oxford Geographic Township, 4.5 kilometres southwest of Beachville. Aggregate may break down during handling.

HDDB fine aggregates may be available from some of the quarries that can produce HDDB coarse aggregate, if properly processed. The Dufferin Aggregates Milton quarry and the Lafarge Canada Inc. Dundas quarry produce HDDB fine aggregates on a routine basis. The Beachvilime Limited and Global Stone Limited quarries have produced

HDBC fine aggregate, but specific approval for quality and gradation is required as the aggregate may break down during processing.

Asphalt pavement mixes may be produced by portable plants operated by contractors close to the project or by stationary plants in Chatham and/or Windsor. The location of portable asphalt plants in pits or other locations need to be cleared with local municipal authorities.

6.3 Concrete

Concrete aggregates may be available from the following sources:

- Lafarge Canada Inc. Manitoulin Dolomite Quarry;
- Lafarge Canada Inc., Lots 3 and 4, Concession 6, London Township;
- Huron Gravel Ltd., Harwich Township (fine aggregate);
- Osbourne Materials Company, Drummond Island quarry via Kingsville or Windsor;
- Presque Isle Corporation, Michigan, via Kingsville or Windsor;
- Michigan Limestone Operations, via Kingsville or Windsor;
- Smelter Bay Aggregates Inc., Sowerby pit, via Kingsville or Windsor;
- Blue Circle Aggregates, Lot 3, Concession 5, London Township;
- Erie Sand and Gravel Limited, Tyhurst Pit (fine aggregate);
- Lobo Sand and Gravel, Lots 11 and 12, Concession 9, Township of Lobo (fine aggregate); and
- Fowler Construction Company Ltd., 11 kilometres north of junction of Highway 12 and Simcoe County Road 44 (coarse aggregate).

7.0 PAVEMENT DESIGN

7.1 General

The proposed pavement sections have been analyzed using the American Association of State Highway and Transportation Officials (AASHTO) design methods, the Canadian Portland Cement Association (PCA) design method and the Asphalt Institute (1991) design method together with our knowledge of previous MTO experiences and standards and employing good engineering judgement. A discussion of the various pavement design methods used for this project is provided in Section 7.2.

It should be noted that only relatively limited cost information is available for SuperPave mixes at this time. Therefore, the life cycle costing analyses reported throughout this report are based on the costs utilized on recent adjacent projects.

The following are relevant SuperPave mixes together with the lift thicknesses currently specified by MTO:

<u>SUPERPAVE MIX</u>	<u>LIFT THICKNESS (mm)</u>
SuperPave 12.5 FC 2	38 to 50
SuperPave 12.5 FC 1	38 to 50
SuperPave 12.5	38 to 50
SuperPave 19.0	50 to 75
SuperPave 25.0	75 to 100
SuperPave 9.5	nil to 30

7.2 Discussion of Pavement Design Methods

7.2.1 AASHTO (1993) Design Method

The AASHTO design method is a statistically based method calibrated using the results from the AASHTO Road Test carried out in the late 1950's and early 1960's. Thus, there are some limitations to the application of this design method. The limitations are discussed in Section 1.4.2 of the Guide and are summarized below:

- a) limited pavement types (asphaltic concrete, granular base and subbase only for flexible pavements, rigid pavements with and without reinforcement but always with dowels);
- b) limited subgrade types;

- c) traffic loadings (road test terminated at 1,114,000 axle applications, thus use of design equivalent single axle loads (ESALs) in excess of eight million requires extrapolation beyond the equations developed);
- d) age (aging effects are ignored as the road test was terminated after two years); and
- e) environment (road test carried out in Ottawa, Illinois, U.S.A.).

The design method has been updated several times since inception, and recently the MTO undertook a study to adapt the AASHTO method to Ontario conditions and the results were provided in ERES Consultants Final Report entitled “Adaptation and Verification of AASHTO Pavement Design Guide for Ontario Conditions” dated February 22, 2001.

The benefits of employing the AASHTO method, particularly when properly calibrated to Ontario conditions, are that advanced conditions such as reliability, drainage, frost susceptible subgrades and swelling may be incorporated directly.

Since the AASHTO design method has not, until very recently, been scrutinized under Ontario conditions, the computed pavement structure must be verified using experience, judgement and comparison with other accepted pavement design methods.

Flexible Pavements

The results of the AASHTO design method for flexible pavements are sensitive to the input parameters, particularly subgrade resilient modulus and the initial and terminal serviceability factors. In addition, the scaling of layer coefficients is not consistent with the GBE concept. Designing for a specific GBE or structural number (SN) will cause discrepancies with other GBE based methods.

Rigid Pavements

Based on the comparison by Darter, Hall and Kuo (1995) as summarized in the Adaptation of AASHTO for Ontario guide, the largest factors to be questioned in the rigid pavement model are the composite modulus of subgrade reaction, and the J factor. The above referenced review notes that the nomograph for composite modulus of subgrade reaction produces unrealistically high values and overestimates the support actually experienced in the field. In addition, the J factor, which is a stress load factor across cracks and/or joints, is not properly applied in the AASHTO method and is intended to account for corner cracking. Joint faulting is not considered in the AASHTO analyses.

The above and other factors were addressed in the 1998 supplement to the guide, and the 1998 method has been examined and calibrated for Ontario conditions.

7.2.2 Asphalt Institute (AI) Design Method

The AI design method is based on a theoretical multilayered elastic system. However, the results of analyses using the AI method tend to provide asphaltic thicknesses well in excess of those typically employed in current practice. The main reason is the perceived notion that base and/or subbase courses greater than 300 millimetres in thickness do not provide additional structural capacity to the pavement structure as stated in the following excerpt from the AI design guide:

"Some agencies employ greater thicknesses of untreated aggregate base to try to minimize the potential for frost damage in the pavement. Although thicknesses greater than 300 millimetres are used for this purpose, pavement analysis studies have shown that this additional thickness adds little benefit if any to the structural behavior of the pavement system....Reducing the asphalt concrete thickness because of very large thicknesses of untreated aggregate is discouraged by the Asphalt Institute and most pavement designers because the potential for other pavement problems will increase."

The results of the AI design method will be scaled using typical GBE factors to develop a more typical pavement section based on past MTO experience and current design philosophies.

7.2.3 Canadian Portland Cement Association (PCA) Design Method

As indicated in the PCA "Thickness Design for Concrete Highway and Street Pavements" manual, the pavement design methods are based on pavement theory, performance, and research experience from:

- theoretical pavement slab studies by Westergaard and Pichett & Ray, and the results of finite element analyses,
- model and full scale testing,
- experimental pavements, and
- performance of existing pavements.

It is indicated that an emphasis has been placed on calibration of the design model to the performance of existing pavements.

The design method allows independent consideration of subgrade and base courses, subgrade resilient modulus, traffic and includes consideration of fatigue (to control fatigue cracking) and erosion (to control subgrade and shoulder erosion, pumping and faulting). The analysis can also consider plain jointed, doweled and continuously reinforced pavements.

The results of the PCA design method will be compared to the results of the AASHTO analyses and MTO experience to determine an appropriate design slab thickness.

7.2.4 Comparison of Pavement Design Methods

Comparisons of the results of the various pavement design methods were carried out as part of the pavement design process. The results of the AASHTO and Asphalt Institute flexible pavement design methods and the results of the AASHTO and Canadian Portland Cement Association rigid pavement design methods are compared on Figure 8 for the project specific conditions. Subgrade condition 1, as described in Sections 7.5.2 and 7.5.4 below, were used for the comparison.

Based on the results of the comparisons and our experience in the Windsor area, the AASHTO method yields reasonable results and has been used as the primary design tool for assessing the various pavement design alternatives.

7.3 Discussion of Life Cycle Cost Methods

The life cycle costing has been carried out in accordance with ERES Consultants Inc. Report entitled "Review of Life Cycle Costing Analysis Procedures" dated December 1998. Consistent with MTO direction, the discount rate utilized in the analyses is 5.0 per cent and, for flexible pavements, rehabilitation strategies have consisted of milling 50 millimetres and paving 50 millimetres of SuperPave 19.0 and 40 millimetres of surface course appropriate for the traffic conditions. For concrete pavements, a 50 millimetre asphalt overlay is provided in year 38.

The unit rates used in the life cycle costing are:

MATERIAL/ACTIVITY	UNIT RATE
SuperPave 12.5 FC2	\$140/tonne
SuperPave 12.5 FC1	\$120/tonne
SuperPave 12.5	\$85/tonne
SuperPave 19.0	\$90/tonne
SuperPave 25.0	\$90/tonne
OGDL	\$13/tonne
Granular A Base	\$20/tonne
Granular B Type III Subbase	\$18/tonne
Concrete – 175 to 200 mm	\$64/m ²
Concrete – 200 to 225 mm	\$66/m ²
Concrete – 225 to 260 mm	\$68/m ²
Concrete – 260 to 290 mm	\$70/m ²
Concrete – 290 to 330 mm	\$73/m ²
rout and seal	\$1.50/m

<u>MATERIAL/ACTIVITY</u>	<u>UNIT RATE</u>
milling (25-50mm depth)	\$1.75/m ²
milling (75-100mm depth)	\$2.25/m ²
asphalt patching	\$50/m ²
concrete joint reseal	\$5/m
surface texturing	\$5/m ²
subdrains	\$25/m

7.4 Concept Pavement Design

Based on the information provided in the Request for Proposal for this project, the MTO has advised that typical pavement designs for freeways in the area of this project consist of:

<u>RIGID PAVEMENT</u>		<u>FLEXIBLE PAVEMENT</u>	
<u>COMPONENT</u>	<u>THICKNESS</u>	<u>COMPONENT</u>	<u>THICKNESS</u>
	(mm)		(mm)
Plain Jointed Concrete	260	Stone Mastic Asphalt	40
Open Graded Drainage Layer	100	SuperPave 19.0	70
Granular A Base	300	SuperPave 25.0	190 (2@95)
		Open Graded Drainage Layer	100
		Granular A Base	500

The above noted pavement structures include continuous subdrains hydraulically connected to the open graded drainage layer (OGDL) that discharges to a positive gravity outlet.

It should be noted that the rigid pavement design noted above is considered to be applicable for the better drained and typically stiffer upper weathered crust in the area and may not necessarily be appropriate for the typical weaker and not as well drained soils in the depressed profile areas.

7.5 Pavement Design Model

7.5.1 Traffic Loading

The average annual daily traffic (AADT) loadings and per cent commercial vehicles for the various pavements under consideration in the Area of Continued Analysis are detailed in Tables I-A to I-F, attached. Calculations of the traffic loadings in terms of equivalent single axle loads (ESALs) were completed using the traffic data provided by URS and appropriate truck factors and lane distribution factors determined based on the anticipated

volume and nature of the loadings and geometry of the proposed pavements. The truck and lane distribution factors utilized are summarized below:

<u>ROAD CLASSIFICATION</u>	<u>NUMBER OF LANES</u> (each direction)	<u>APPROXIMATE ESALs (millions)</u>		<u>TRUCK FACTOR</u>	<u>LANE DISTRIBUTION FACTOR</u>
		<u>Flexible Pavt's</u>	<u>Rigid Pavt's</u>		
Local	1	less than 0.2	less than 10	1.5	1.0
Minor Collector	1	0.2 to 1	10 to 20	1.5	1.0
Major Collector	1 to 2	1 to 6	20 to 50	1.5	1.0
Minor Arterial	2	6 to 20	50 to 100	2.0	0.8 to 0.9
Major Arterial	2	20 to 50	100 to 200	2.0	0.8 to 0.9
Freeway	3	50 to 125	greater than 200	2.5	0.6 to 0.8

The truck factors in the table were based on the anticipated heavy traffic loading conditions for the various types of pavements. The lane distribution factors were determined based on the proposed pavement lane configurations, traffic volumes and Table 8-3 of the Ontario AASHTO design guidelines.

The cumulative ESAL loading were used to describe the roadway classification for the purposes of this report. While it may be more convenient for the reader to classify the pavements based on AADT or the like, varying percentage of commercial traffic, lane configuration and the like would not be taken into account, therefore, classification by ESALs was utilized.

The cumulative 19 and 21 year ESALs for the various flexible pavements are summarized in Tables II-A to II-F, attached. The freeway pavements have been based on an initial design life of 21 years and the remainder of the pavements based on an initial design life of 19 years, consistent with MTO standards.

The cumulative 25 year ESALs for the various rigid pavements are summarized in Tables III-A to III-F, attached.

7.5.2 AASHTO Analyses – Flexible Pavements

To carry out the AASHTO analyses, estimates of initial and terminal serviceability indices (ISI & TSI, respectively) and reliability (R) are required. The criteria for selecting the appropriate values of these parameters is dependent on the anticipated volume, the nature of the loadings and geometry of the proposed pavements. The initial and terminal serviceability indices and reliability used for the various pavements are summarized below:

<u>ROAD CLASSIFICATION</u>	<u>INITIAL SERVICEABILITY INDEX</u>	<u>TERMINAL SERVICEABILITY INDEX</u>	<u>RELIABILITY (%)</u>
Local	4.5	2.0	50
Minor Collector	4.5	2.0	75
Major Collector	4.5	2.0	75
Minor Arterial	4.5	2.2	85
Major Arterial	4.5	2.5	90
Freeway	4.5	2.5	95

The initial and terminal serviceability indices were selected in general accordance with the revised Table 8-4 of the Ontario AASHTO design guidelines. The design reliability was selected based on experience gained with other similar projects and the AASHTO design guidelines.

Subgrade Resilient Modulus, M_R

Based on the nature of the subgrade soils and our related experience in the Windsor area, a subgrade resilient modulus of 25 megapascals (MPa) is considered appropriate for the clayey crust soils that typically have an undrained shear strength in excess of about 100 kilopascals and is consistent with the recommendations for low to medium plasticity clays as indicated in Table 8.6 of the "Adaptation and Verification of AASHTO Pavement Design Guide for Ontario Conditions" document and is also consistent with the typical pavement thicknesses constructed by MTO in this area.

As indicated in Section 3.1.2 and on Figures 5, 6 and 7, the undrained shear strength beneath the upper weathered crust decreases significantly. While undrained shear strength cannot be directly correlated to resilient modulus, the shear strength data clearly indicates a reduction in support characteristics for the pavements. Therefore, for the various alternatives, the resilient moduli used in the analyses are:

<u>SUBGRADE CONDITION</u>	<u>SUBGRADE UNDRAINED SHEAR STRENGTH (kPa)</u>	<u>SUBGRADE RESILIENT MODULUS (MPa)</u>
1	greater than 100	25
2	75 to 100	20
3	less than 75	15

Based on the results of the investigation carried out for the foundations component of the project, the following table summarizes the subgrade conditions expected at various locations throughout the project for subgrade levels at grade, 3 metres below grade and 6 metres below grade:

LOCATION	SUBGRADE CONDITION		
	At Grade	3 m Below Grade	6 m Below Grade
<i>Proposed Highway 401</i>			
Highway 401/Highway 3 to Geraedts Drive	1	1	2
Geraedts Drive to 700m west of Huron Church Road	1	1	3
700m west of Huron Church Road to Cabana Road	1	1	2
Cabana Road to 100 m north of Pulford Street	1	1	3
100m north of Pulford Street to 200m west of Malden Road	1	1	2
200 to 600 m west of Malden Road	1	2	2
600 m west of Malden Road to Ojibway Parkway	1	2	3
Ojibway Parkway (E.C. Row Expressway to Sandwich Street)	1	2	3

Structural Number

The above-noted classifications and the required structural numbers suggested for each roadway class are summarized on Figures 9, 10 and 11, attached, for the three subgrade conditions considered.

Utilizing the above information, the following representative flexible pavement structural numbers were calculated for the major pavements to be constructed. The following structural numbers provide a general sense of the pavement structure required. Detailed information for the individual pavement sections analyzed is provided in Tables II-A to II-F:

LOCATION	STRUCTURAL NUMBER (mm)		
	Condition 1	Condition 2	Condition 3
Highway 401			
- westbound	208	222	240
- eastbound	222	236	255
Huron Church Road			
- northbound	144	154	167
- southbound	177	189	205
E.C. Row Expressway			
- westbound	148	158	172
- eastbound	174	185	201

LOCATION	STRUCTURAL NUMBER (mm)		
	Condition 1	Condition 2	Condition 3
Ojibway Parkway			
- northbound	112	120	130
- southbound	135	145	158

The structural analyses for the proposed new pavement sections were carried out using the design nineteen year traffic loadings and the following equivalency factors and AASHTO layer coefficients:

MATERIAL	EQUIVALENCY FACTOR	AASHTO LAYER COEFFICIENT	AASHTO DRAINAGE COEFFICIENT
Asphalt	2.0	0.42	-
Granular A Base	1.0	0.14	1.0
Granular B Type III Subbase	0.67	0.09	1.0
Granular B Type II Subbase	1.0	0.14	1.0

7.5.3 Preliminary Flexible Pavement Design

As noted above, while it may be more convenient for the reader to classify the pavements based on AADT or the like, varying percentage of commercial traffic, lane configuration and the like would not be taken into account, therefore, classification by ESALs was utilized. To aid in the assessment of appropriate pavement structures for the various roadways under consideration, Tables II-A to II-F summarize the cumulative ESALs and roadway classification to be used for design.

Based on the results of the analyses, appropriate pavement structures for the various pavements within the project consist of:

- Subgrade Condition 1

ROAD CLASSIFICATION	RANGE OF ESALs (millions)	STRUCTURAL NUMBER (mm)	PAVEMENT COMPONENT THICKNESS (mm)			
			Asphalt		OGDL	Granular A Base
			Surface	Binder		
Local	less than 0.2	<75	40	50	-	300
Minor Collector	0.2 to 1	75-95	40	90	-	300
Major Collector	1 to 6	95-120	40	110	-	400
Minor Arterial	6 to 20	120-150	40	120	-	550
Major Arterial	20 to 50	150-180	40	190	-	600
Freeway	50 to 125	>180	40	260	100	500

- Subgrade Condition 2

<u>ROAD CLASSIFICATION</u>	<u>RANGE OF ESALs (millions)</u>	<u>STRUCTURAL NUMBER (mm)</u>	<u>PAVEMENT COMPONENT THICKNESS (mm)</u>			
			<u>Asphalt</u>		<u>OGDL</u>	<u>Granular A Base</u>
			<u>Surface</u>	<u>Binder</u>		
Local	less than 0.2	<75	40	50	-	300
Minor Collector	0.2 to 1	75-95	40	90	-	350
Major Collector	1 to 6	95-130	40	120	-	450
Minor Arterial	6 to 20	130-160	40	150	-	600
Major Arterial	20 to 50	160-190	40	220	-	600
Freeway	50 to 125	>190	40	260	100	600

- Subgrade Condition 3

<u>ROAD CLASSIFICATION</u>	<u>RANGE OF ESALs (millions)</u>	<u>STRUCTURAL NUMBER (mm)</u>	<u>PAVEMENT COMPONENT THICKNESS (mm)</u>			
			<u>Asphalt</u>		<u>OGDL</u>	<u>Granular A Base</u>
			<u>Surface</u>	<u>Binder</u>		
Local	less than 0.2	<75	40	50	-	350
Minor Collector	0.2 to 1	75-100	40	90	-	350
Major Collector	1 to 6	100-135	40	130	-	500
Minor Arterial	6 to 20	135-180	40	190	-	600
Major Arterial	20 to 50	180-220	40	260	-	600
Freeway	50 to 125	>220	40	300	100	650

Alternative 3 involves the construction of cut and cover tunnels to convey the traffic and it is understood that the Highway 401 main line pavements will be constructed over the tunnel base slab. Therefore, the Alternative 3 main line pavements should consist of:

<u>COMPONENT</u>	<u>THICKNESS (mm)</u>
Surface Asphalt	40
Binder Asphalt	160
OGDL	100

Tables II-A to II-F provide the detailed results of the ESAL calculations and the resultant pavement classification based on the above for each of the pavement sections examined.

In addition, for comparison, alternative pavement designs were considered for Subgrade Condition 1 that incorporate a granular subbase component. These are summarized as follows:

ROAD CLASSIFICATION	RANGE OF ESALs (millions)	PAVEMENT COMPONENT THICKNESS (mm)			
		Asphalt		Granular A	Granular B Type III
		Surface	Binder	Base	Subbase
Local	less than 0.2	40	50	100	300
Minor Collector	0.2 to 1	40	90	100	300
Major Collector	1 to 6	40	110	150	400
Minor Arterial	6 to 20	40	120	150	650
Major Arterial	20 to 50	40	190	150	650
Freeway	50 to 125	40	260	100 OGDL 100 Gran A	650

7.5.4 AASHTO Analyses – Rigid Pavements

For the rigid pavement analyses, the initial and terminal serviceability indices and reliabilities used for the flexible pavement analyses were utilized.

Modulus of Subgrade Reaction

Based on the nature of the subgrade soils and our related experience, a modulus of subgrade reaction of 30 megapascals per metre (MPa/m) is considered appropriate for the clayey crust soils that typically have an undrained shear strength in excess of about 100 kilopascals and is consistent with the recommendations for low to medium plasticity clays as indicated in Table 8.9 of the “Adaptation and Verification of AASHTO Pavement Design Guide for Ontario Conditions” document.

As indicated in Section 3.1.2 and on Figures 5, 6 and 7, the undrained shear strength beneath the upper weathered crust decreases significantly. While undrained shear strength cannot be directly correlated to modulus of subgrade reaction, the shear strength data clearly indicates a reduction in support characteristics for the pavements. Therefore, for the various alternatives, the moduli of subgrade reaction used in the analyses are:

SUBGRADE CONDITION	SUBGRADE UNDRAINED SHEAR STRENGTH (kPa)	MODULUS OF SUBGRADE REACTION (MPa/m)
1	greater than 100	30
2	75 to 100	20
3	less than 75	10

The table previously provided in Section 7.5.2 summarizes the subgrade conditions expected at various locations throughout the project for subgrade levels of at grade, 3 metres below grade and 6 metres below grade.

The structural analyses for the proposed new pavement sections were carried out using the design twenty five year traffic loadings, in terms of ESALs, and the effective moduli of subgrade reaction for various Granular A and OGDL thicknesses beneath the slabs for the following effective moduli of subgrade reaction, k.

<u>COMBINED THICKNESS OF GRANULAR A AND OGDL (mm)</u>	<u>EFFECTIVE MODULUS OF SUBGRADE REACTION (MPa/m)</u>		
	<u>k=30MPa/m</u>	<u>k=20MPa/m</u>	<u>k=10MPa/m</u>
300	52	38	24
400	62	46	30

The values for effective modulus of subgrade reaction are based on Table 1 in the CPCA “Thickness Design for Concrete Highway and Street Pavements.”

In addition to the above, the following input parameters were used in the AASHTO rigid pavement analyses:

<u>PARAMETER</u>	<u>VALUE</u>
Modulus of Rupture, S_c'	5 MPa
Elastic Modulus, E_c	30,000 MPa
Load Transfer Coefficient, J	
- 0.5m monolithic shoulder	2.5
- no tied shoulder	3.2

The appropriate pavement classifications and the required slab thicknesses suggested for each roadway class and subgrade condition, based on the above input information, are summarized on Figures 9, 10 and 11.

7.5.5 Preliminary Rigid Pavement Design

As noted above, while it may be more convenient for the reader to classify the pavements based on AADT or the like, varying percentage of commercial traffic, lane configuration and the like would not be taken into account, therefore, classification by ESALs was utilized. To aid in the assessment of appropriate pavement structures for the various roadways under consideration, Tables III-A to III-F summarize the cumulative ESALs and roadway classification to be used for design.

Based on the results of the analyses, appropriate pavement structures for the various pavements within the project consist of:

- Subgrade Condition 1

<u>ROAD CLASSIFICATION</u>	<u>RANGE OF ESALs</u> (millions)	<u>PAVEMENT COMPONENT THICKNESS (mm)</u>		
		<u>Cement Concrete</u>	<u>OGDL</u>	<u>Granular A Base</u>
Local	less than 10	concrete pavement not warranted		
Minor Collector	10 to 20	175	-	300
Major Collector	20 to 50	210	-	300
Minor Arterial	50 to 100	260	-	300
Major Arterial	100 to 200	290	100	300
Freeway	greater than 200	330	100	300

- Subgrade Condition 2

<u>ROAD CLASSIFICATION</u>	<u>RANGE OF ESALs</u> (millions)	<u>PAVEMENT COMPONENT THICKNESS (mm)</u>		
		<u>Cement Concrete</u>	<u>OGDL</u>	<u>Granular A Base</u>
Local	less than 10	concrete pavement not warranted		
Minor Collector	10 to 20	175	-	300
Major Collector	20 to 50	220	-	300
Minor Arterial	50 to 100	260	-	300
Major Arterial	100 to 200	300	100	300
Freeway	greater than 200	330	100	300

- Subgrade Condition 3

<u>ROAD CLASSIFICATION</u>	<u>RANGE OF ESALs</u> (millions)	<u>PAVEMENT COMPONENT THICKNESS (mm)</u>		
		<u>Portland Cement Concrete</u>	<u>OGDL</u>	<u>Granular A Base</u>
Local	less than 10	concrete pavement not warranted		
Minor Collector	10 to 20	190	-	300
Major Collector	20 to 50	225	-	300
Minor Arterial	50 to 100	260	-	400
Major Arterial	100 to 200	300	100	400
Freeway	greater than 200	330	100	400

Alternative 3 involves the construction of cut and cover tunnels to convey the traffic and it is understood that the Highway 401 main line pavements will be constructed over the tunnel base slab. Therefore, the Alternative 3 main line pavements should consist of:

<u>COMPONENT</u>	<u>THICKNESS (mm)</u>
Portland Cement Concrete	260
OGDL	100

Tables III-A to III-F provide the detailed results of the ESAL calculations and the resultant pavement classification based on the above for each of the pavement sections examined.

7.6 Life Cycle Cost Analyses

Life cycle cost analyses were carried out for the alternative pavement designs above consistent with the procedures outlined in Section 7.3. In accordance with MTO procedures, the following maintenance and rehabilitation strategies were utilized in assessing the life cycle costs:

<u>YEAR</u>	<u>LIGHT AND MEDIUM DUTY PAVEMENTS</u>	<u>HEAVY DUTY PAVEMENTS (FREEWAY)</u>	<u>CONCRETE PAVEMENTS</u>
3	Rout & Seal (59m)	Rout & Seal (59m)	
9	Mill 40mm and Patch (293m ²) Rout & Seal (156m)	Mill 40mm and Patch (293m ²) Rout & Seal (156m)	
12			Joint Reseal (50% of transverse joints, 25% of longitudinal joints)
15	Mill 40mm and Patch (880 m ²) Rout & Seal (352m)	Mill 40mm and Patch (880m ²) Rout & Seal (352m)	
18			Diamond Grinding, Resealing, Patching
19	Mill 50mm and 90mm Overlay	Mill 40mm and Patch (440m ²) Rout & Seal (244m)	
21		Mill 50mm and 90mm Overlay	
22	Rout & Seal (59m)		
24		Rout & Seal (59m)	
27	Mill 40mm and Patch (440m ²) Rout & Seal (244m)		
28		Mill 40mm and Patch (440m ²) Rout & Seal (352m)	Extensive Patching, Diamond Grinding, Joint Resealing
31	Mill 50mm and 90mm Overlay	Rout & Seal (244m)	
34	Rout & Seal (59m)	Mill 50mm and 90mm Overlay	
37		Rout & Seal (59m)	
38	Mill 40mm and Patch (440m ²) Rout & Seal (244m)		50mm asphalt overlay
41			Rout & Seal (70% of transverse cracks)
42	Mill 50mm and 90mm Overlay	Mill 40mm and Patch (440m ²) Rout & Seal (352m)	
44		Rout & Seal (244m)	Rout & Seal (30% of transverse cracks, 50% of longitudinal cracks)
45	Rout & Seal (59m)		
46		Mill 50mm and 90mm Overlay	
48	Mill 40mm and Patch (440m ²) Rout & Seal (244m)		
49		Rout & Seal (59m)	

These strategies are illustrated on Figure 12.

Using the above-noted rehabilitation strategies, the following life cycle costs were determined:

- Subgrade Condition 1

ROAD CLASSIFICATION	LIFE CYCLE COST (\$ PER LANE KM)					
	Flexible Pavement				Rigid Pavement	
	Granular Base Alternative		Granular Subbase Alternative		Rigid Pavement	
	Capital Cost	Total Life Cycle Cost	Capital Cost	Total Life Cycle Cost	Capital Cost	Total Life Cycle Cost
Local	109,400	192,964	122,020	205,529		
Minor Collector	137,890	221,330	150,704	234,088	272,990	334,651
Major Collector	180,694	271,672	201,952	292,837	280,959	344,616
Minor Arterial	201,689	292,576	233,057	323,807	275,471	339,151
Major Arterial	264,991	360,016	288,502	383,424	284,752	349,552
Freeway	382,246	479,759	413,684	511,060	346,807	411,337
Tunnel Section	208,601	306,871	208,601	306,871	270,326	335,189

- **bold** represents preferred alternative

Based on the life cycle costing, flexible pavements are preferred for the local roadways, minor and major collector and minor arterial pavements. Rigid pavements are preferred for the major arterial and freeway pavements.

- Subgrade Condition 2

ROAD CLASSIFICATION	LIFE CYCLE COST (\$ PER LANE KM)			
	Flexible Pavement		Rigid Pavement	
	Capital Cost	Total Life Cycle Cost	Capital Cost	Total Life Cycle Cost
Local	109,400	192,964	-	-
Minor Collector	146,808	230,209	272,990	334,651
Major Collector	197,284	288,190	281,488	345,142
Minor Arterial	230,935	321,694*	275,471	339,151*
Major Arterial	286,125	381,058	294,303	359,061
Freeway	404,541	501,957	346,807	411,337

*these alternatives are considered equivalent since the life cycle costs are within 5 per cent.

Based on the life cycle costing, flexible pavements are preferred for the local roadways and minor and major collector pavements. Rigid pavements are preferred for the major arterial and freeway pavements and the flexible and rigid pavement alternatives are considered to be equivalent for the minor arterial pavements.

- Subgrade Condition 3

ROAD CLASSIFICATION	LIFE CYCLE COST (\$ PER LANE KM)			
	Flexible Pavement		Rigid Pavement	
	Capital Cost	Total Life Cycle Cost	Capital Cost	Total Life Cycle Cost
Local	118,180	201,706	-	-
Minor Collector	146,808	230,209	273,750	335,408
Major Collector	214,123	304,955	281,755	345,408
Minor Arterial	259,066	349,702*	291,406	355,017*
Major Arterial	314,353	409,163	310,376	375,064
Freeway	446,678	543,910	367,927	432,364

*these alternatives are considered equivalent since the life cycle costs are within 5 per cent.

Based on the life cycle costing, flexible pavements are preferred for the local roadways and minor and major collector pavements. Rigid pavements are preferred for the major arterial and freeway pavements and the flexible and rigid pavement alternatives are considered to be equivalent for the minor arterial pavements.

The above-noted total life cycle costs are referred to year 2011 dollars. The alternatives with the lowest total life cycle costs are indicated in **bold** for each road classification and each pavement type. However, based on both the MTO's and our experience in the Windsor area, rigid pavements are preferred at major intersections and any areas where frequent starting/stopping of heavy vehicles is expected.

8.0 PRELIMINARY RECOMMENDATIONS

8.1 Pavement Components

Based on the results of our analyses and the results of the life cycle costing, flexible pavements should be utilized for the local roadways, minor and major collectors and minor arterials, as follows:

<u>COMPONENT</u>	<u>THICKNESS (mm)</u>			
	<u>Local</u>	<u>Minor Collector</u>	<u>Major Collector</u>	<u>Minor Arterial</u>
Subgrade Condition 1				
Surface Asphalt	40	40	40	40
Binder Asphalt	50	90 (2 @ 45)	110 (2 @ 55)	120 (2 @ 60)
Granular A Base	300	300	400	550
Subgrade Condition 2				
Surface Asphalt	40	40	40	40
Binder Asphalt	50	90 (2 @ 45)	120 (2 @ 60)	150 (2 @ 75)
Granular A Base	300	350	450	600
Subgrade Condition 3				
Surface Asphalt	40	40	40	40
Binder Asphalt	50	90 (2 @ 45)	130 (2 @ 65)	190 (2 @ 60, 1 @ 70)
Granular A Base	350	350	500	600

However, based on both the MTO's and our experience in the Windsor area, rigid pavements are preferred at major intersections and any areas where frequent starting/stopping of heavy vehicles is expected.

The major arterial and freeway pavements should consist of rigid pavements, as follows:

<u>COMPONENT</u>	<u>THICKNESS (mm)</u>		
	<u>Major Arterial</u>	<u>Freeway</u>	<u>Tunnel</u>
Subgrade Condition 1			
Concrete	290	330	260
OGDL	100	100	100
Granular A	300	300	-

COMPONENT	THICKNESS (mm)		
	Major Arterial	Freeway	Tunnel
Subgrade Condition 2			
Concrete	300	330	260
OGDL	100	100	100
Granular A	300	300	-
Subgrade Condition 3			
Concrete	300	330	260
OGDL	100	100	100
Granular	400	400	-

All of the above-noted structures incorporate a 0.5 metre wide tied monolithic shoulder.

8.2 Mixture Types

Based on the anticipated traffic volumes and roadway usage, the following mixtures are recommended for the flexible pavements:

- Subgrade Condition 1

ROAD CLASSIFICATION	SURFACE COURSE			BINDER COURSES		
	Mixture	Thickness (mm)	Traffic Category	Mixture	Thickness (mm)	Traffic Category
Local	SuperPave 12.5	40	A	SuperPave 19.0	50	A
Minor Collector	SuperPave 12.5	40	B	SuperPave 19.0	90 (2@45)	B
Major Collector	SuperPave 12.5	40	C	SuperPave 19.0	110 (2@55)	C
	FC1					
Minor Arterial	SuperPave 12.5	40	D	SuperPave 19.0	120 (2@60)	D
	FC1					

- Subgrade Condition 2

ROAD CLASSIFICATION	SURFACE COURSE			BINDER COURSES		
	Mixture	Thickness (mm)	Traffic Category	Mixture	Thickness (mm)	Traffic Category
Local	SuperPave 12.5	40	A	SuperPave 19.0	50	A
Minor Collector	SuperPave 12.5	40	B	SuperPave 19.0	90 (2@45)	B
Major Collector	SuperPave 12.5 FC1	40	C	SuperPave 19.0	120 (2@60)	C
Minor Arterial	SuperPave 12.5 FC1	40	D	SuperPave 19.0	150 (2@75)	D

- Subgrade Condition 3

ROAD CLASSIFICATION	SURFACE COURSE			BINDER COURSES		
	Mixture	Thickness (mm)	Traffic Category	Mixture	Thickness (mm)	Traffic Category
Local	SuperPave 12.5	40	A	SuperPave 19.0	50	A
Minor Collector	SuperPave 12.5	40	B	SuperPave 19.0	90 (2@45)	B
Major Collector	SuperPave 12.5 FC1	40	C	SuperPave 19.0	130 (2@65)	C
Minor Arterial	SuperPave 12.5 FC1	40	D	SuperPave 19.0	190 (2@60, 1@70)	D

8.3 Construction Considerations

The granular materials should extend laterally full width and daylight to ditches having inverts at least 0.5 metres below subgrade level. In urban sections or in areas where ditches are not practical or feasible, perforated, continuous subdrains should be installed with outlets at appropriate intervals.

For the flexible pavements, if the top lift of binder asphalt is exposed through a winter, thorough cleaning and visual inspection will be required prior to placement of tack coat and surface course asphalt.

8.4 Paved Shoulders

Paved shoulders throughout the project should consist of two lifts of asphalt. Lift thicknesses should be equivalent to the adjacent surface course and upper binder course. The surface and binder courses for the non-driveable paved shoulders should be SuperPave 12.5. In any areas where tracking onto paved shoulders is considered likely, such as the inner loops of interchange ramps, the full design lane asphalt thickness and mix type should be provided.

8.5 Asphalt Cement Grades

Performance grade asphalt cement is to be used on this project. The asphalt cement grades to be used on this project are:

LOCATION	COMPONENT	PGAC
Major Collector and Minor Arterial	Surface Course	64-28
	Upper Binder Course	64-28
	Lower Binder Courses	58-28

<u>LOCATION</u>	<u>COMPONENT</u>	<u>PGAC</u>
Local Roads and Minor Collector	All components	58-28

8.6 Tack Coat

Tack coat should be applied between all asphaltic lifts and on any milled surfaces consistent with MTO Southwestern Region directives.

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9.0 CUT MATERIALS AND DESIGN

9.1 Transition Point Treatment

Transition treatment from earth cuts to earth fills and at the limits of subexcavation areas are to be provided in accordance with Ontario Provincial Standard Drawing (OPSD) 205.01. The transition treatment depth, t , should be taken as 1.0 metres less the thickness of asphalt or concrete. For preliminary purposes, the thickness of organic, leached and accumulation layers, D_a , should be taken as 0.0 metres.

For this project, the frost penetration depth should be taken as 1.0 metres.

Reference should be made to the approach foundation engineering report for additional information regarding the requirement for cut stability and construction issues.

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10.0 EMBANKMENT MATERIALS AND DESIGN

10.1 Embankment Construction

The embankments should be constructed using good quality, approved earth borrow. Based on the project location, it is anticipated that clayey borrow materials will be utilized. Following stripping of the surficial topsoil and any softened or unsuitable materials, the embankment fill material should be placed in maximum 300 millimetre thick loose lifts and compacted. Upon completion of filling to proposed subgrade level, the embankment side slopes should be trimmed to a final inclination of 2.5 horizontal to one vertical or flatter.

Reference should be made to the appropriate foundation engineering reports for additional information regarding requirement for embankment, settlements and construction issues.

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11.0 DRAINAGE

11.1 Ditches

The ditches throughout the project should be constructed in accordance with the OPSD 200 series for earth grading with inverts at least 0.5 metres below subgrade level. Subdrains should be provided where this minimum ditch depth is not practical.

Pipe subdrains should be perforated, 100 millimetres in diameter, and placed in a geotextile lined trench 300 millimetres wide in accordance with OPSD 207.041. The trench should be backfilled with 19 millimetre clear stone or OGDL aggregates. At least 300 millimetres of overlap should be provided between the joints in the geotextile.

Any ditch areas which may be wet from conditions such as poor grading or from adjacent surface flows or the like, should be provided with an approved geotextile and appropriately sized rip rap consisting of angular stone or recycled concrete conforming to OPSS 1004.

Geotextile used for pipe subdrains should be Class I, non-woven with a FOS of 75 to 150 microns. Geotextile used under rip rap should be Class II, non-woven with a FOS of 75 to 150 microns.

12.0 OTHER DESIGN FEATURES

12.1 Recycling of Existing Pavement Materials

Consideration may be given to recycling the existing asphalt, concrete and/or granular materials into the new pavements. A maximum of 20 per cent recycled asphalt pavement (RAP) may be utilized in the binder courses throughout below a depth of 150 millimetres. A maximum of 30 per cent RAP may be utilized in the granular base in any unpaved shoulders. The contractor must demonstrate that he can produce a suitable product that meets all of the appropriate material specifications on a consistent basis.

MEB/PRB/MLJM/cr
\\win1-s-filesrv1\data\active\2005\1140-000\05-1140-003 detroit river international crossing, ea, windsor\05-1140-003 urs - pavements\reports\05-1140-003-1\updated draft - mar 14 08\mar 14 08 - (updated draft) - rpt text - pvmt eng planning - dric.doc

DRAFT

TABLE 1B

SUMMARY OF TRAFFIC DATA - ALTERNATIVE 1B

Detroit River International Crossing
Windsor, Ontario

Table with columns: LOCATION (Pavement Section, From, To), 2015 (AADT, Comm Vehicles), 2025 (AADT, Comm Vehicles), 2035 (AADT, Comm Vehicles). Rows include Huron Church Road, South Service Road, North Service Road, Ojibway Parkway, CROSS ROADS, HIGHWAY 401 Ramps, and Highway 401 Mainline.

TABLE I-E
SUMMARY OF TRAFFIC DATA - ALTERNATIVE 3

Detroit River International Crossing
Windsor, Ontario

Table with columns: Pavement Section, LOCATION (From, To), 2015 (AADT, Comm Vehicles), 2025 (AADT, Comm Vehicles), 2035 (AADT, Comm Vehicles). Rows include various road sections like Huron Church Road, Talbot Road, Ojibway Parkway, and Highway 401 Ramps/Mainline.

TABLE II-B

SUMMARY OF ESAL CALCULATIONS - FLEXIBLE PAVEMENTS - ALTERNATIVE 1B

Detroit River International Crossing
Windsor, Ontario

Table with columns: Pavement Section, LOCATION (From, To), LDF, TF, Design Period (years), ESALs, Class, LDF, TF, Design Period (years), ESALs, Class. Rows include Huron Church Road, South Service Road, North Service Road, Ojibway Parkway, CROSS ROADS, and HIGHWAY 401 Ramps.

SUMMARY OF ESAL CALCULATIONS - RIGID PAVEMENTS - ALTERNATIVE 1A

Detroit River International Crossing
Windsor, Ontario

Table with columns: Pavement Section, LOCATION (From, To), Northbound/Westbound (LDF, TE, Design_Period, ESALs, Class), and Southbound/Eastbound (LDF, TE, Design_Period, ESALs, Class). Rows include Huron Church Road, South Service Road, North Service Road, Ojibway Parkway, CROSS ROADS, and HIGHWAY 401 Ramps.

SUMMARY OF ESAL CALCULATIONS - RIGID PAVEMENTS - ALTERNATIVE 1B

Detroit River International Crossing Windsor, Ontario

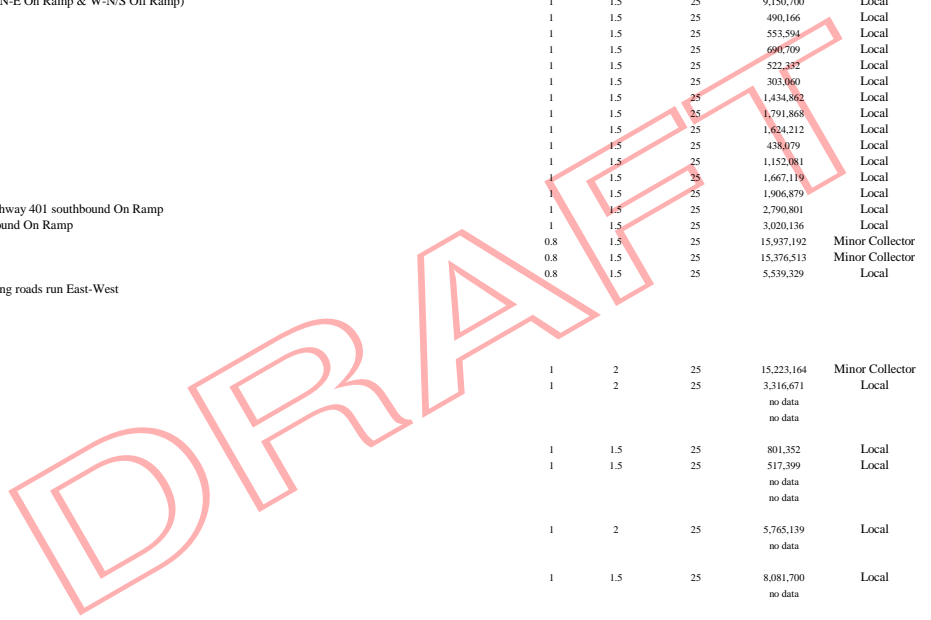
Table with columns: LOCATION, Pavement Section, From, To, LDF, TF, Design Period (years), ESALs, Class, Southbound/Eastbound (LDF, TF, Design Period, ESALs, Class). Rows include Huron Church Road, South Service Road, North Service Road, Ojibway Parkway, CROSS ROADS, HIGHWAY 401 Ramps, Highway 401 Mainline, and Malden Road.

SUMMARY OF ESAL CALCULATIONS - RIGID PAVEMENTS - ALTERNATIVE 3

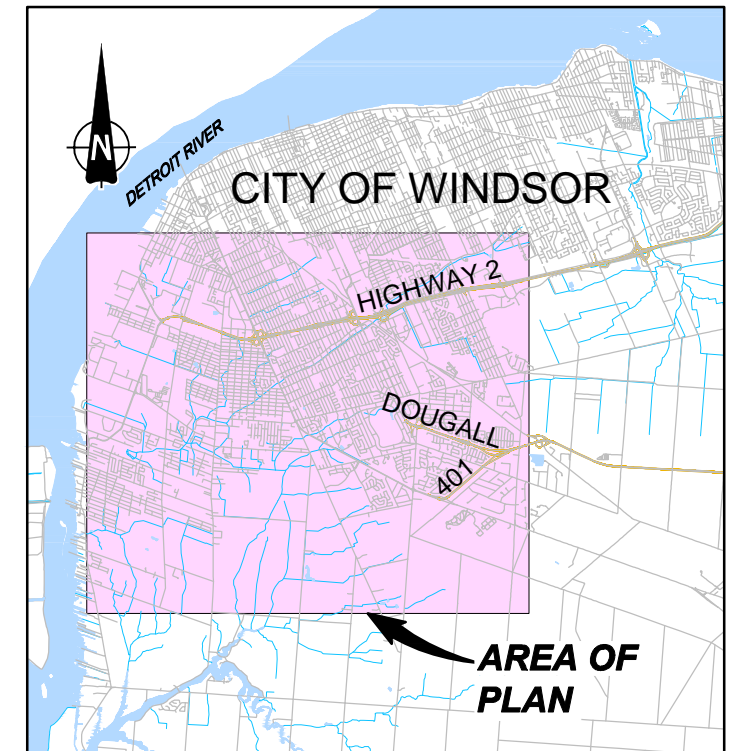
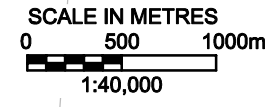
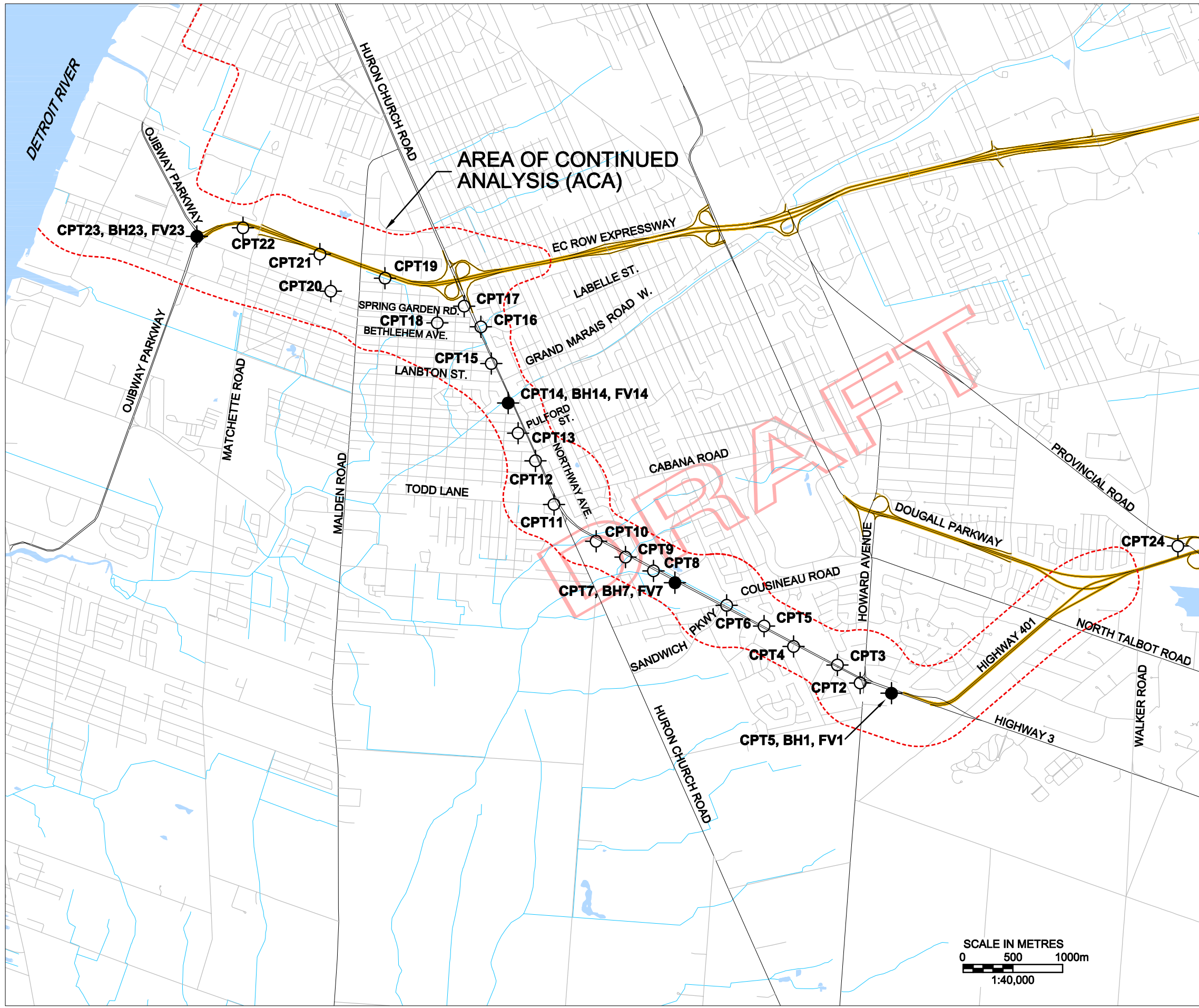
Detroit River International Crossing
Windsor, Ontario

Table with columns: Pavement Section, LOCATION (From, To), Northbound/Westbound (LDF, IF, Design Period (years), ESALs, Class), Southbound/Eastbound (LDF, IF, Design Period (years), ESALs, Class). Rows include sections like Huron Church Road, Talbot Road, Ojibway Parkway, and various crossings.

* - For consistency, Huron Church Rd/Talbot Rd runs North-South and all crossing roads run East-West





Drawing file: 051140003-R01001.dwg Dec 19, 2007 - 2:55pm



KEY PLAN

LEGEND

-  PIEZO CONE PENETRATION TEST
-  BOREHOLE, CONE PENETRATION TEST, FIELD VANE

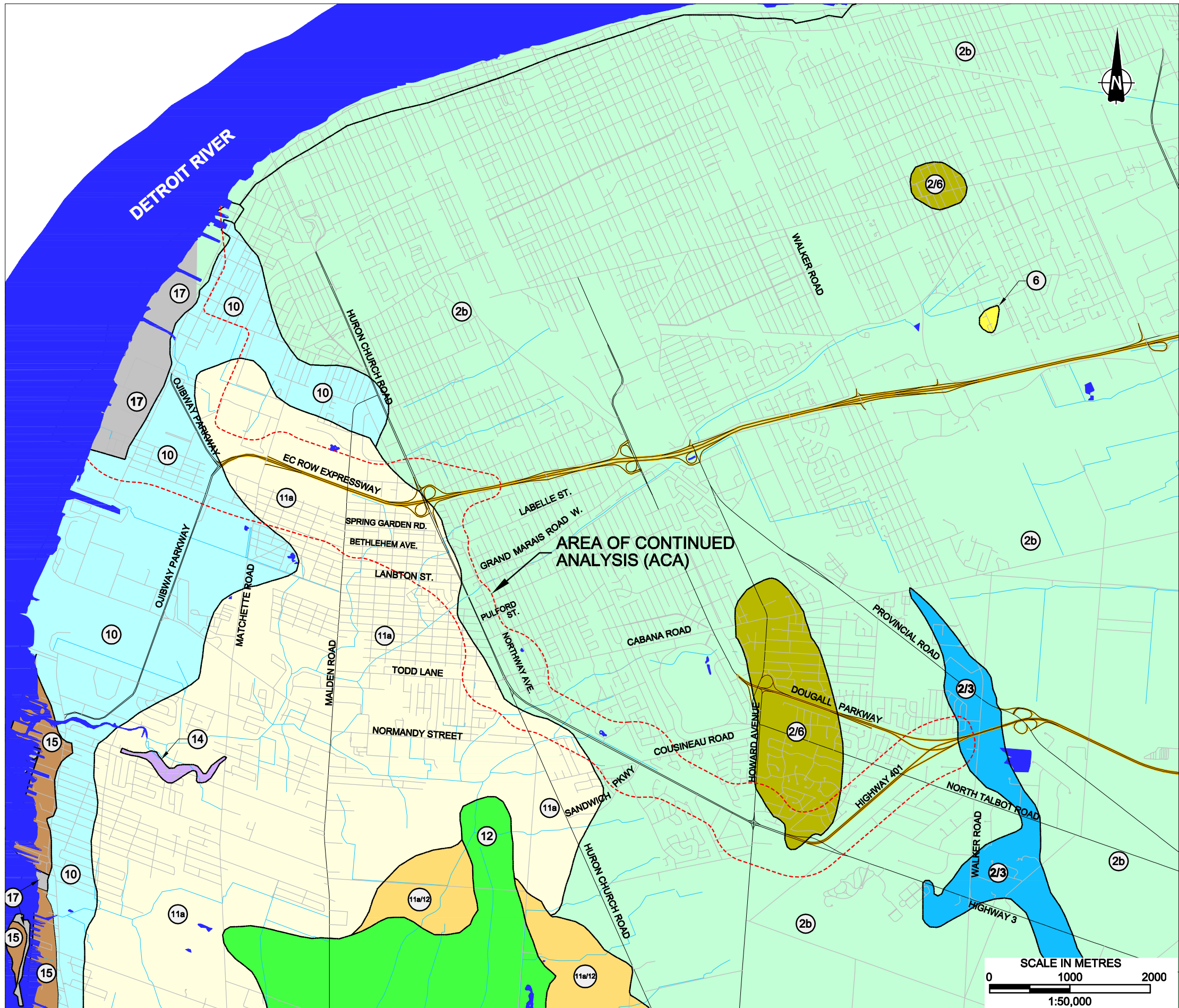
REFERENCE

DRAWING BASED ON CANMAP STREET FILES V2005.4

NOTES

THIS DRAWING IS SCHEMATIC ONLY AND IS TO BE READ IN CONJUNCTION WITH ACCOMPANYING TEXT.
ALL LOCATIONS ARE APPROXIMATE.

PROJECT				
DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO				
TITLE				
LOCATION PLAN				
 Golder Associates LONDON, ONTARIO	PROJECT No.	05-1140-003	FILE No.	051140003-R01001
	CADD	LMK	Nov. 15/07	SCALE AS SHOWN
	CHECK			REV.
FIGURE 1				



LEGEND

QUATERNARY GEOLOGY:

- 2b Clayey silt till.
- 2/3 Area of till (2) and glaciofluvial gravel or gravelly sand (3).
- 6 Glaciolacustrine: medium sand.
- 2/6 Area of till (2) with a thin and discontinuous cover of glaciolacustrine medium sand (6).
- 10 Lacustrine (Lake Rouge): gravel, gravelly sand.
- 11a Lacustrine sand.
 - a) Lake Rouge: medium sand.
- 11a/12 Lacustrine (Lake Rouge): area of medium sand on the knolls and silt in the depressions.
- 12 Lacustrine (Lake Rouge): silt, clay. May include some glaciolacustrine silt-clay (8).
- 14 Unsubdivided modern alluvium: silty loam, some organic matter.
- 15 Bogs, marshes and swamps: peat, muck, marl, loam.
- 17 Fill, concrete, other cultural features.

REFERENCE

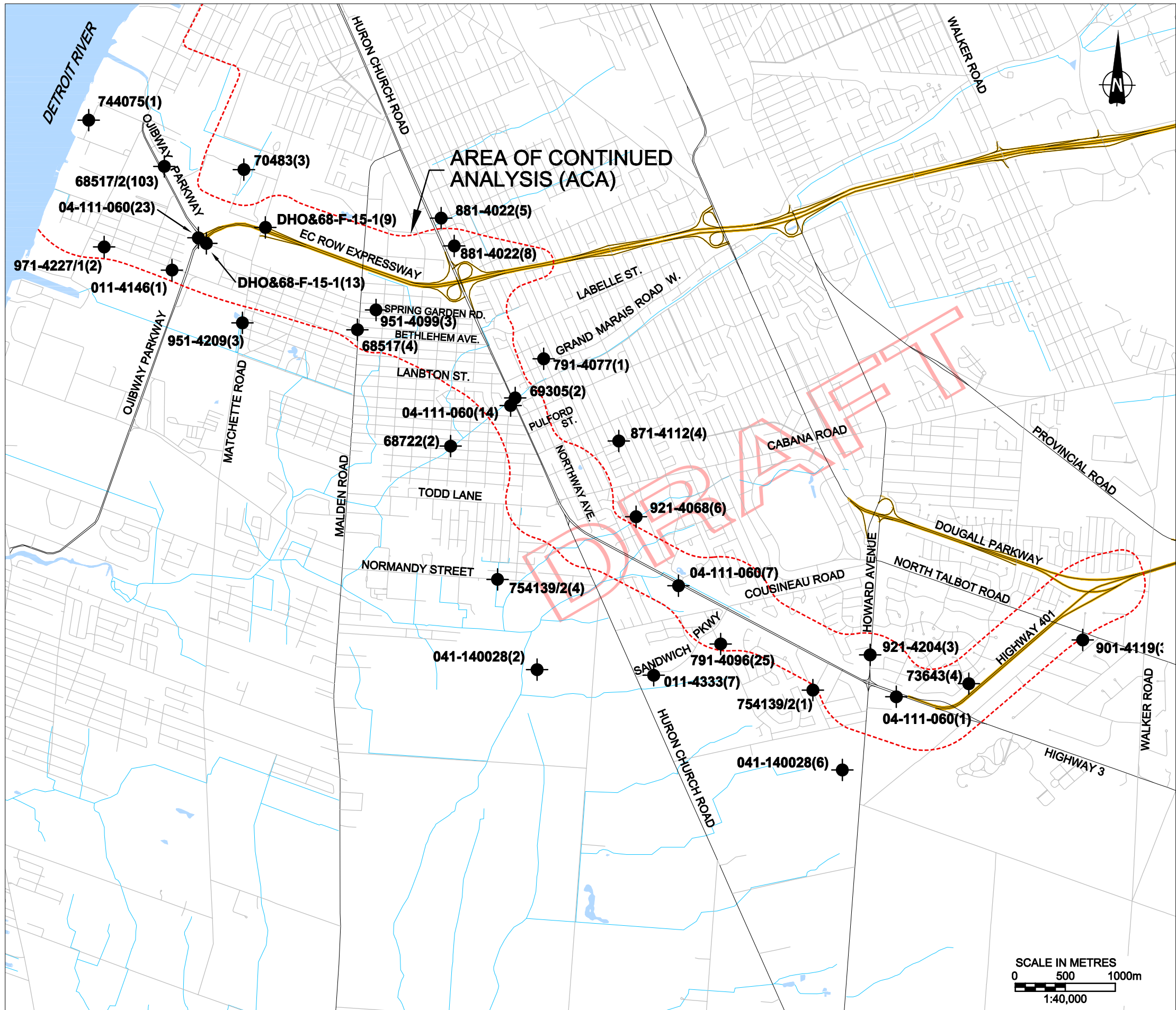
DRAWING BASED ON CANMAP STREET FILES V2005.4
 QUATERNARY GEOLOGY: Ontario Division of Mines (1972)
 "Quaternary Geology of the Windsor-Essex Area, Western Part",
 Map 749, Geol. Ser., Scale 1:50,000.

NOTES

THIS DRAWING IS SCHEMATIC ONLY AND IS TO BE
 READ IN CONJUNCTION WITH ACCOMPANYING TEXT.
 ALL LOCATIONS ARE APPROXIMATE.

PROJECT			
DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE			
QUATERNARY GEOLOGY			
PROJECT No.	05-1140-003	FILE No.	051140003-R01002
CADD	LMK	SCALE	AS SHOWN
CHECK		REV.	
FIGURE 2			





LEGEND

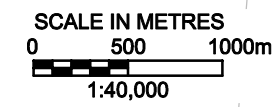
● BOREHOLE

REFERENCE

DRAWING BASED ON CANMAP STREET FILES V2005.4

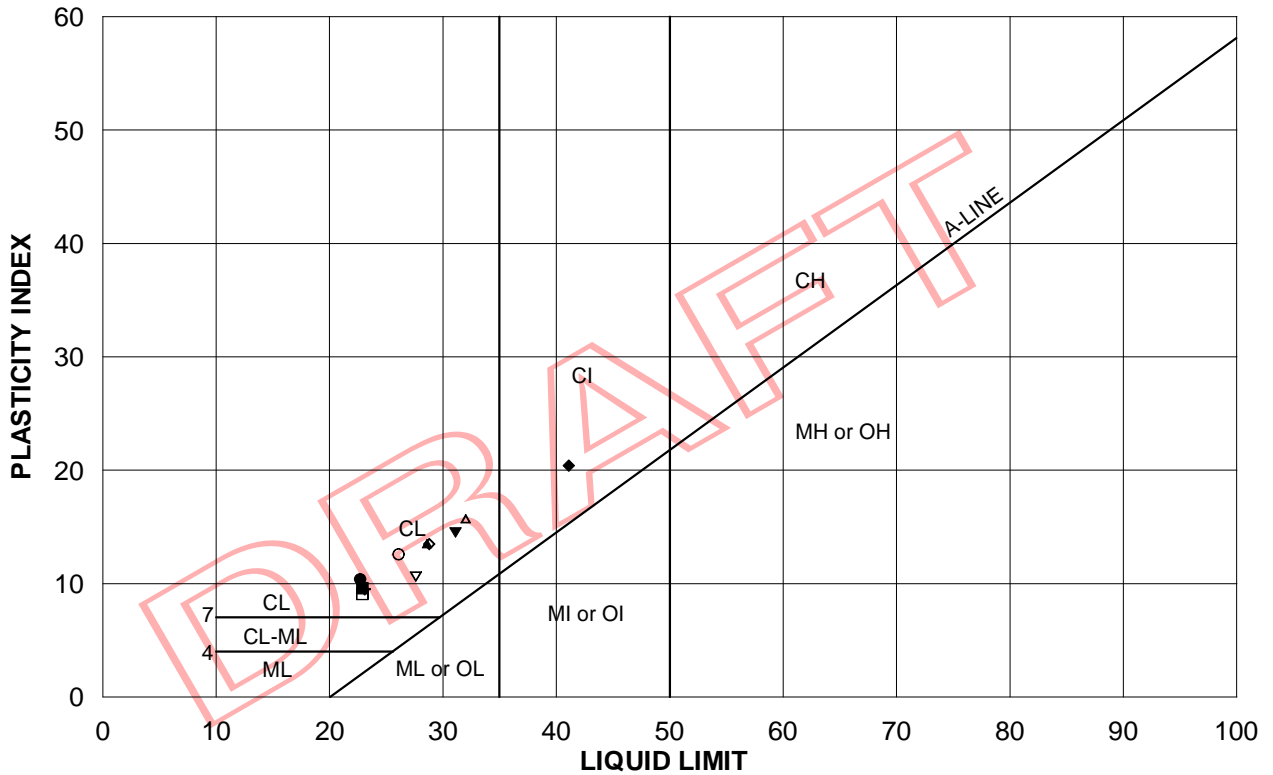
NOTES

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ALL LOCATIONS ARE APPROXIMATE.




PROJECT		DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO	
TITLE		SUBSURFACE DATABASE	
PROJECT No.	05-1140-003	FILE No.	051140003-R01003
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CHECK			REV.
			FIGURE 3

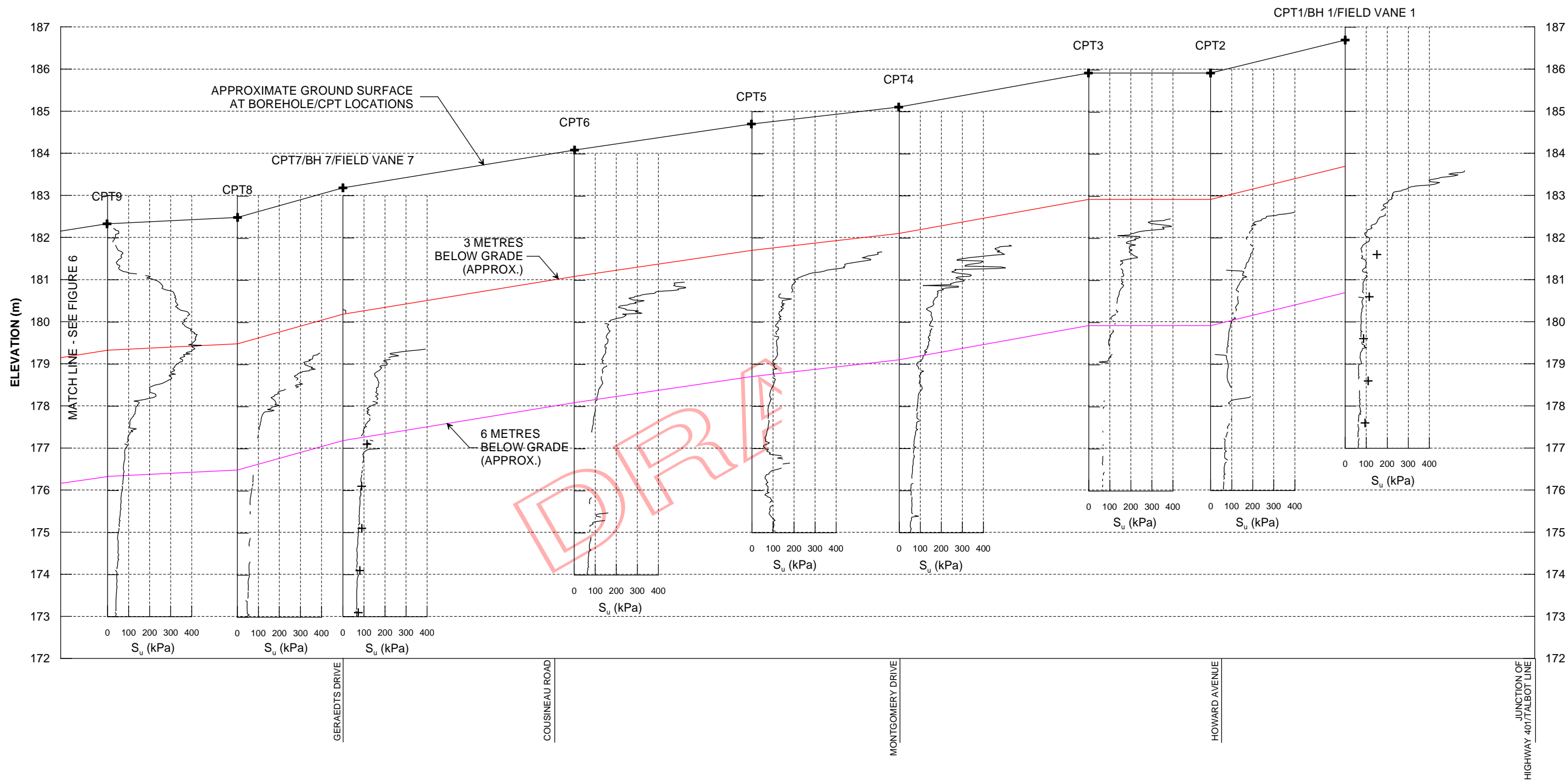




LEGEND (04-1111-060)

- Borehole 1, Sample 5
- Borehole 1, Sample 7
- Borehole 7, Sample 5
- Borehole 7, Sample 7
- ◇ Borehole 7, Sample 9
- ◆ Borehole 14, Sample 4
- △ Borehole 14, Sample 6
- ▲ Borehole 14, Sample 9
- ▽ Borehole 23, Sample 4
- ▼ Borehole 23, Sample 7
- + Borehole 23, Sample 9

PROJECT DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE PLASTICITY CHART			
PROJECT No. 05-1140-003-1		FILE No. 051140003-1-R01004	
		SCALE AS SHOWN REV. 0	
CADD	MEB	Nov. 15/07	
CHECK			
 Golder Associates			FIGURE 4



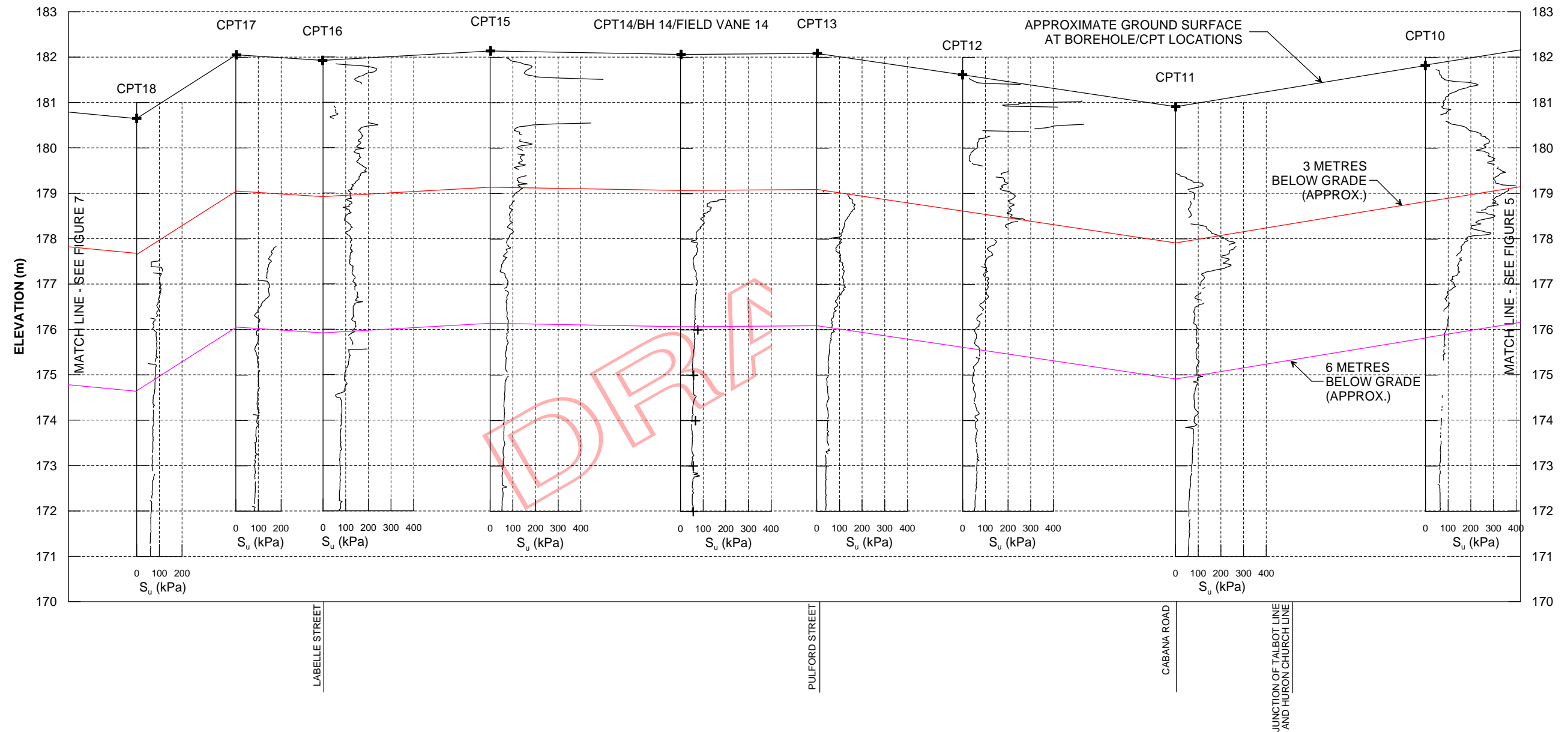
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- CPT DATA
- + FIELD VANE DATA

SCALE

VERTICAL SCALE - 1:100
 HORIZONTAL SCALE - 1:10,000

PROJECT DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE UNDRAINED SHEAR STRENGTH PROFILE			
PROJECT No.	05-1140-003-1	FILE No.	051140003-1-R01005
CADD	MEB	Nov. 15/07	SCALE AS SHOWN REV. 0
CHECK			
Golder Associates			FIGURE 5




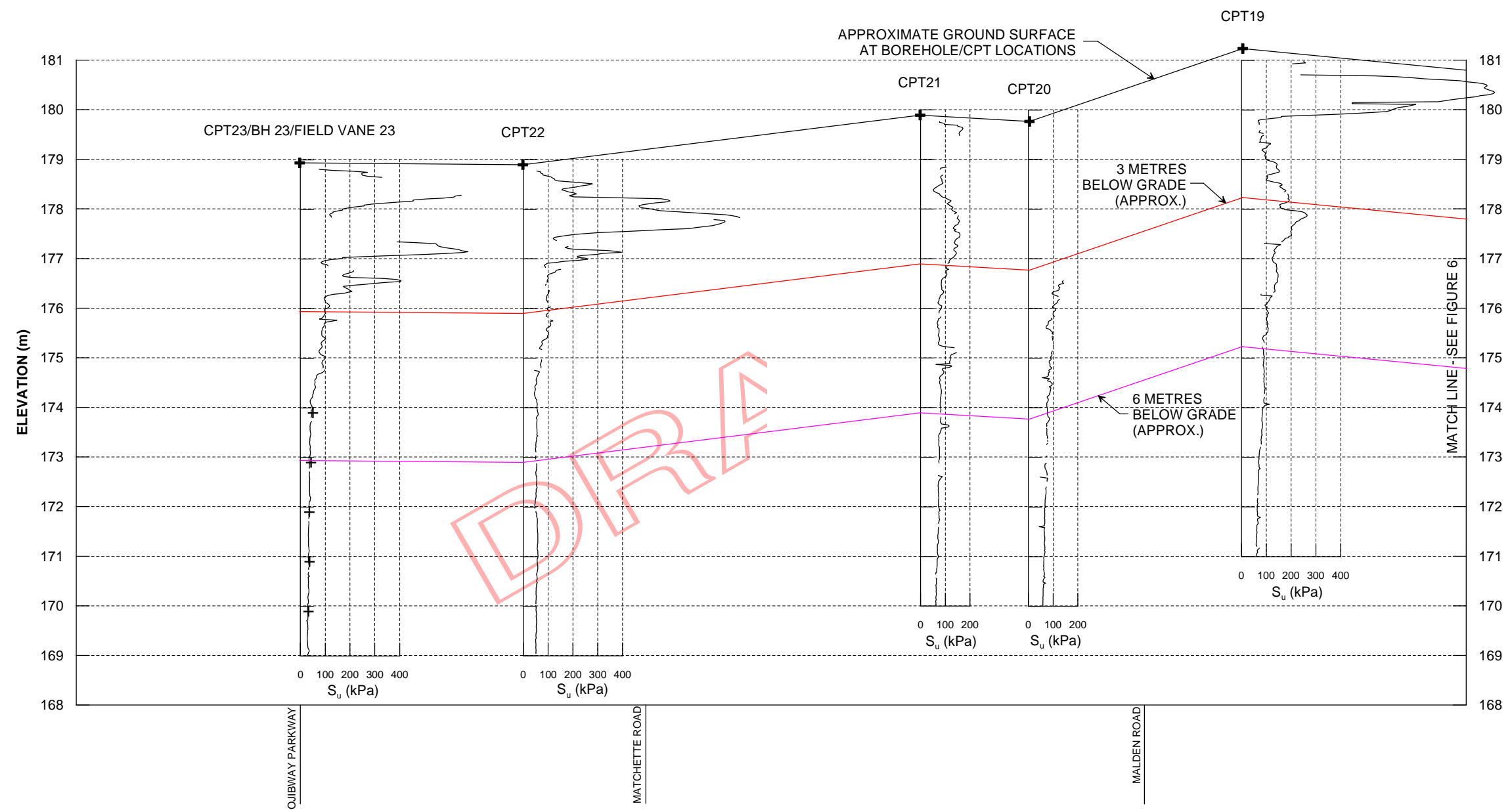
LEGEND (04-1111-060)

- CPT DATA
- + FIELD VANE DATA

SCALE

VERTICAL SCALE - 1:100
 HORIZONTAL SCALE - 1:10,000

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DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE			
UNDRAINED SHEAR STRENGTH PROFILE			
PROJECT No. 05-1140-003-1		FILE No. 051140003-1-R01006	
CADD	MEB	Nov. 15/07	SCALE AS SHOWN REV. 0
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			FIGURE 6



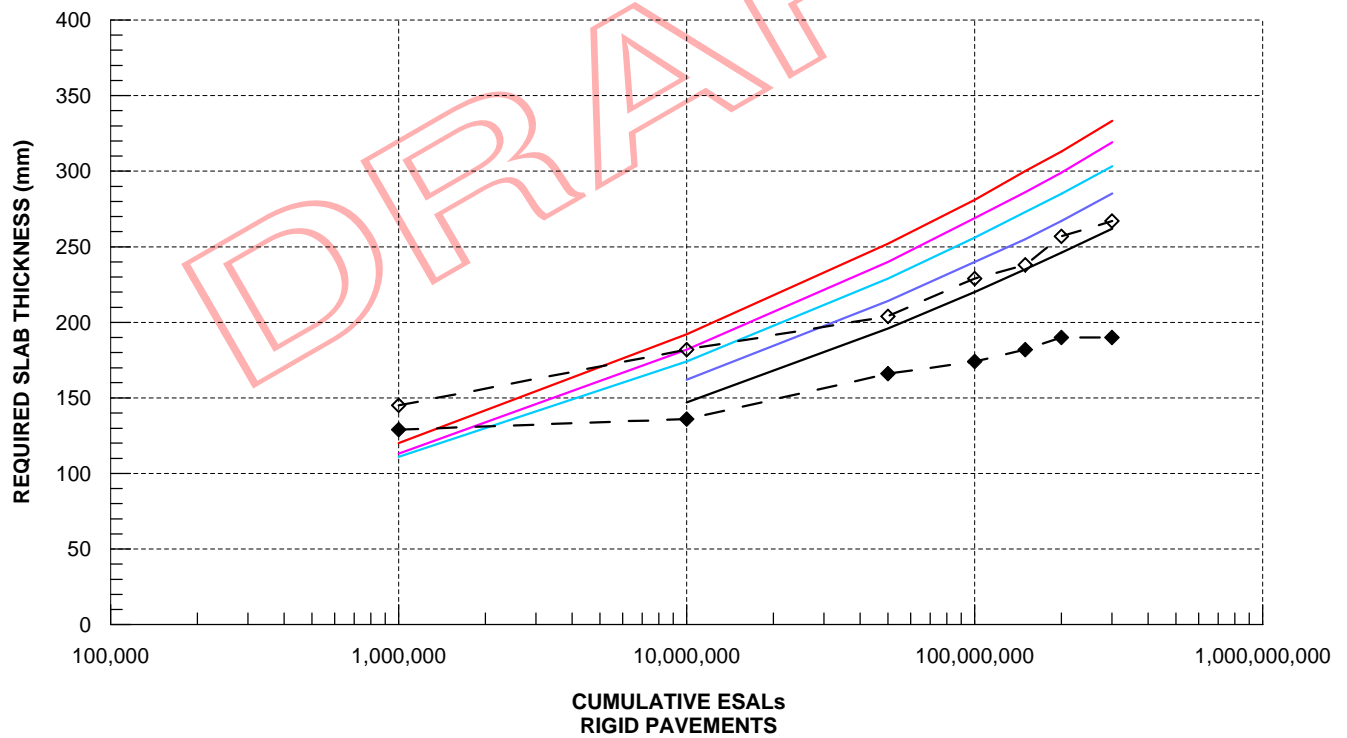
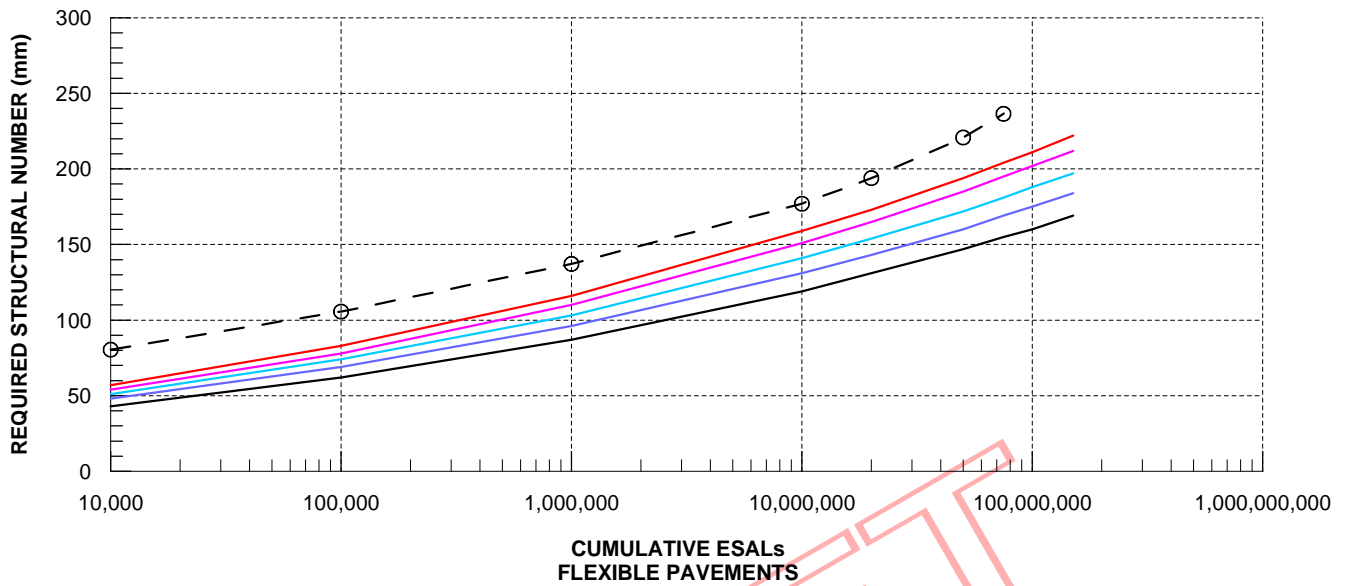
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- CPT DATA
- + FIELD VANE DATA

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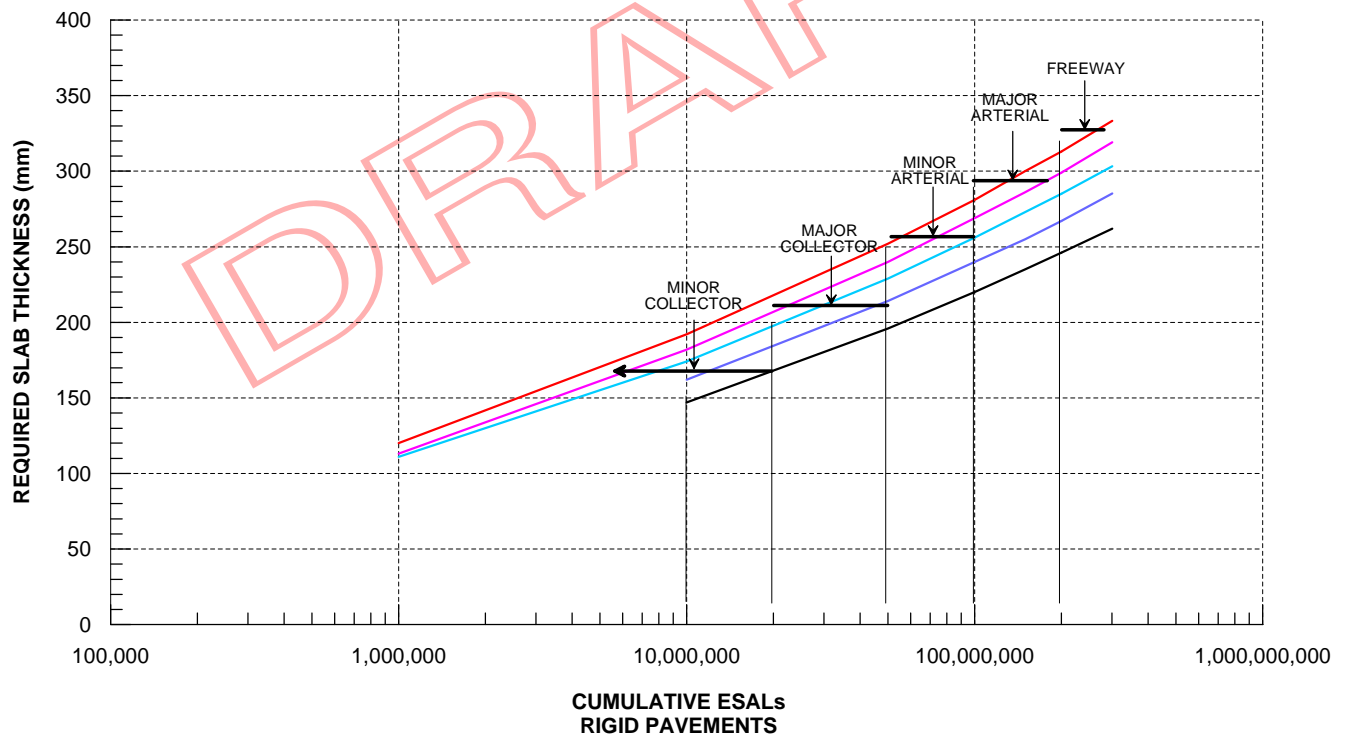
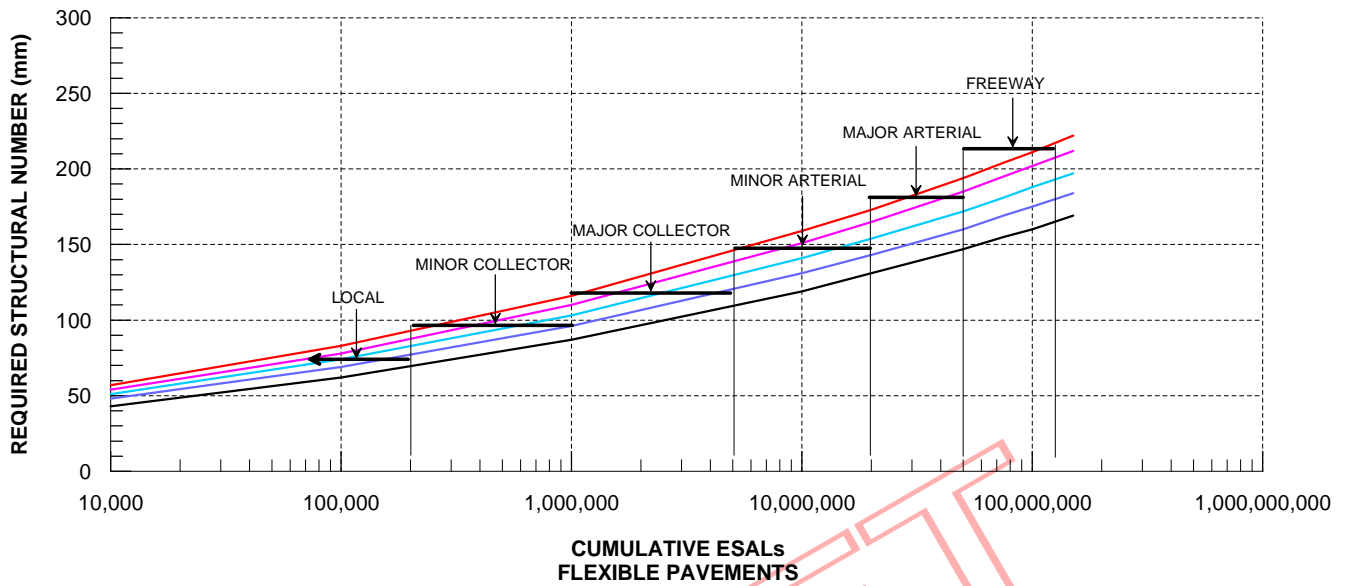
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PROJECT DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE UNDRAINED SHEAR STRENGTH PROFILE			
PROJECT No.	05-1140-003-1	FILE No.	051140003-1-R01007
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CHECK			
Golder Associates			FIGURE 7




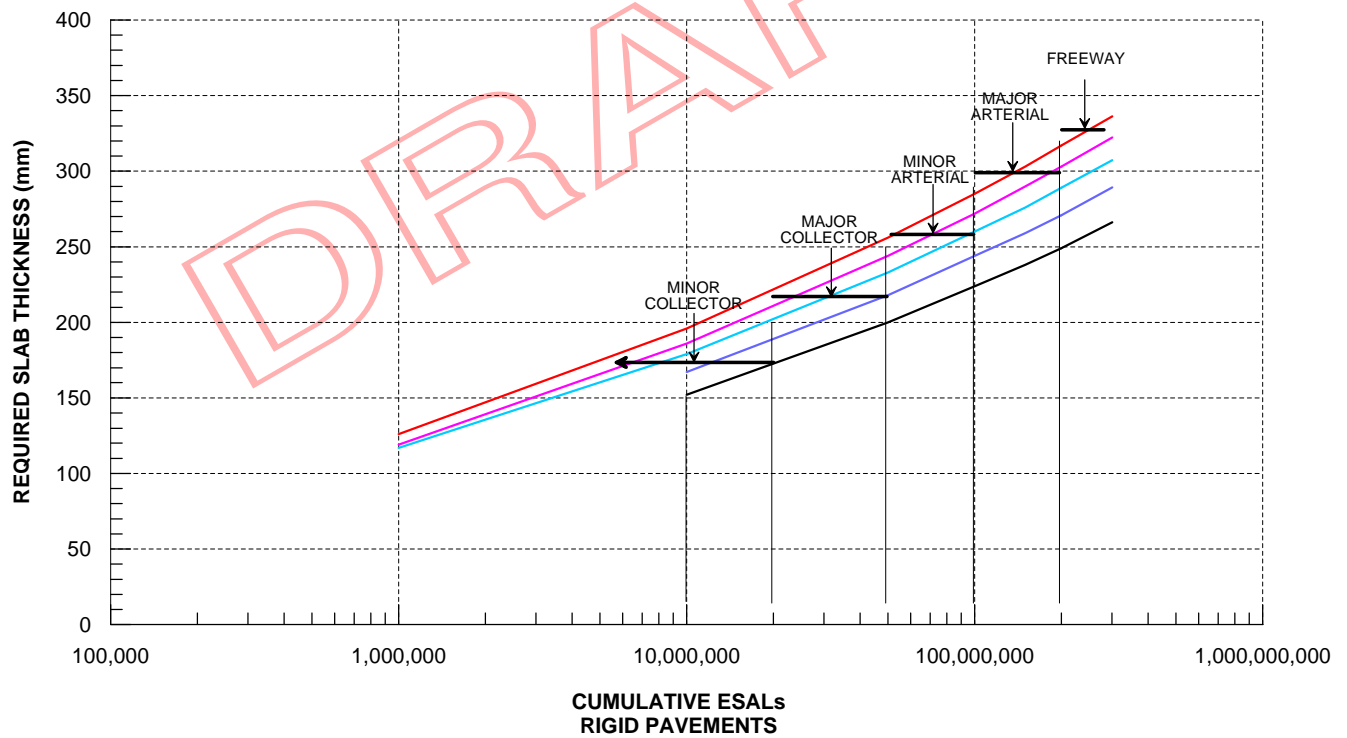
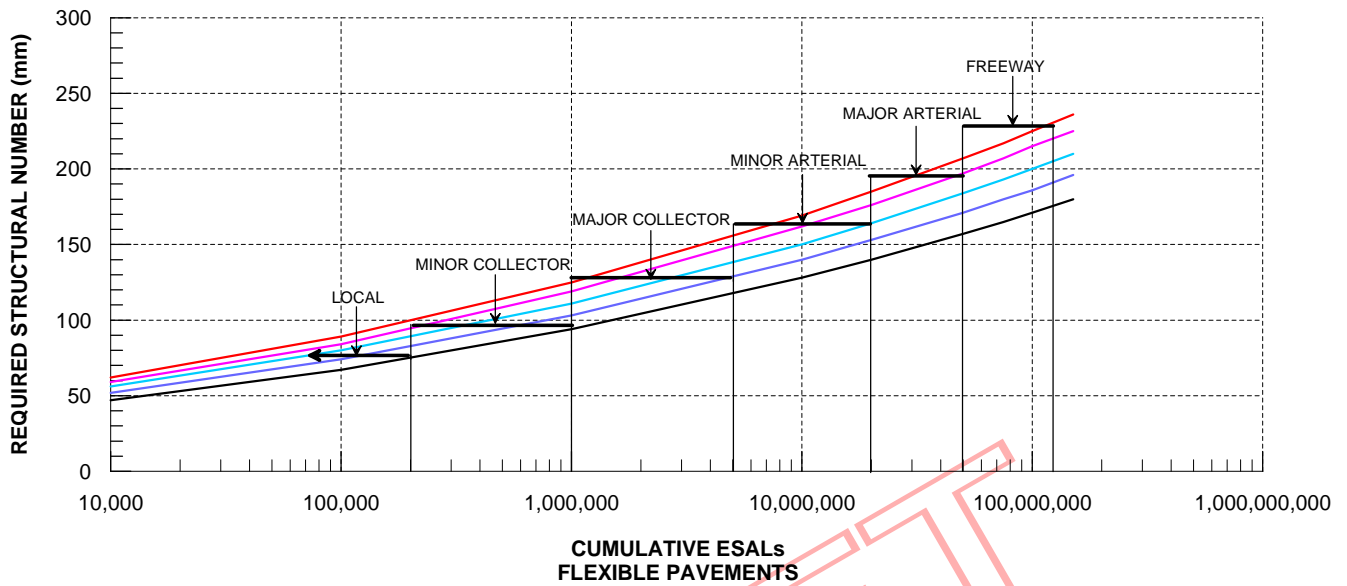
- AASHTO METHOD**
- R=50%, TSI=2.0 (Local & Minor Collector)
 - R=75%, TSI=2.0 (Major Collector)
 - R=85%, TSI=2.2 (Minor Arterial)
 - R=90%, TSI=2.5 (Major Arterial)
 - R=95%, TSI=2.5 (Freeway)
- ○ — ASPHALT INSTITUTE METHOD
- ◇ — PCA METHOD - NO TIED SHOULDER
- ◆ — PCA METHOD - WITH TIED SHOULDER

PROJECT			
DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE			
COMPARISON OF PAVEMENT DESIGN METHODS (using Subgrade Condition 1)			
PROJECT No. 05-1140-003-1		FILE No. 051140003-1-R01008	
CADD	MEB	Nov. 15/07	SCALE JUN 29-07/REV. 0
CHECK			
			FIGURE 8




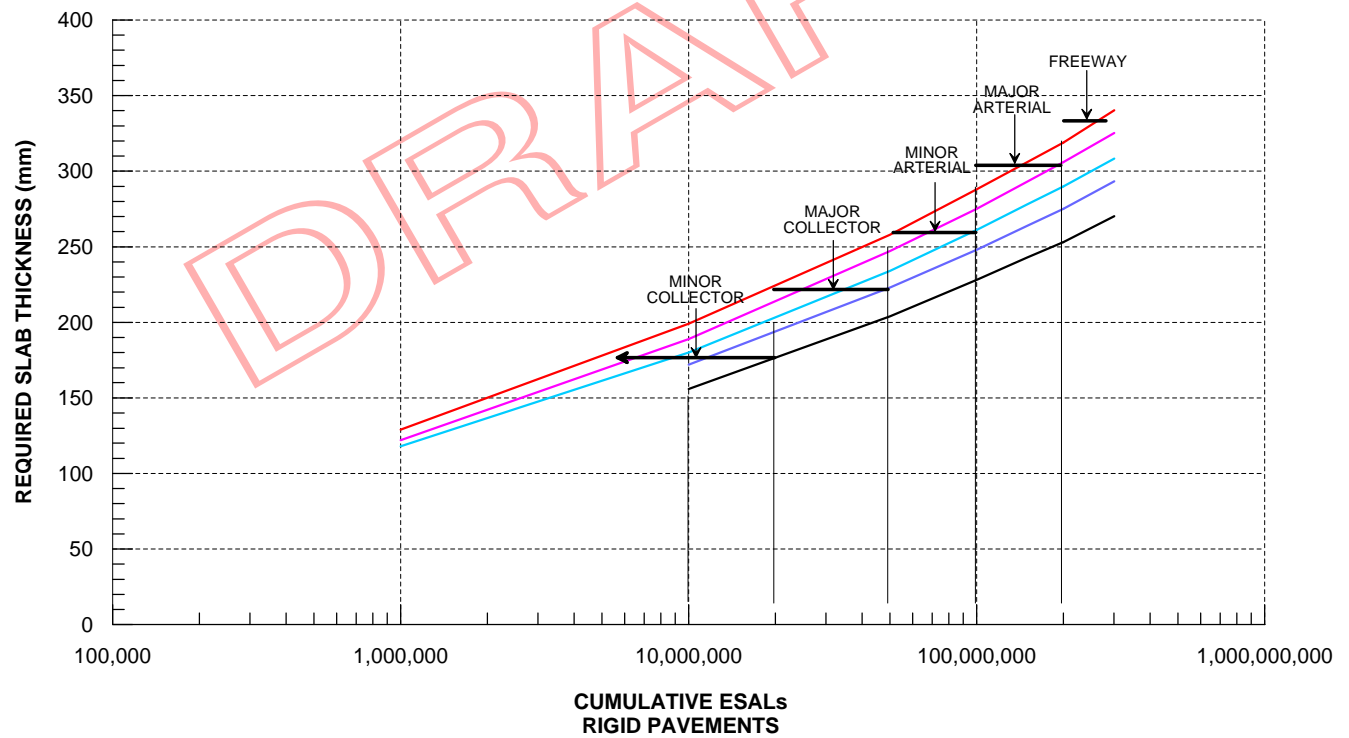
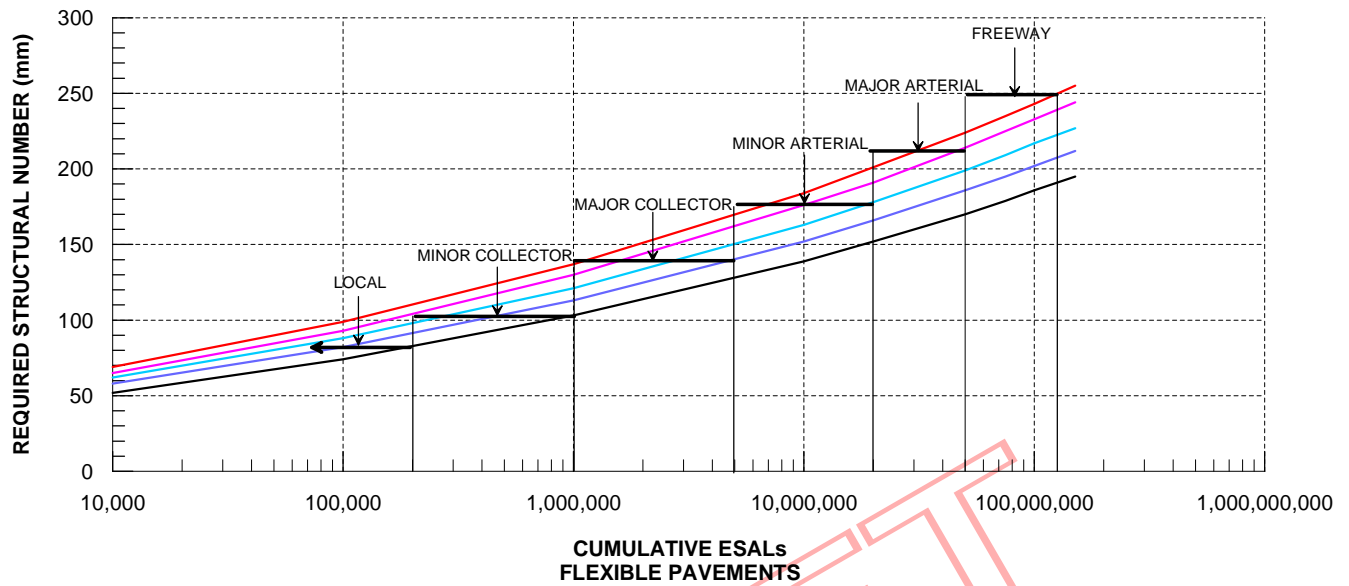
- R=50%, TSI=2.0 (Local & Minor Collector)
- R=75%, TSI=2.0 (Major Collector)
- R=85%, TSI=2.2 (Minor Arterial)
- R=90%, TSI=2.5 (Major Arterial)
- R=95%, TSI=2.5 (Freeway)

PROJECT			
DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE			
PAVEMENT STRUCTURAL REQUIREMENTS VERSUS CUMULATIVE ESALs SUBGRADE CONDITION 1			
PROJECT No. 05-1140-003-1		FILE No. 051140003-1-R01009	
		SCALE JUN 29-07/REV. 0	
CADD	MEB	Nov. 15/07	
CHECK			
 Golder Associates		FIGURE 9	




- R=50%, TSI=2.0 (Local & Minor Collector)
- R=75%, TSI=2.0 (Major Collector)
- R=85%, TSI=2.2 (Minor Arterial)
- R=90%, TSI=2.5 (Major Arterial)
- R=95%, TSI=2.5 (Freeway)

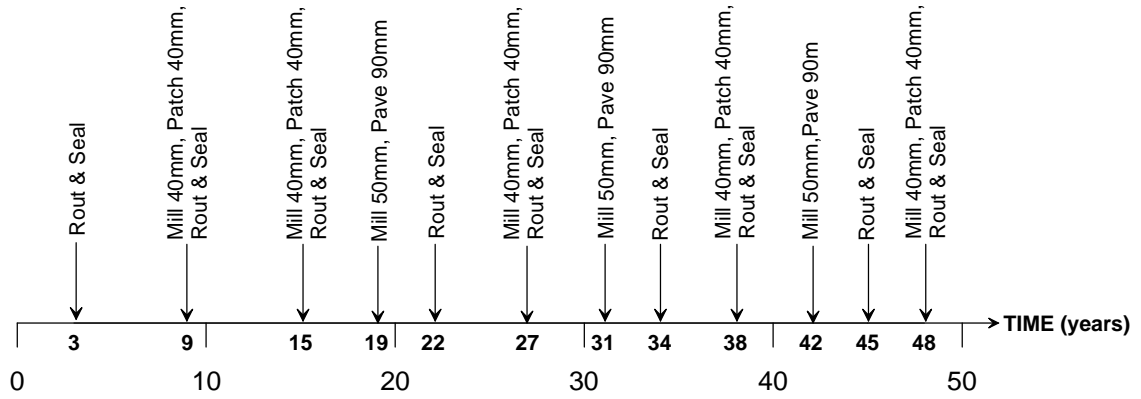
PROJECT			
DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE			
PAVEMENT STRUCTURAL REQUIREMENTS VERSUS CUMULATIVE ESALs SUBGRADE CONDITION 2			
PROJECT No. 05-1140-003-1		FILE No. 051140003-1-R01010	
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CADD	MEB	Nov. 15/07	
CHECK			
 Golder Associates		FIGURE 10	



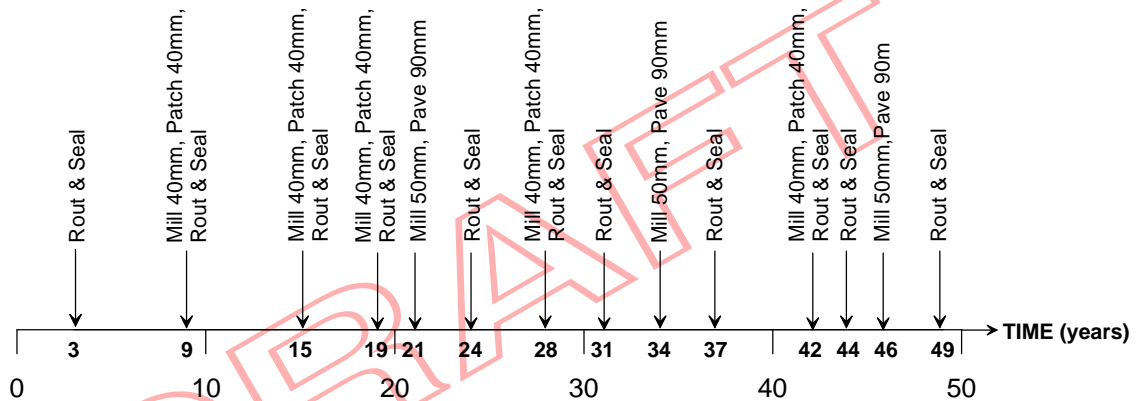
- R=50%, TSI=2.0 (Local & Minor Collector)
- R=75%, TSI=2.0 (Major Collector)
- R=85%, TSI=2.2 (Minor Arterial)
- R=90%, TSI=2.5 (Major Arterial)
- R=95%, TSI=2.5 (Freeway)

PROJECT			
DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE			
PAVEMENT STRUCTURAL REQUIREMENTS VERSUS CUMULATIVE ESALs SUBGRADE CONDITION 3			
PROJECT No. 05-1140-003-1		FILE No. 051140003-1-R01011	
		SCALE JUN 29-07/REV. 0	
CADD	MEB	Nov. 15/07	
CHECK			
			FIGURE 11

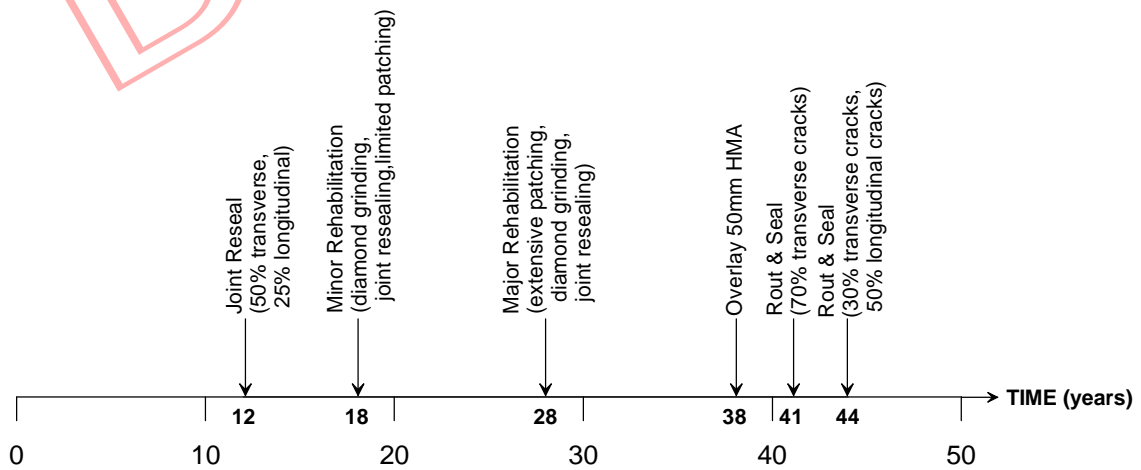
LIGHT AND MEDIUM DUTY FLEXIBLE PAVEMENTS



HEAVY DUTY FLEXIBLE PAVEMENTS (FREEWAY)



RIGID PAVEMENTS



PROJECT				DETROIT RIVER INTERNATIONAL CROSSING WINDSOR, ONTARIO			
TITLE				SUMMARY OF LIFE CYCLE COST STRATEGIES			
PROJECT No.		05-1140-003-1		FILE No.		051140003-1-R01012	
CADD		MEB		Nov. 15/07		SCALE AS SHOWN REV. 0	
CHECK						FIGURE 12	



APPENDIX A
DRAFT INTERIM REPORT - PAVEMENT ENGINEERING
DETROIT RIVER INTERNATIONAL CROSSING

APPENDIX A

DRAFT INTERIM REPORT ON

**PAVEMENTS ENGINEERING
DETROIT RIVER INTERNATIONAL CROSSING
WINDSOR, ONTARIO**

DRAFT

Submitted to:

URS Canada Inc.
Consulting Engineers
75 Commerce Valley Drive East
Markham, Ontario
L3T 7N9

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August 2005

05-1140-003

INTRODUCTION

This report summarizes the background information on the subsurface conditions pertaining to pavement engineering in the Windsor area in support of the Environmental Assessment being undertaken for the proposed Detroit River International Crossing transportation corridors. A group of consulting firms, lead by URS Corporation, is completing the study under contract to the Ministry of Transportation, Ontario. The Ministry of Transportation, Ontario is one of the four principal members of an international partnership organized to study potential transportation corridors to permit more efficient transit of travelers and goods between the highway systems in the Windsor and Detroit metropolitan areas. Other members of the partnership include Transport Canada (TC), the United States Federal Highway Administration (US FHWA), and the State of Michigan Department of Transportation (MDOT).

Proposed Project

It is understood that current and projected cross-border traffic conditions in the greater Detroit and Windsor areas are such that a new river crossing is needed. The project will generally consist of either upgrading of existing transportation routes or building of new roadways to connect Provincial Highway 401 in Ontario with the interstate highway system surrounding Detroit. This connection will require either a bridge or tunnel crossing of the Detroit River and has the potential to include multiple overpass or underpass bridge structures associated with interchanges or grade separations along the connecting routes. Consideration may also be given to roadway sections lowered between retaining structures to achieve grade separations and which may minimize the effects of the corridor on surrounding areas. At this time, the details of the crossing facilities, locations, and corridors to and from the regional highways have not been established.

The general study area is illustrated on Figure 1.

Scope of Report

The scope of this report is to compile, review and provide a summary of the background information related to the subsurface conditions in the context of feasibility evaluation, route selection, and environmental assessment for the pavement engineering component of the proposed project. No subsurface investigations were conducted for this report and all subsurface data has been gathered from existing internal and published sources.

SUBSURFACE DATA REVIEW

Existing subsurface data was compiled and used to develop the soil classification maps described in this report. Information was gathered from Ministry of Transportation Ontario files (through the GEOCREs system), Golder Associates project files, Ontario Ministry of Natural Resources (MNR), and published papers and books. These data were used to assist in building an electronic database of subsurface information, and map the soil classification throughout the Windsor area at the following approximate intervals of depth: 0.3, 1.5, 3.0, and 6.0 metres below existing ground surface at the borehole locations.

Subsurface data existing in Golder Associates and the Ministry of Transportation Ontario (MTO) GEOCREs library files for projects completed in Windsor and surrounding areas over the last 40 years were reviewed for this project using the following criteria:

- Available boreholes and test pits drilled or excavated from ground surface to a minimum depth of 3 metres, and
- One representative borehole or test pit per minimum area of 0.25 square kilometres, where available, within the City of Windsor and Towns of LaSalle and Tecumseh.

The resultant data base is provided in Appendix A-I.

GEOLOGY OF THE WINDSOR AREA

The subsurface conditions in the Windsor area are characterized by regionally extensive, flat-lying soil including:

- Surface layers of miscellaneous fill materials at localized areas, associated with industrial and urban growth,
- Native deposits of sand and silt present at or near the surface in some locations, particularly in the west end of the City of Windsor and Town of LaSalle,
- Beneath the sand, where present, extensive deposits of clayey silt to silty clay,
- Bedrock throughout the study area is generally encountered at depths of 20 to 35 metres.

Figure 2 illustrates the general surficial sedimentary geology of the study area based on geologic interpretation of widely-spaced sample locations and an understanding of geomorphologic

processes. When reviewing this figure, it should be noted that only the natural sediments encountered closest to the surface are identified, and that other sediments will exist beneath these.

Sedimentary Geology

The study area is located in the physiographic region of Southwestern Ontario known as the St. Clair Clay Plains¹. Within this region, Essex County and the southwestern part of Kent County are normally discussed as a subregion known as the Essex Clay Plain. The clay plain was deposited during the retreat of the ice sheets (late Pleisocene Era) when a series of glacial lakes inundated the area. In general, the ice sheets deposited till in the area of Windsor and Detroit. Depending on the locations of the glacial ice sheets and depths of water in the ice-contact glacial lakes, the till may have been directly deposited at the contact between the ice sheet and the bedrock or, as the lake levels rose and the ice sheets retreated and floated, the soil and rock debris within and at the base of the ice may have been deposited through the lake water (lacustrine). Glacial till, in its common usage, often indicates a very dense or hard composition resulting from consolidation and densification under the weight of the ice sheet. The mineral soil particles typically have a distribution of grain sizes ranging from cobbles to clay. However, in many areas of Windsor and Detroit, the soils described as “glacial till” were deposited through water and have a softer consistency as a result. A large end moraine of glacial till is mapped in the area of Windsor-Detroit, generally trending northwest to southeast near the outlet of Lake St. Clair as illustrated throughout Essex County near the terminus of Provincial Highway 401. In other areas, the lacustrine deposits overlie the hard glacial till. The major clay stratum, typically ranging in thickness from about 20 metres to 30 metres, exhibits a till-like structure exemplified by a random distribution of coarser particles within the primarily fine-grained silt and clay deposit (this type of deposit is also called “diamict”).

Surficial layers or pockets of more typical layered lacustrine (lake deposited) silty clay, silt, or sand may be encountered overlying the extensive stratum of “till-like” silty clay. Silt and sand deposits, on the order of 2 to 4 metres thick, are often found near the ground surface in areas near the western side of Windsor and the southwestern limits of the study area.

General Subsurface Conditions

The following section describes the general subsurface conditions in the Windsor and surrounding areas, at intervals of depths of approximately 0.3, 1.5, 3.0 and 6.0 metres below existing ground surface. The information was gathered from previous geotechnical investigations by Golder Associates and others, using the criteria defined in Section 2.0. The following descriptions of the subsurface conditions are generalized and should not be interpreted to be

¹ Chapman, L. J., The Physiography of Southern Ontario, 1984.

exact, nor used in the design of the pavement structure of a proposed corridor. Further detailed site specific geotechnical investigation is required once a corridor has been selected.

Beneath the topsoil and/or surficial fill materials in west Windsor and LaSalle, at a depth of approximately 0.3 metres below existing ground surface, the subsurface conditions consists of fine grained granular material, such as sand, silty sand and sandy silt. In central and east Windsor, and Tecumseh, the subsurface conditions at a depth of approximately 0.3 metres below existing ground surface generally consist of a silty clay to clayey silt, with localized areas of surficial granular soil. Figure 3 illustrates the subsurface soil conditions at an approximate depth of 0.3 metres below existing ground surface.

In general, in west Windsor and LaSalle, at a depth of approximately 1.2 to 1.5 metres below the existing ground surface, the subsurface conditions consists of a clayey silt to silty clay, with areas of granular deposits, consisting of sand, silty sand and sandy silt. In central and east Windsor and Tecumseh, the subsurface conditions at a depth of approximately 1.5 metres below existing ground surface generally consist of a clayey silt to a silty clay. Figure 4 illustrates the subsurface soil conditions at a depth of approximately 1.5 metres below existing ground surface.

At an approximate depth of 3.0 metres below existing ground surface, the subsurface conditions in the Windsor and surrounding areas generally consist of clayey silt to silty clay. However, localized areas of granular soil exist at a depth of approximately 3.0 metres below existing ground surface. Figure 5 illustrates the subsurface soil conditions at a depth of approximately 3.0 metres below existing ground surface.

At an approximate depth of 6.0 metres below existing ground surface, the subsurface conditions in the Windsor and surrounding areas generally consist of clayey silt to silty clay. Fine-grained granular material was encountered in previous investigations at a depth of about 6.0 metres below existing ground surface in the northeast corner of the study area. Figure 6 illustrates the subsurface soil conditions at a depth of approximately 6.0 metres below existing ground surface.

SOIL CHARACTERISTICS INFLUENCING PAVEMENT DESIGN

The subsurface conditions as described in this report have an important influence on the design of the pavement structure for the proposed transportation corridor. Two main factors affecting roadway pavement design are:

- i) frost susceptibility of the subgrade soil which influences the design of the pavement structure component thicknesses, and
- ii) soil erodibility which influences the design of embankment surfacing and drainage ditches that may be proposed in the design of a proposed corridor.

The following comments are general and should not be used for actual design of the roadway cross-section along a proposed corridor.

Frost Susceptibility

The generalized subsurface conditions at a depth of approximately 1.2 to 1.5 metres below existing ground surface, typical subgrade depths, have been categorized into categories of frost susceptibility, low, medium and high. At a depth of about 1.2 to 1.5 metres below existing ground surface, the subsurface conditions generally consist of clayey silt in the west end of Windsor, which is considered to have medium frost susceptibility. The west end of Windsor has areas of silty clay, which are considered to have low susceptibility to frost. Figure 7 illustrates the frost susceptibility of the soils at a depth of approximately 1.2 to 1.5 metres below existing ground surface in the Windsor and surrounding area.

Soil Erodibility

It is anticipated that drainage ditches and embankments may be incorporated in the design of the proposed corridor. As previously indicated, the subsurface conditions in Windsor and surrounding area mainly consist of a silty clay at an approximate depth of 1.2 to 1.5 metres below existing ground surface. The areas of silty clay and localized areas of sand and silty sand are considered to be slightly erodible. However, some localized areas of clayey silt and sandy silt exist, and these are considered to be moderately erodible. Figure 8 illustrates the soil erodibility of the Windsor and surrounding area at a depth of approximately 1.2 to 1.5 metres below existing ground surface.

ADDITIONAL GEOTECHNICAL WORK

This interim report has been prepared on the basis of existing background information available through a limited number of resources. The work completed for this report is not sufficient to permit preliminary or final design and must be considered within the context of feasibility study and environmental assessment. Preliminary and final design, once a preferred route or routes have been selected, will require site-specific subsurface explorations, testing, analysis, and reporting.

GOLDER ASSOCIATES LTD.

André Bom, M.A.Sc., EIT

Philip R. Bedell, P.Eng.
Principal

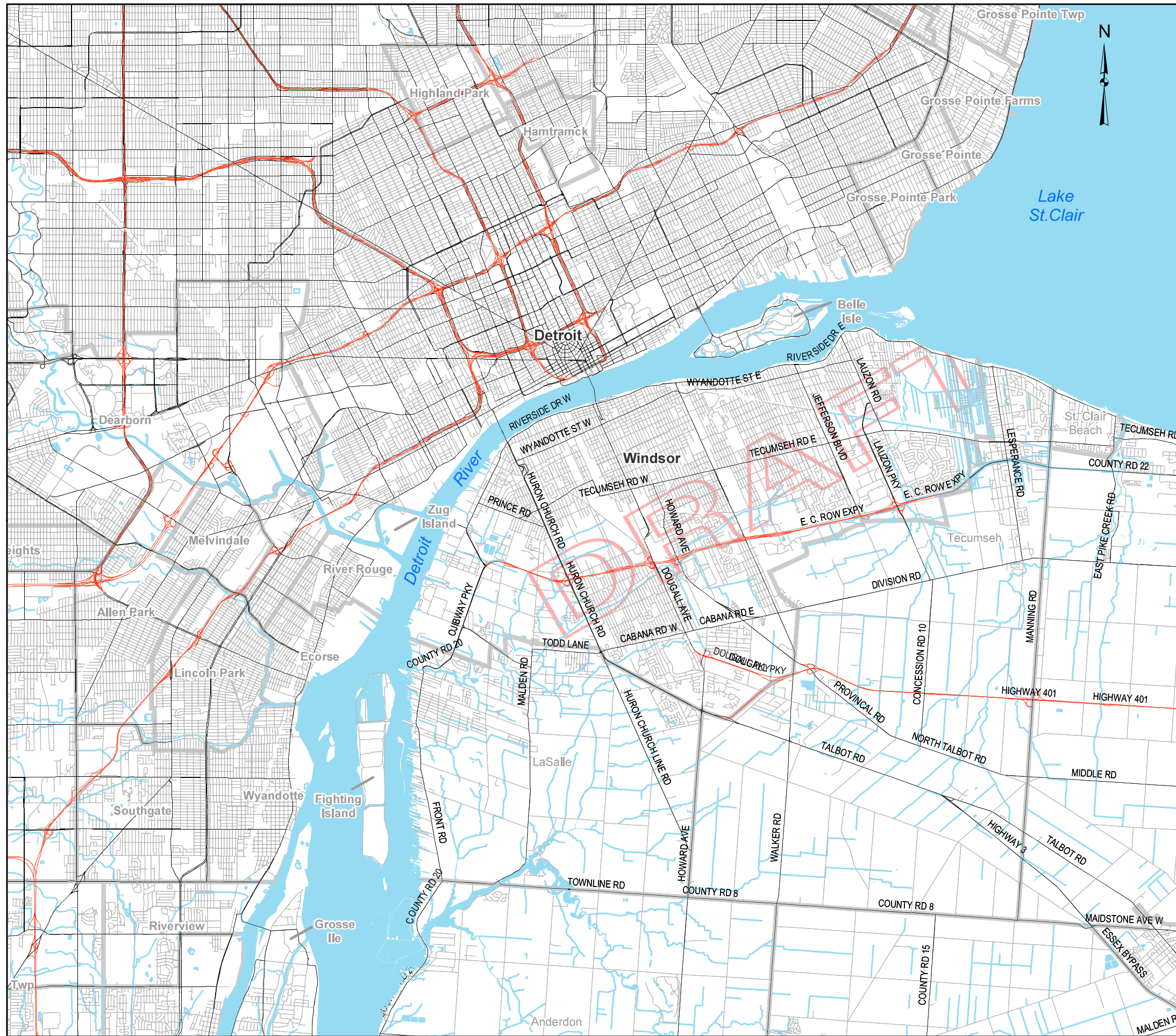
Attachments

Figures A-1 to A-8

Appendix A-1

N:\ACTIVE\PROJECTS - OTHER OFFICES\WINDSOR\2005\05-1140-003 URS - PAVEMENTS\REPORTS\05-1140-003-1\UPDATED DRAFT - DEC 17 07\APPENDIX A - INTERIM RPT\RPT-DRAFT-050711-INTERIM PAVEMENT REPORT-AB.DOC

G:\Projects\2005\05-1140-003_DRIC_Pavements\GIS\MXDs\Draft\July2005_Edits\FIG1_LOCATION_MAP.mxd



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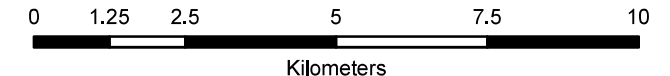
- Expressway
- Major Road
- Local Road
- Water
- Municipal Boundary

NOTE: Figure to be read in conjunction with accompanying report.

DRAFT

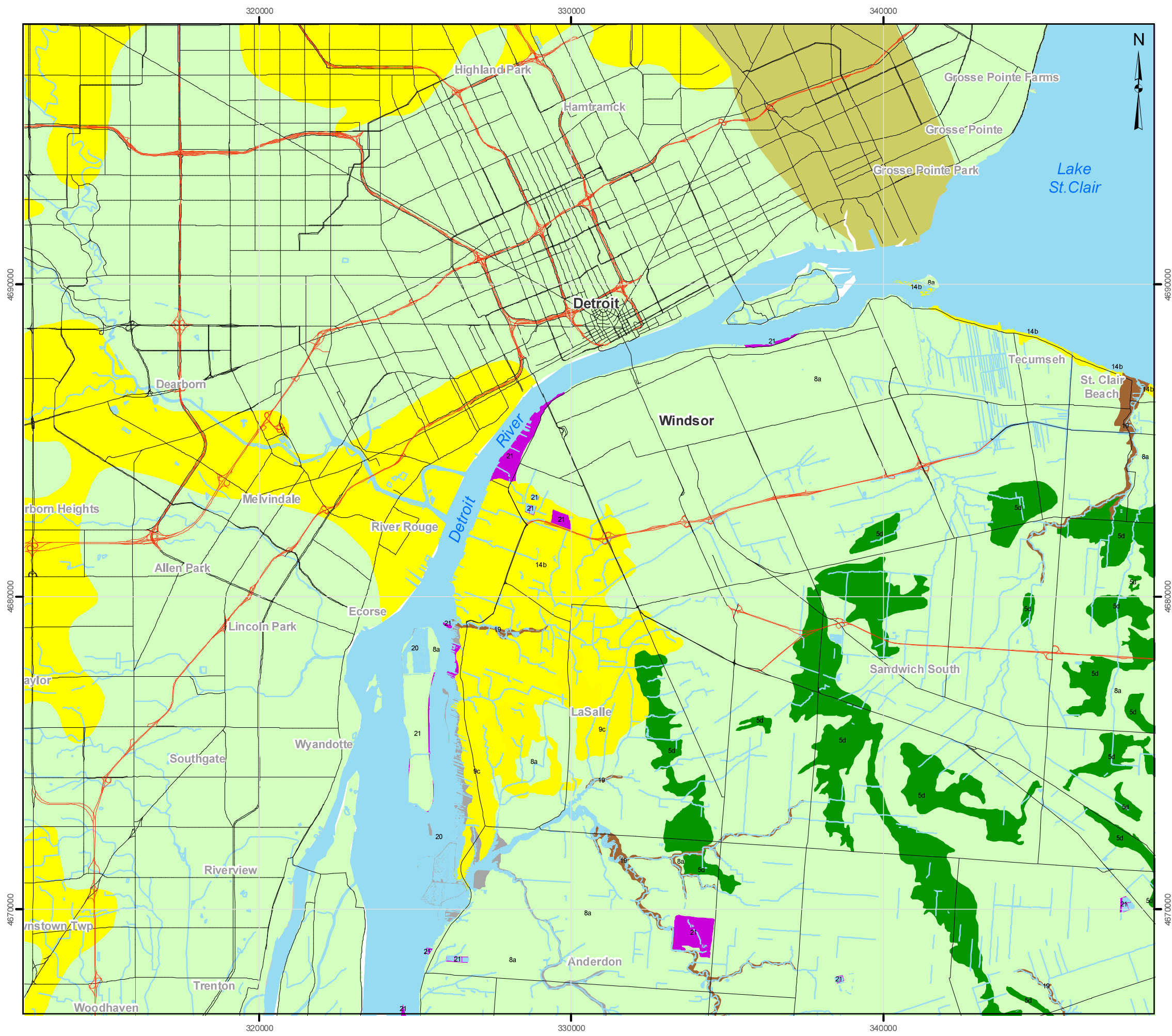


REFERENCE
 Base Data - MNR NRVIS, obtained 2004, CANMAP v7.3 2003
 Produced by Golder Associates Ltd under licence from Ontario Ministry of Natural Resources, © Queens Printer 2005
 Datum: NAD 83 Projection: UTM Zone 17N



PROJECT		ROUTE PLANNING	
		DETROIT RIVER INTERNATIONAL CROSSING	
		CANADIAN SIDE	
TITLE			
SITE LOCATION PLAN			
 Golder Associates Mississauga, Ontario	PROJECT No.	05-1140-003	SCALE 1:125,000
	DESIGN	AW 16 Feb. 2005	REV. 0
	GIS	CC 13 Jul. 2005	FIGURE: 1
	CHECK	AB 14 Jul. 2005	
	REVIEW		

G:\Projects\2005\05-1140-003_DRIC_Pavements\GIS\MXDs\Draft\July2005_Edits\FIG2_QUAT_GEOLOGY.mxd



LEGEND

Quaternary Geology - Ontario

- 5d: Glaciolacustrine-derived silty to clayey till
- 8a: Massive to well laminated clayey silt to silty clay diamict
- 9c: Foreshore-basinal deposits of silty sand to sandy silt
- 14b: Littoral-foreshore deposits of silty sand to sandy silt
- 19: Modern alluvial deposits
- 20: Organic deposits
- 21: Man-made deposits

Quaternary Geology - Michigan

- End moraines of fine-textured till
- Lacustrine clay and silt
- Lacustrine sand and gravel

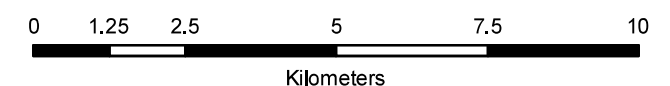
NOTE: Figure to be read in conjunction with accompanying report.


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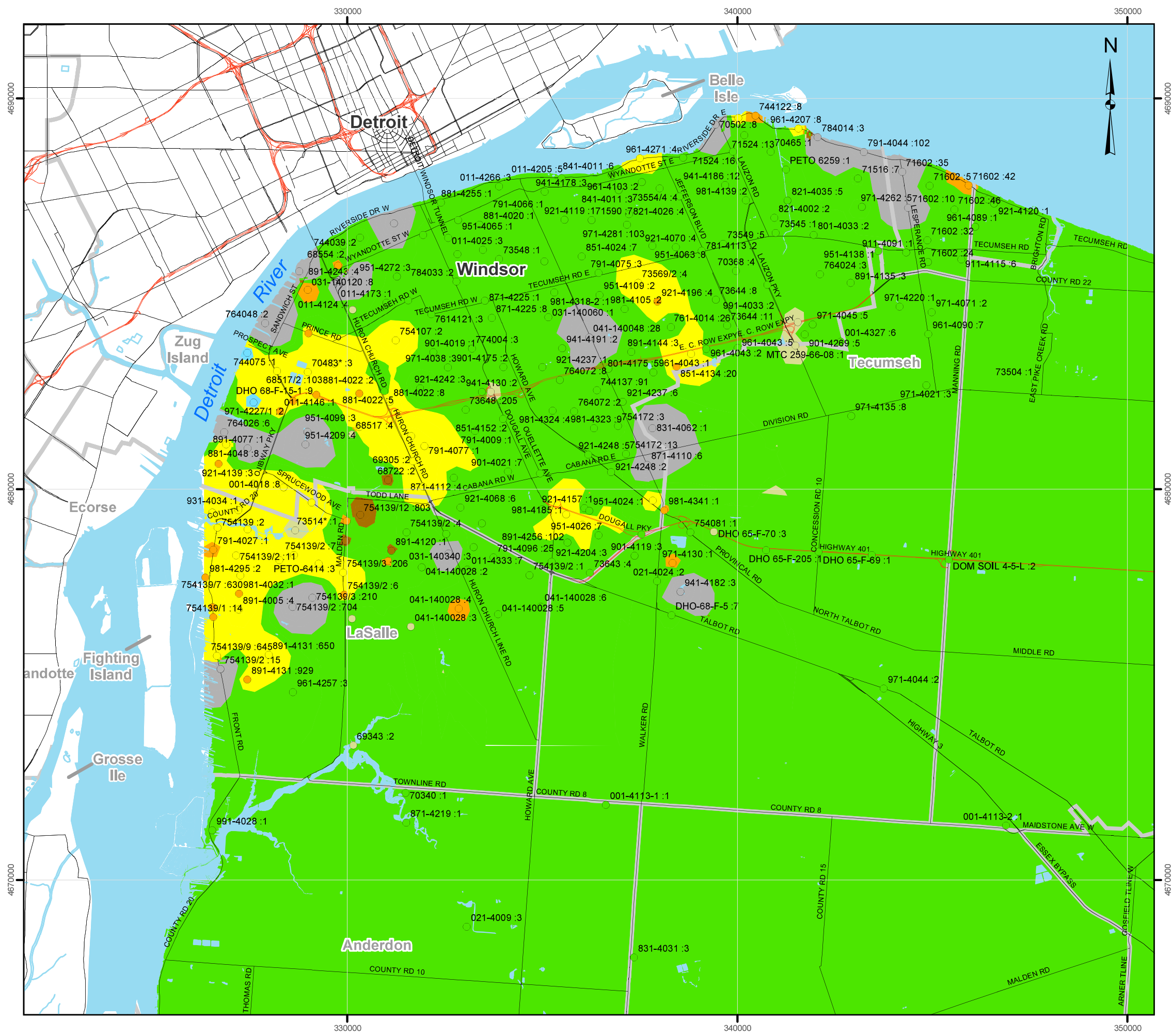
REFERENCE

Base Data - MNR NRVIS, obtained 2004 and CANMAP v7.3 obtained 2003, Ontario Geology - OGS digital 1:50 000, obtained 2003, Michigan Geology - downloaded from MI Geographic Data Library - 1:500000, obtained Feb. 2004. Produced by Golder Associates Ltd under licence from Ontario Ministry of Natural Resources, © Queens Printer 2005
 Datum: NAD 83 Projection: UTM Zone 17N



PROJECT	ROUTE PLANNING DETROIT RIVER INTERNATIONAL CROSSING CANADIAN SIDE		
TITLE	QUATERNARY GEOLOGY		
 Golder Associates Mississauga, Ontario	PROJECT No.	05-1140-003	SCALE 1:125,000
	DESIGN	AW 16 Feb. 2005	REV. 0
	CHECK	AB 14 Jul. 2005	FIGURE: 2
	REVIEW		

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LEGEND

- Expressway
- Major Road
- Fill
- Silt/Sandy Silt
- Silty Clay
- Clayey Silt
- Silty Sand
- Sand
- Sand and Gravel
- Water
- Municipal Boundary

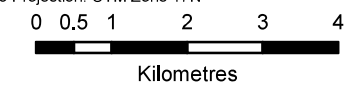
Borehole Labels: Job Number:Borehole Number

NOTE: Figure to be read in conjunction with accompanying report.



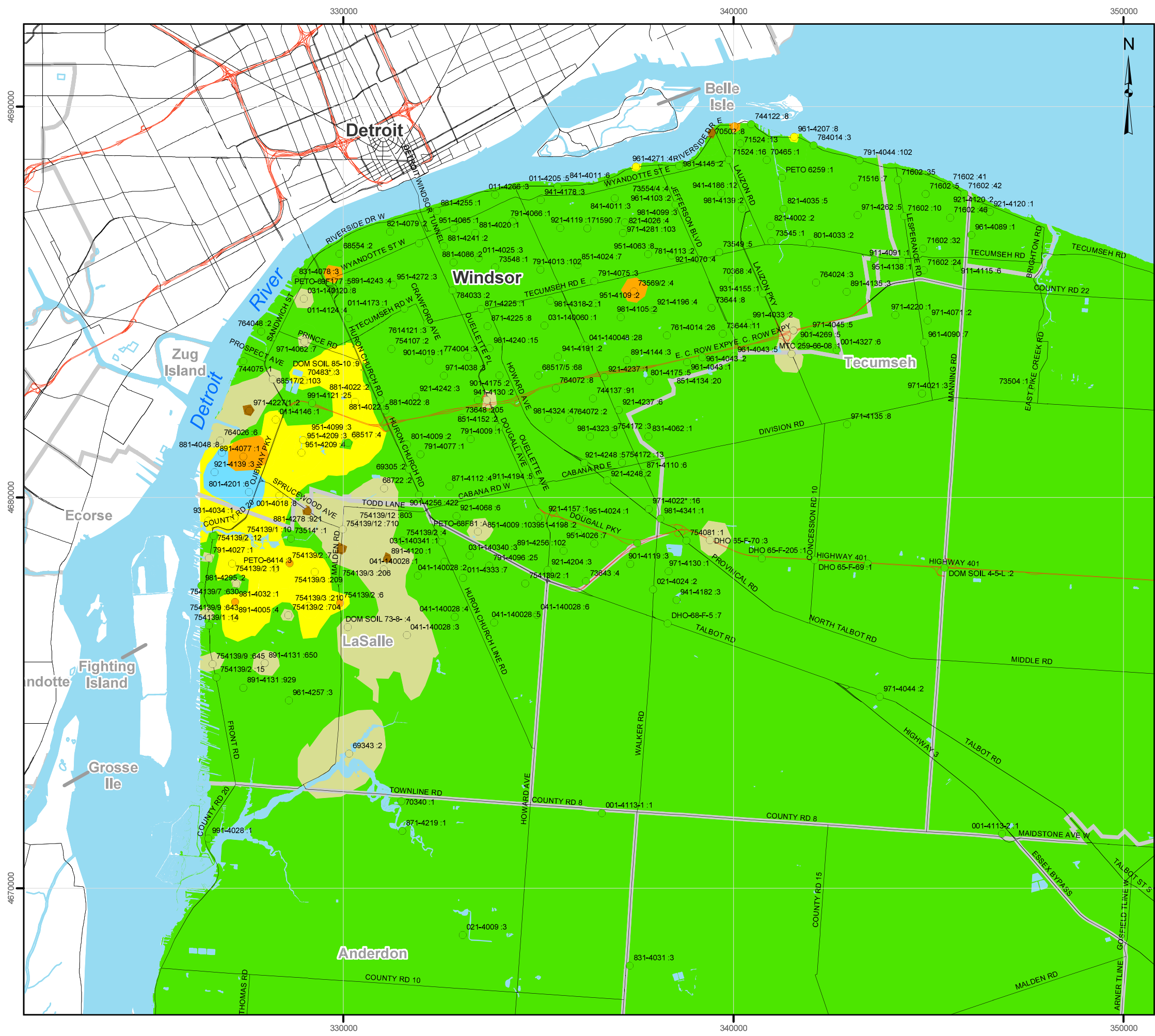
REFERENCE

Base Data - MNR NRVIS, obtained 2004, CANMAP v7.3 2003
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 Datum: NAD 83 Projection: UTM Zone 17N



PROJECT	ROUTE PLANNING DETROIT RIVER INTERNATIONAL CROSSING CANADIAN SIDE		
TITLE	SOIL CLASSIFICATION 0.3 M DEPTH		
 Golder Associates Mississauga, Ontario	PROJECT No. 05-1140-003	SCALE 1:100,000	REV. 0
	DESIGN CC 06 Jun. 2005		
	CHECK CC 17 Aug. 2005		
	REVIEW		
FIGURE: 3			

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LEGEND

- Expressway
- Major Road
- Clayey Silt
- Silty Clay
- Silt/Sandy Silt
- Silty Sand
- Sand
- Sand and Gravel
- Water
- Municipal Boundary

Borehole Labels: Job Number:Borehole Number

NOTE: Figure to be read in conjunction with accompanying report.

DRAFT



REFERENCE

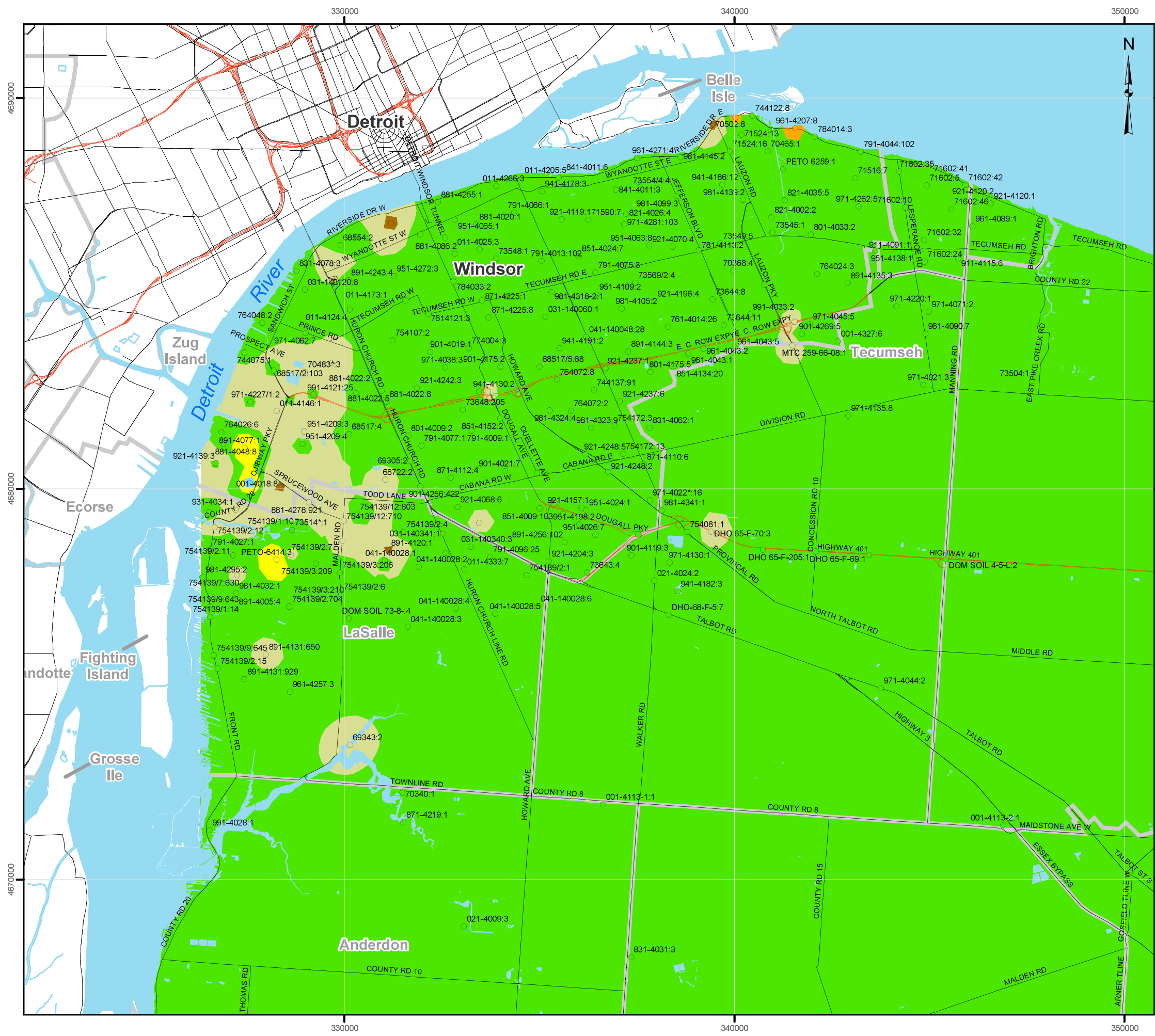
Base Data - MNR NRVIS, obtained 2004, CANMAP v7.3 2003
 Produced by Golder Associates Ltd under licence from Ontario Ministry of Natural Resources. © Queens Printer 2005
 Datum: NAD 83 Projection: UTM Zone 17N

0 0.5 1 2 3 4

 Kilometres

PROJECT	ROUTE PLANNING DETROIT RIVER INTERNATIONAL CROSSING CANADIAN SIDE		
TITLE	SOIL CLASSIFICATION 1.2 TO 1.5 M DEPTH		
 Golder Associates Mississauga, Ontario	PROJECT No. 05-1140-003	SCALE 1:100,000	REV. 0
	DESIGN CC 06 Jun. 2005		
	CHECK CC 17 Aug. 2005		
	REVIEW		
FIGURE: 4			

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LEGEND

- Expressway
- Major Road
- Clayey Silt
- Silty Clay
- Silt/Sandy Silt
- Silty Sand
- Sand
- Sand & Gravel

Borehole Labels: Job Number:Borehole Number

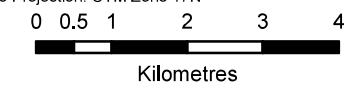
NOTE: Figure to be read in conjunction with accompanying report.

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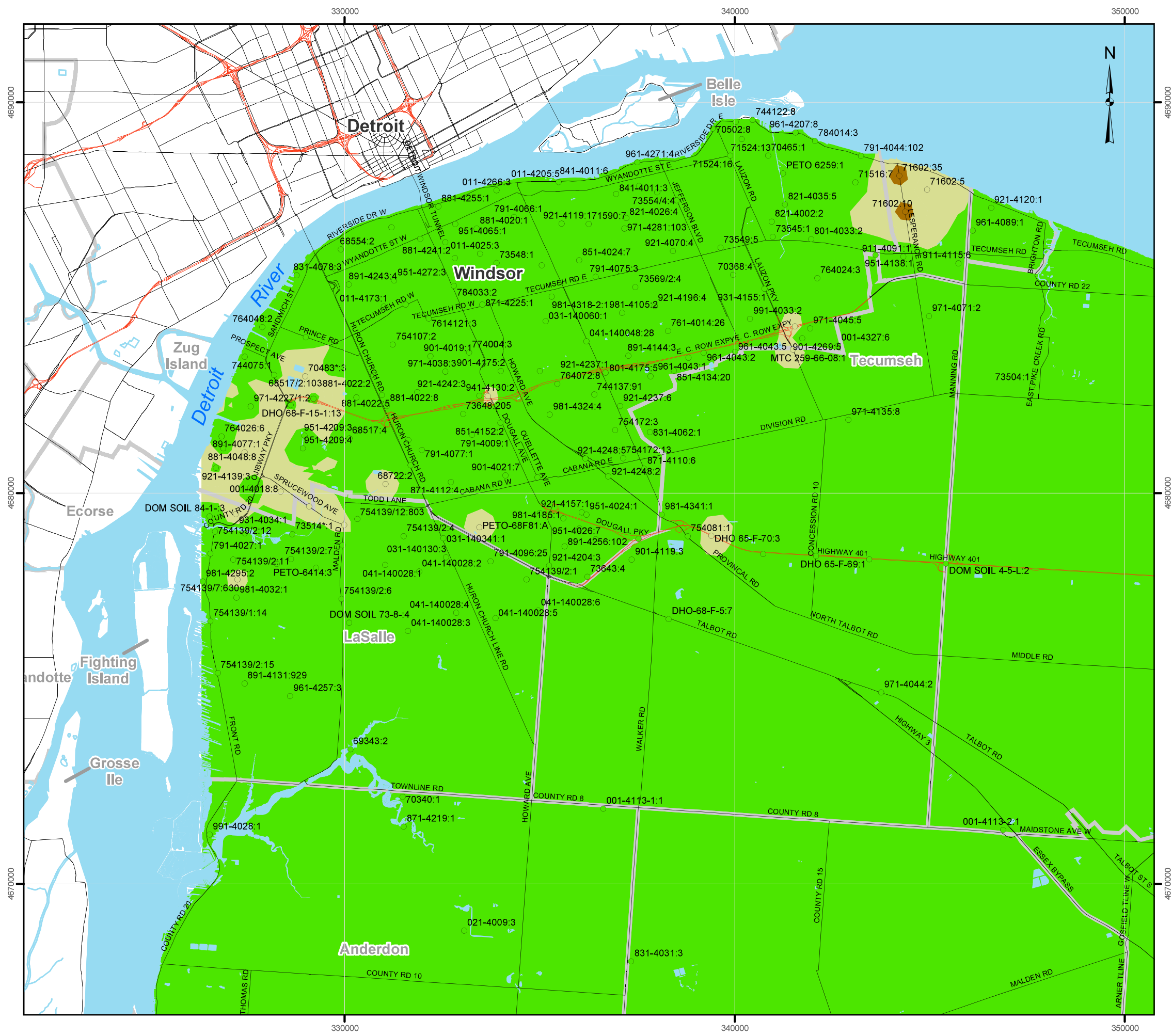
REFERENCE

Base Data - MNR NRVIS, obtained 2004, CANMAP v7.3 2003
 Produced by Golder Associates Ltd under licence from Ontario Ministry of Natural Resources, © Queens Printer 2005
 Datum: NAD 83 Projection: UTM Zone 17N



PROJECT	ROUTE PLANNING DETROIT RIVER INTERNATIONAL CROSSING CANADIAN SIDE		
TITLE	SOIL CLASSIFICATION 3 M DEPTH		
 Golder Associates Mississauga, Ontario	PROJECT No. 05-1140-003	SCALE 1:100,000	REV. 0
	DESIGN CC 06 Jun. 2005		
	GIS CC 17 Aug. 2005		
	CHECK REVIEW		
FIGURE: 5			

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LEGEND

- Expressway
- Major Road
- Clayey Silt
- Silt/Sandy Silt
- Silty Clay
- Water
- Municipal Boundary

Borehole Labels: Job Number:Borehole Number

NOTE: Figure to be read in conjunction with accompanying report.



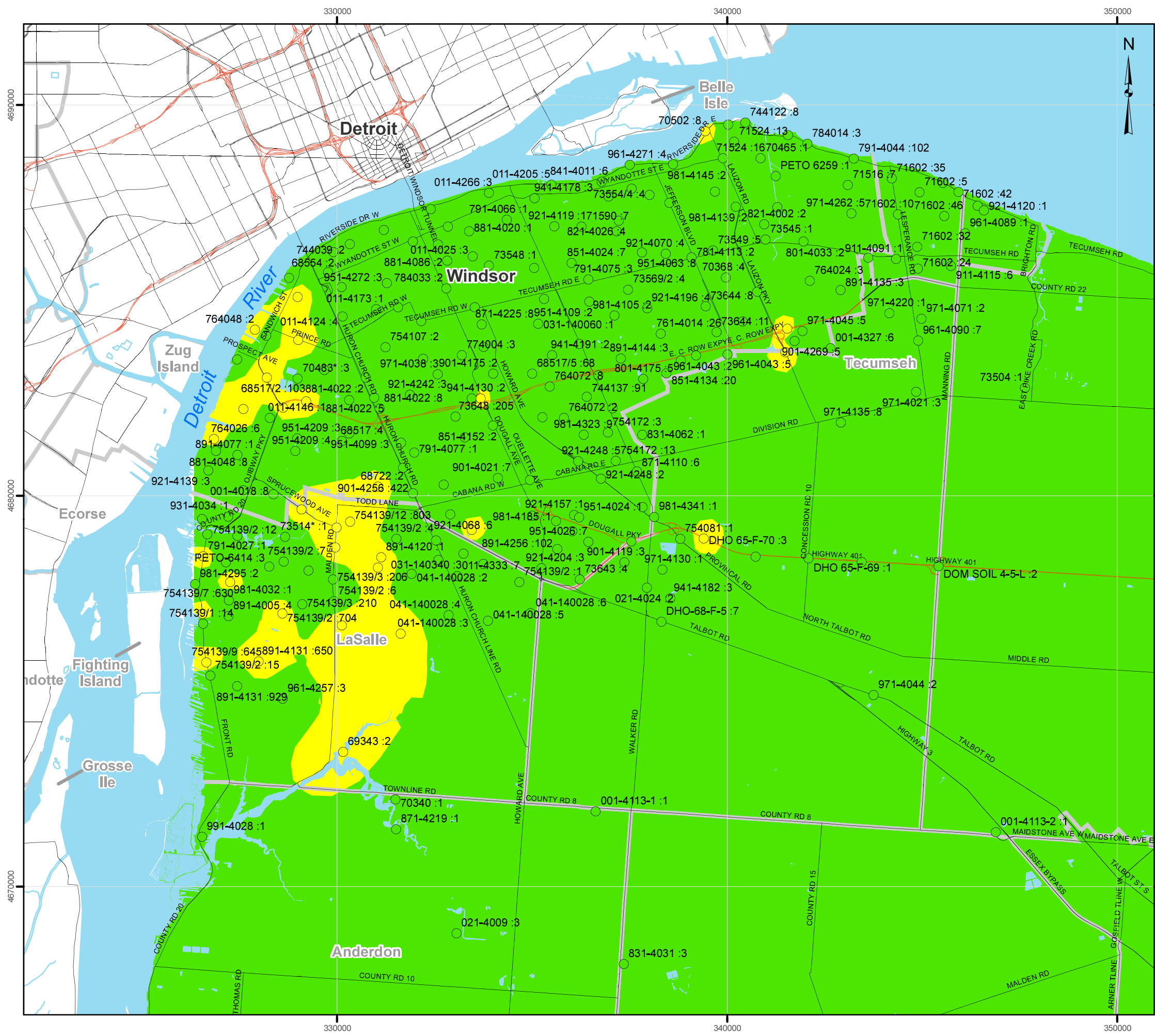
REFERENCE

Base Data - MNR NRVIS, obtained 2004, CANMAP v7.3 2003
 Produced by Golder Associates Ltd under licence from Ontario Ministry of Natural Resources. © Queens Printer 2005
 Datum: NAD 83 Projection: UTM Zone 17N

0 0.5 1 2 3 4
 Kilometres

PROJECT	ROUTE PLANNING DETROIT RIVER INTERNATIONAL CROSSING CANADIAN SIDE		
TITLE	SOIL CLASSIFICATION 6 M DEPTH		
 Golder Associates Mississauga, Ontario	PROJECT No. 05-1140-003	SCALE 1:100,000	REV. 0
	DESIGN CC 06 Jun. 2005		
	CHECK CC 17 Aug. 2005		
	REVIEW		
		FIGURE: 6	

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LEGEND

- Expressway
- Major Road
- Susceptibility**
 - Medium
 - Low
- Water
- Municipal Boundary

Borehole Labels: Job Number:Borehole Number

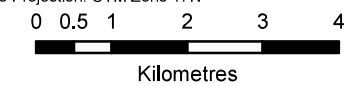
NOTE: Figure to be read in conjunction with accompanying report.

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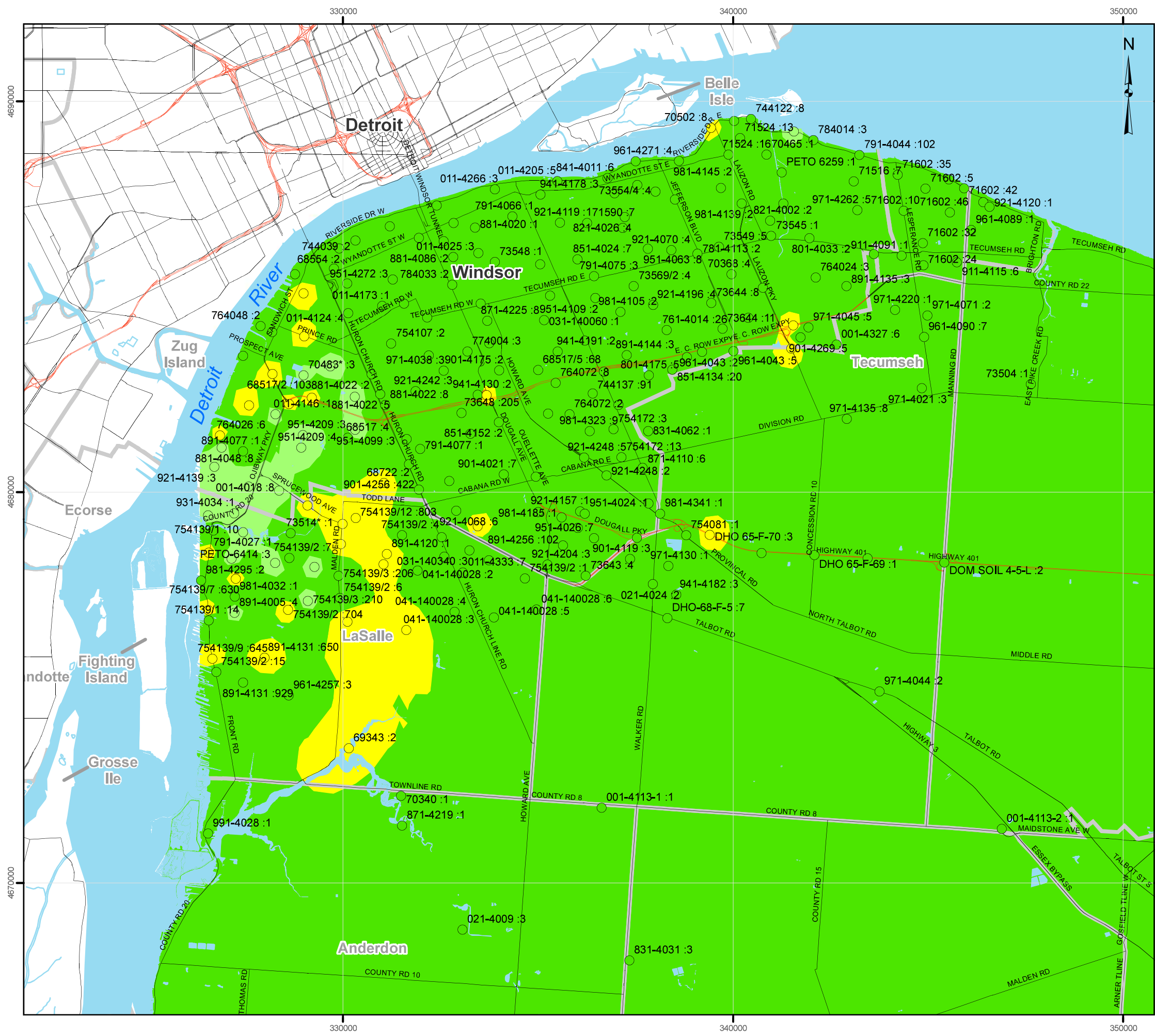
REFERENCE

Base Data - MNR NRVIS, obtained 2004, CANMAP v7.3 2003
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 Datum: NAD 83 Projection: UTM Zone 17N



PROJECT	ROUTE PLANNING DETROIT RIVER INTERNATIONAL CROSSING CANADIAN SIDE		
TITLE	FROST SUSCEPTIBILITY 1.2 TO 1.5 M DEPTH		
 Golder Associates Mississauga, Ontario	PROJECT No. 05-1140-003	SCALE 1:100,000	REV. 0
	DESIGN CC 06 Jun. 2005		
	GIS CC 17 Aug. 2005		
	CHECK REVIEW		
			FIGURE: 7

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LEGEND

- Expressway
- Major Road

Erodibility

- Moderate
- Slight
- Very Slight
- Water
- Municipal Boundary

Borehole Labels: Job Number:Borehole Number

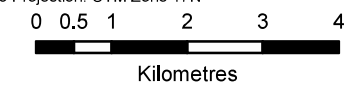
NOTE: Figure to be read in conjunction with accompanying report.

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REFERENCE

Base Data - MNR NRVIS, obtained 2004, CANMAP v7.3 2003
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 Datum: NAD 83 Projection: UTM Zone 17N



PROJECT	ROUTE PLANNING DETROIT RIVER INTERNATIONAL CROSSING CANADIAN SIDE		
TITLE	SOIL ERODIBILITY 1.2 TO 1.5 M DEPTH		
 Golder Associates Mississauga, Ontario	PROJECT No. 05-1140-003	SCALE 1:100,000	REV. 1
	DESIGN CC 06 Jun. 2005		
	CHECK CC 17 Aug. 2005		
	REVIEW		
			FIGURE: 8

APPENDIX A-I
SUBSURFACE DATABASE

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APPENDIX A-I
SUBSURFACE DATABASE

				Soil	Soil	Soil	Soil				
Job	BH # /			Classification	Classification	Classification	Classification	Frost			
#	TP #	Northing	Easting	0.3 m (1')	1.5 m (5')	3 m (10')	6 m (20')	Susceptibility	Erodibility		
				depth	depth	depth	depth	at 1.5 m depth	at 1.5 m depth		
<i>Note: bold soil classification indicates GSD carried out on a sample at about this depth</i>											
											Frost
											Susceptibility:
981-4032	1	4677335	327219	SiSa	SiSa	SiCl	SiCl	L	S		- H - High
981-4295	2	4677750	326357	SiSa	SiCl	SiCl	SiCl	L	S		- M - Medium
791-4027	1	4678463	326556	SiSa	CISi	SiCl	SiCl	M	M		- L - Low
754139/1	14	4676731	326555	SiSa	SiCl	SiCl	SiCl	L	S		
754139/9	643	4676989	326542	SiSa	CISi	SiCl	--	M	M		Erodibility:
754139/9	645	4675742	326647	Sa	CISi	SiCl	--	M	M		- VS - Very Slightly
754139/2	15	4675406	326746	Fill	SiCl	SiCl	SiCl	L	S		- S - Slightly
891-4131	929	4675134	327434	SiSa	SiCl	SiCl	SiCl	L	S		- M - Moderately
764026	6	4681463	326840	Fill	CISi	SiCl	SiCl	M	M		- H - Highly
921-4139	3	4680654	326696	SiSa	Sa&Gr	SiCl to CISi	SiCl to CISi	L	VS		
881-4048	8	4681152	326887	Sa	Sa&Gr	SiCl	SiCl	L	VS		
931-4034	1	4679475	327205	Sa	Sa	CISi	CISi	L	VS		
OM SOIL 84-1-W	3	4679417	326536	Sa	Sa	SiCl	SiCl	L	VS		
754139/2	12	4679032	326671	Sa	SiCl	CISi	SiCl	L	S		
754139/1	10	4678988	327437	Sa	Sa	CISi	SiCl	L	VS		
754139/2	11	4678313	327142	Sa	Sa	SiCl	SiCl	L	VS		
754139/7	630	4677798	327249	Sa	CISi	CISi	CISi	M	M		
891-4005	4	4676925	327211	Sa	Sa	SiCl	--	L	VS		
754139/2	704	4676996	328588	Fill	CISi	SiCl	--	M	M		
891-4131	650	4675768	327983	Sa	CISi	CISi	--	M	M		
961-4257	3	4674809	328599	SiCl	SiCl	SiCl	SiCl	L	S		
--	--	--	--	--	--	--	--				
744075	1	4683499	327434	Sa & Gr	SiCl	SiCl	SiCl	L	S		
971-4227/1	2	4682232	327588	Sa & Gr	SaSi	SiCl to CISi	SiCl	M	M		
891-4077	1	4681051	327438	Fill	SiSa	Sa	CISi	L	S		
011-4146	1	4682001	328266	SiSa	Sa	CISi	--	L	VS		
PETO-6414	3	4678200	328252	Sa	Sa	Sa	SiCl	L	VS		
801-4201	6	4680147	327581	Sa	Sa&Gr	Sa&Gr	SiCl	L	VS		
001-4018	8	4680049	328361	Sa	Sa	Si	CISi	L	VS		
73514	1	4678963	328656	CISi	SiCl	SiCl	SiCl	L	S		
881-4278	921	4679662	329082	Sa	SaSi	CISi	CISi	M	M		
754139/2	7	4678329	328623	Sa	SiSa	CISi	SiCl	L	S		
754139/3	209	4678101	329262	Sa	Sa	SiCl	SiCl	L	VS		
754139/3	210	4677230	329098	Fill	Sa	SiCl	--	L	VS		
754139/2	6	4677306	329912	SiSa	SiSa	SiCl	SiCl	L	S		
OM SOIL 73-8-W	4	4676694	330112	CISi	CISi	SiCl	SiCl	M	M		

APPENDIX A-I
SUBSURFACE DATABASE

Job #	BH # / TP #	Northing	Easting	Soil	Soil	Soil	Soil	Frost	Erodibility at 1.5 m depth		
				Classification	Classification	Classification	Classification	Susceptibility			
				0.3 m (1') depth	1.5 m (5') depth	3 m (10') depth	6 m (20') depth	at 1.5 m depth			
031-140089	3	4676323	330286	CISi	CISi	SiCl	SiCl	M	M		
--	--	--	--	--	--	--	--				
69343	2	4673459	330146	CISi	CISi	CISi	SiCl	M	M		
764048	2	4684264	327887	Fill	Alluvium (SiCl)	SiCl	SiCl	M	S		
981-4046	9	4684109	328176	Fill	SiCl	SiCl	SiCl	L	S		
70483	3	4683005	328984	Sa	Sa	CISi to SiCl	CISi to SiCl	L	VS		
68517/2	103	4683037	328189	Sa	CISi	SiCl	SiCl	M	M		
DHO 68-F-15-1	13	4682267	328609	SiSa	SiCl to CISi	SiCl to CISi	SiCl to CISi	M	M		
DHO 68-F-15-1	9	4682427	329199	SiSa	SiCl to CISi	SiCl to CISi	SiCl to CISi	M	M		
951-4209	3	4681474	328969	Fill	Sa	CISi	SiCl	L	VS		
951-4209	4	4681151	328923	Fill	Sa	SiCl	SiCl	L	VS		
68517	4	4681405	330118	CISi	SiCl	SiCl	SiCl	L	S		
754139/12	710	4679198	329980	SiSa	CISi	CISi	CISi	H	H		
754139/12	709	4678686	329944	SaSi	SaSi	CISi	SiCl	M	M		
754139/12	803	4679354	330326	Si	CISi	SiCl	SiCl	M	M		
891-4120	1	4678433	331130	SaSi	SaSi	SaSi	--	M	M		
041-140028	1	4678170	331043	SiSa	CISi	SiCl	SiCl	M	M		
754139/3	206	4677872	329883	Sa	SiCl	SiCl	--	L	S		
041-140028	3	4676484	331630	CISi	CISi	SiCl	SiCl	M	M		
PETO-69F177	5	4685730	329761	SiSa	SiSa	CISi to SiCl	SiCl	L	S		
831-4078	3	4685585	328762	Fill	SiCl	SiCl	SiCl	L	S		
031-140120	8	4685104	328981	SiSa	CISi	SiCl	--	M	M		
DM SOIL 85-10-V	9	4683231	329670	Sa	Sa	CISi	SiCl	L	VS		
011-4124	4	4684598	330131	CISi	SiCl	SiCl	--	L	S		
971-4062	7	4684003	329001	SiSa	CISi to SiCl	SiCl	SiCl	M	M		
881-4022	2	4683032	330734	SiCl	SiCl	SiCl	SiCl	L	S		
881-4022	5	4682520	330954	SiCl	SiCl	SiCl	SiCl	L	S		
991-4121	25	4682456	330299	SiSa	Sa	SiCl	SiCl	L	VS		
881-4022	8	4682242	331085	Sa	SiCl	SiCl	SiCl	L	S		
951-4099	3	4681603	330303	Sa	Sa	SiCl	SiCl	L	VS		
801-4009	2	4681369	331638	Sa	SiCl	SiCl	SiCl	L	S		
69305	2	4680724	331693	Sa	SiCl	SiCl	SiCl	L	S		
68722	2	4680242	331049	SaSi to SiSa	CISi	CISi	CISi	M	M		
901-4256	422	4680070	331948	Sa	SiCl	SiCl	SiCl	L	S		
031-140130	3	4678366	332597	Fill	SiCl	SiCl	SiCl	L	S		
031-140341	1	4678864	332535	SiCl	SiCl	SiCl	SiCl	L	S		
754139/2	4	4678911	331517	SiCl	SiCl	SiCl	SiCl	L	S		
011-4333	7	4677953	333077	SiCl	SiCl	SiCl	--	L	S		

APPENDIX A-I
SUBSURFACE DATABASE

Job #	BH # / TP #	Northing	Easting	Soil	Soil	Soil	Soil	Frost	Erodibility			
				Classification	Classification	Classification	Classification	Susceptibility	at 1.5 m depth	at 1.5 m depth		
				0.3 m (1') depth	1.5 m (5') depth	3 m (10') depth	6 m (20') depth					
041-140028	2	4678009	331912	SiCl	SiCl	SiCl	SiCl	L	S			
041-140028	4	4676950	332860	SiSa	SiCl	SiCl	SiCl	L	S			
041-140028	5	4676808	333869	SiCl	SiCl	SiCl	SiCl	L	S			
744039	2	4686446	330317	SiCl	SiCl	SiCl	SiCl	L	S			
68554	2	4686244	329895	SiCl	SiCl	SiCl	SiCl	L	S			
891-4243	4	4685347	330105	Fill	SiCl	SiCl	SiCl	L	S			
951-4272	3	4685446	331275	SiCl	SiCl	SiCl	SiCl	L	S			
011-4173	1	4684784	331005	SiCl	SiCl	SiCl	SiCl	L	S			
851-4126	2	4684842	331563	SiCl	SiCl	SiCl	SiCl	L	S			
754107	2	4683817	331240	Sa	SiCl	SiCl	SiCl	L	S			
901-4019	1	4683515	332203	SiCl	SiCl	SiCl	SiCl	L	S			
971-4038	3	4683123	332583	SiCl	SiCl	SiCl	SiCl	L	S			
921-4242	3	4682597	331864	SiCl to ClSi	SiCl	SiCl	SiCl	L	S			
73648	205	4682039	333040	SiCl	SiCl	SiCl	SiCl	L	S			
791-4077	1	4681115	331978	Sa	SiCl	SiCl	SiCl	L	S			
791-4009	1	4681500	333270	SiCl	SiCl	SiCl	SiCl	L	S			
871-4112	4	4680294	332728	SiCl	SiCl	SiCl	SiCl	L	S			
PETO-68F81	A	4679141	333452	SiCl	ClSi	ClSi	ClSi	M	M			
031-140340	3	4678526	333241	SiCl	SiCl	SiCl	--	L	S			
921-4068	6	4679537	332901	SiCl	SiCl	SiCl	--	L	S			
754139/2	1	4677805	334667	SiCl	SiCl	SiCl	SiCl	L	S			
791-4096	25	4678266	333746	SiCl	SiCl	SiCl	SiCl	L	S			
041-140028	6	4677007	334961	SiCl	SiCl	SiCl	SiCl	L	S			
821-4079	2	4686814	331199	Fill	SiCl	SaSi	SiCl	L	S			
881-4241	2	4686435	332584	SiCl	SiCl	SiCl	SiCl	L	S			
011-4079	2	4686515	331944	SiCl	SiCl	SiCl	SiCl	L	S			
7614121	3	4684486	332157	SiCl	SiCl	SiCl	SiCl	L	S			
881-4086	2	4686026	332828	SiCl	SiCl	SiCl	SiCl	L	S			
784033	2	4685316	332802	SiCl	SiCl	SiCl	SiCl	L	S			
774004	3	4683612	333194	SiCl	SiCl	SiCl	SiCl	L	S			
871-4225	1	4684839	333530	SiCl	SiCl	SiCl	SiCl	L	S			
981-4240	15	4683826	333755	SiCl	SiCl	SiCl	SiCl	L	S			
901-4175	2	4683134	334004	SiCl	SiCl	SiCl	SiCl	L	S			
941-4130	2	4682501	333455	SiCl	SiCl	SiCl	SiCl	L	S			
DHO-68-F-15-2	4	4682497	333680	ClSi	ClSi	ClSi	ClSi	M	M			
851-4152	2	4681803	333997	SiCl	SiCl	SiCl	SiCl	L	S			
901-4021	7	4680461	334123	SiCl	SiCl	SiCl	SiCl	L	S			
911-4194	5	4680406	334943	SiCl	SiCl	SiCl	SiCl	L	S			

APPENDIX A-I
SUBSURFACE DATABASE

Job #	BH # / TP #	Northing	Easting	Soil	Soil	Soil	Soil	Frost	Erodibility			
				Classification	Classification	Classification	Classification	Susceptibility	at 1.5 m depth	at 1.5 m depth		
				0.3 m (1') depth	1.5 m (5') depth	3 m (10') depth	6 m (20') depth					
851-4009	103	4679506	335005	SiCl	SiCl	SiCl	SiCl	L	S			
981-4185	1	4679368	335609	Sa	SiCl	SiCl	SiCl	L	S			
951-4198	2	4679102	336022	Sa	SiCl	SiCl	SiCl	L	S			
891-4256	102	4678649	335644	SiCl	SiCl	SiCl	SiCl	L	S			
921-4204	3	4678157	335239	SiCl	SiCl	SiCl	SiCl	L	S			
73643	4	4677869	336224	SiCl	SiCl	SiCl	SiCl	L	S			
881-4255	1	4687346	332404	SiCl	SiCl	SiCl	SiCl	L	S			
951-4065	1	4686899	332838	SiCl	SiCl	SiCl	SiCl	L	S			
881-4020	1	4686779	333388	SiCl	SiCl	SiCl	SiCl	L	S			
73548	1	4685896	333892	SiCl	SiCl	SiCl	SiCl	L	S			
011-4025	3	4686131	333475	SiCl	SiCl	SiCl	SiCl	L	S			
871-4225	8	4684401	333703	SiCl	SiCl	SiCl	--	L	S			
031-140060	1	4684414	335154	SiCl	SiCl	SiCl	SiCl	L	S			
941-4191	2	4683614	335508	SiCl	SiCl	SiCl	--	L	S			
68517/5	68	4683135	335004	SiCl	SiCl	SiCl	SiCl	L	S			
764072	8	4682805	335453	SiCl	SiCl	SiCl	SiCl	L	S			
981-4324	4	4682019	335262	SiCl	SiCl	SiCl	SiCl	L	S			
981-4323	9	4681568	336322	SiCl	SiCl	SiCl	--	L	S			
764072	2	4682002	335811	SiCl	SiCl	SiCl	--	L	S			
921-4248	5	4680892	336178	SiCl	SiCl	SiCl	SiCl	L	S			
921-4248	2	4680439	336762	SiCl	SiCl	SiCl	SiCl	L	S			
921-4157	1	4679521	336078	SiCl	SiCl	SiCl	SiCl	L	S			
951-4024	1	4679460	336200	SiCl	SiCl	SiCl	SiCl	L	S			
PETO-01TF072A	71-4	4678846	337540	SiCl	SiCl	SiCl	SiCl	L	S			
951-4026	7	4678835	336437	SiCl	SiCl	SiCl	SiCl	L	S			
901-4119	3	4678306	337362	SiCl	SiCl	SiCl	SiCl	L	S			
021-4024	2	4677655	337942	SiCl	SiCl	SiCl	--	L	S			
DHO-68-F-5	7	4676785	338312	SiCl	SiCl	SiCl	SiCl	L	S			
011-4266	3	4687762	333893	SiCl	SiCl	SiCl	SiCl	L	S			
941-4178	3	4687623	335062	SiCl	SiCl	SiCl	--	L	S			
791-4066	1	4687071	334348	SiCl	SiCl	SiCl	SiCl	L	S			
921-4119	1	4686907	335567	SiCl	SiCl	SiCl	SiCl	L	S			
791-4013	102	4685836	335052	SiCl	SiCl	SiCl	SiCl	L	S			
981-4318-2	1	4685046	335309	SiCl	SiCl	SiCl	SiCl	L	S			
951-4109	2	4684982	336466	SiCl	SiCl	SiCl	--	L	S			
851-4024	7	4685965	336013	SiCl	SiCl	SiCl	SiCl	L	S			
041-140048	28	4683896	336202	Fill	SiCl	SiCl	SiCl	L	S			
921-4237	1	4683093	336694	SiCl	SiCl	SiCl	SiCl	L	S			

APPENDIX A-I
SUBSURFACE DATABASE

Job #	BH # / TP #	Northing	Easting	Soil	Soil	Soil	Soil	Frost	Erodibility			
				Classification	Classification	Classification	Classification	Susceptibility	at 1.5 m depth	at 1.5 m depth		
				0.3 m (1') depth	1.5 m (5') depth	3 m (10') depth	6 m (20') depth					
891-4144	3	4683522	337275	SiCl	SiCl	SiCl	SiCl	L	S			
921-4237	6	4682243	337062	SiCl	SiCl	SiCl	SiCl	L	S			
754172	3	4681625	336937	SiCl	SiCl	SiCl	SiCl	L	S			
744137	91	4682539	336402	SiCl	SiCl	SiCl	SiCl	L	S			
831-4062	1	4681571	337825	Fill	SiCl	SiCl	SiCl	L	S			
754172	13	4680904	337143	SiCl	SiCl	SiCl	SiCl	L	S			
871-4110	6	4680629	337683	Fill	SiCl	SiCl	SiCl	L	S			
754081	1	4678913	338795	SiCl	SiCl	SiCl	SiCl	L	S			
981-4341	1	4679465	338126	SiSa	SiCl	SiCl	SiCl	L	S			
971-4022	16	4679713	337831	Sa	SiCl	SiCl	--	L	S			
971-4130	1	4678122	338341	SiSa	SiCl	SiCl	--	L	S			
941-4182	3	4677387	338543	Fill	SiCl	SiCl	--	L	S			
011-4205	5	4687972	335492	SiCl	SiCl	SiCl	SiCl	L	S			
841-4011	6	4688049	336780	SiCl	SiCl	SiCl	SiCl	L	S			
841-4011	3	4687664	336960	SiCl	SiCl	SiCl	SiCl	L	S			
71590	7	4686899	336259	SiCl	SiCl	SiCl	SiCl	L	S			
821-4026	4	4687049	337221	SiCl	SiCl	SiCl	SiCl	L	S			
791-4075	3	4685539	336437	SiCl	SiCl	SiCl	SiCl	L	S			
971-4281	103	4686775	337172	SiCl	SiCl	SiCl	SiCl	L	S			
73569/2	4	4685283	337456	Sa	SiSa	SiCl	SiCl	L	S			
981-4105	2	4684629	337113	SiCl	SiCl	SiCl	SiCl	L	S			
921-4196	4	4684809	337952	SiSa	SiCl	SiCl	SiCl	L	S			
761-4014	26	4684160	338295	SiCl	SiCl	SiCl	SiCl	L	S			
801-4175	5	4683003	337846	SiCl	SiCl	SiCl	SiCl	L	S			
851-4134	20	4683146	338443	SiSa	SiCl	SiCl	SiCl	L	S			
961-4043	1	4683472	338814	Sa	SiCl	SiCl	SiCl	L	S			
DHO 65-F-70	3	4678919	339400	CiSi	CiSi	CiSi	CiSi	M	M			
DHO 65-F-205	1	4678453	340730	SiCl	SiCl	SiCl	SiCl	L	S			
961-4271	4	4688482	337500	Sa	Sa	SiCl	SiCl	L	S			
961-4103	2	4687870	337548	SiCl	SiCl	SiCl	SiCl	L	S			
73554/4	4	4687706	338008	SiCl	SiCl	SiCl	SiCl	L	S			
981-4099	3	4687503	338515	SiCl	SiCl	SiCl	SiCl	L	S			
951-4063	8	4686237	337817	SiCl	SiCl	SiCl	--	L	S			
921-4070	4	4686204	338420	SiCl	SiCl	SiCl	SiCl	L	S			
781-4113	2	4686111	339086	SiCl	SiCl	SiCl	--	L	S			
73644	8	4684860	339444	SiCl	SiCl	SiCl	--	L	S			
73644	11	4684200	339731	SiCl	SiCl	SiCl	--	L	S			
961-4043	2	4683603	339202	SiCl	SiCl	SiCl	SiCl	L	S			

APPENDIX A-I
SUBSURFACE DATABASE

Job #	BH # / TP #	Northing	Easting	Soil	Soil	Soil	Soil	Frost	Erodibility			
				Classification	Classification	Classification	Classification	Susceptibility	at 1.5 m depth	at 1.5 m depth		
				0.3 m (1') depth	1.5 m (5') depth	3 m (10') depth	6 m (20') depth					
961-4043	5	4683624	340006	SiCl	SiCl	SiCl	SiCl	L	S			
DHO 65-F-69	1	4678407	342084	SiCl	SiCl	SiCl	SiCl	L	S			
971-4044	2	4674907	343750	SiCl	SiCl	SiCl	SiCl	L	S			
70502	8	4689341	339438	Fill	SaSi	SaSi	SiCl	M	M			
981-4145	2	4688493	338611	SiCl	SiCl	SiCl	--	L	S			
71524	16	4688648	339885	SiCl	SiCl	SiCl	SiCl	L	S			
941-4186	12	4687794	339685	SiCl	SiCl	SiCl	--	L	S			
981-4139	2	4687402	340218	SiCl	SiCl	SiCl	--	L	S			
73549	5	4686285	339635	SiCl	SiCl	SiCl	SiCl	L	S			
73545	1	4686565	340976	SiCl	SiCl	SiCl	SiCl	L	S			
70368	4	4685589	339963	SiCl	SiCl	SiCl	SiCl	L	S			
931-4155	1	4685238	340766	SiCl	SiCl	SiCl	SiCl	L	S			
MTC 259-66-08	1	4683685	341491	CiSi	CiSi	CiSi	CiSi	M	M			
MTC 259-66-04	106	4684284	341543	CiSi	CiSi	CiSi	CiSi	M	M			
991-4033	2	4684478	340395	SiCl	SiCl	SiCl	SiCl	L	S			
901-4269	5	4683973	341728	SiCl	SiCl	SiCl	SiCl	L	S			
971-4135	8	4681882	342915	SiCl	SiCl	SiCl	SiCl	L	S			
DOM SOIL 5-3-7	1	4678321	343445	SiCl	SiCl	SiCl	SiCl	L	S			
744122	1	4689513	340025	Sa	SiSa	SiSa	SiCl	L	S			
744122	8	4689567	340460	SiSa	SiCl	SiCl	SiCl	L	S			
70465	1	4688648	340854	SiCl	SiCl	SiCl	SiCl	L	S			
71524	13	4689072	340179	SiCl	SiCl	SiCl	SiCl	L	S			
PETO 6259	1	4688192	341242	SiCl	SiCl	SiCl	SiCl	L	S			
821-4035	5	4687394	341285	SiCl	SiCl	SiCl	SiCl	L	S			
821-4002	2	4686956	340948	SiCl	SiCl	SiCl	SiCl	L	S			
801-4033	2	4686511	341957	SiCl	SiCl	SiCl	SiCl	L	S			
764024	3	4685505	342115	SiCl	SiCl	SiCl	SiCl	L	S			
891-4135	3	4685282	342904	SiCl	SiCl	SiCl	--	L	S			
971-4045	5	4684227	341930	SiCl	SiCl	SiCl	SiCl	L	S			
001-4327	6	4683788	342651	SiCl	SiCl	SiCl	SiCl	L	S			
DOM SOIL 4-5-L	2	4678207	345416	SiCl	SiCl	SiCl	SiCl	L	S			
961-4207	8	4689240	341560	Sa	Sa	SiSa	SiCl	L	VS			
784014	3	4689020	342058	Fill	SiCl	SiCl	SiCl	L	S			
791-4044	102	4688642	343237	Fill	SiCl	SiCl	SiCl	L	S			
71516	7	4687965	343090	SiCl	SiCl	SiCl	SiCl	L	S			
971-4262	5	4687231	343186	SiCl	SiCl	SiCl	--	L	S			
951-4138	1	4686102	343610	SiCl	SiCl	SiCl	SiCl	L	S			
911-4091	1	4686062	344321	SiCl	SiCl	SiCl	SiCl	L	S			

APPENDIX A-I
SUBSURFACE DATABASE

Job #	BH # / TP #	Northing	Easting	Soil	Soil	Soil	Soil	Frost	Erodibility			
				Classification	Classification	Classification	Classification	Susceptibility	at 1.5 m depth	at 1.5 m depth		
				0.3 m (1')	1.5 m (5')	3 m (10')	6 m (20')					
				depth	depth	depth	depth					
971-4220	1	4684682	344153	SiCl	SiCl	SiCl	--	L	S			
971-4071	2	4684539	344979	SiCl	SiCl	SiCl	SiCl	L	S			
961-4090	7	4683983	344892	SiCl	SiCl	SiCl	--	L	S			
971-4021	3	4682670	344838	SiCl	SiCl	SiCl	--	L	S			
71602	35	4688132	344219	Fill	SiCl	SiCl	SaSi	L	S			
71602	10	4687212	344375	Fill	SiCl	SiCl	SaSi	L	S			
71602	5	4687777	344929	SiCl	SiCl	SiCl	ClSi	L	S			
71602	46	4687167	345558	SiCl	SiCl	SiCl	--	L	S			
71602	32	4686380	344863	SiCl	SiCl	SiCl	--	L	S			
71602	24	4685818	344881	SiCl	SiCl	SiCl	--	L	S			
911-4115	6	4685888	345735	SiCl	SiCl	SiCl	SiCl	L	S			
71602	41	4688017	345528	SiSa	SiCl	SiCl	--	L	S			
71602	42	4687784	345923	SiSa	SiCl	SiCl	--	L	S			
921-4120	2	4687430	346415	SiCl	SiCl	SiCl	--	L	S			
961-4089	1	4686724	346097	SiCl	SiCl	SiCl	SiCl	L	S			
County Road 8 and East of Manning Road												
70340	1	46722391	331493	SiCl	SiCl	SiCl	SiCl	L	S			
991-4028	1	4671276	326536	SiCl	SiCl	SiCl	SiCl	L	S			
871-4219	1	4671475	331513	SiCl	SiCl	SiCl	SiCl	L	S			
831-4031	3	4668031	337346	SiCl	SiCl	SiCl	SiCl	L	S			
001-4113-1	1	4671933	336630	SiCl	SiCl	SiCl	SiCl	L	S			
001-4113-2	1	4671395	346882	SiCl	SiCl	SiCl	SiCl	L	S			
021-4009	3	4668807	333066	SiCl	SiCl	SiCl	SiCl	L	S			
921-4120	1	4687315	346575	SiCl	SiCl	SiCl	SiCl	L	S			
73504	1	4682776	347631	SiCl	SiCl	SiCl	SiCl	L	S			

APPENDIX B
SUBSURFACE DATABASE
DETROIT RIVER INTERNATIONAL CROSSING

SUBSURFACE DATABASE

REFERENCE	BOREHOLE OR TEST PIT	NORTHING (m)	EASTING (m)	SOIL CLASSIFICATION				FROST SUSCEPTIBILITY	ERODIBILITY
				0.3 m Depth	1.5 m Depth	3 m Depth	6 m Depth	1.5 m depth	1.5 m depth
744075	1	4,683,499	327,434	Sa & Gr	Si Cl	Si Cl	Si Cl	L	S
971-4227/1	2	4,682,232	327,588	Sa & Gr	SaSi	Si Cl to Cl Si	Si Cl	M	M
011-4146	1	4,682,001	328,266	Si Sa	Sa	Cl Si	--	L	VS
70483	3	4,683,005	328,984	Sa	Sa	Cl Si to Si Cl	Cl Si to Si Cl	L	VS
68517/2	103	4,683,037	328,189	Sa	Cl Si	Si Cl	Si Cl	M	M
DHO 68-F-15-1	13	4,682,267	328,609	Si Sa	Si Cl to Cl Si	Si Cl to Cl Si	Si Cl to Cl Si	M	M
DHO 68-F-15-1	9	4,682,427	329,199	Si Sa	Si Cl to Cl Si	Si Cl to Cl Si	Si Cl to Cl Si	M	M
951-4209	3	4,681,474	328,969	Fill	Sa	Cl Si	Si Cl	L	VS
68517	4	4,681,405	330,118	Cl Si	Si Cl	Si Cl	Si Cl	L	S
881-4022	5	4,682,520	330,954	Si Cl	Si Cl	Si Cl	Si Cl	L	S
881-4022	8	4,682,242	331,085	Sa	Si Cl	Si Cl	Si Cl	L	S
951-4099	3	4,681,603	330,303	Sa	Sa	Si Cl	Si Cl	L	VS
69305	2	4,680,724	331,693	Sa	Si Cl	Si Cl	Si Cl	L	S
68722	2	4,680,242	331,049	Sa Si to Si Sa	Cl Si	Cl Si	Cl Si	M	M
754139/2	4	4,678,911	331,517	Si Cl	Si Cl	Si Cl	Si Cl	L	S
011-4333	7	4,677,953	333,077	Si Cl	Si Cl	Si Cl	--	L	S
041-140028	2	4,678,009	331,912	Si Cl	Si Cl	Si Cl	Si Cl	L	S
791-4077	1	4,681,115	331,978	Sa	Si Cl	Si Cl	Si Cl	L	S
871-4112	4	4,680,294	332,728	Si Cl	Si Cl	Si Cl	Si Cl	L	S
921-4068	6	4,679,537	332,901	Si Cl	Si Cl	Si Cl	--	L	S
754139/2	1	4,677,805	334,667	Si Cl	Si Cl	Si Cl	Si Cl	L	S
791-4096	25	4,678,266	333,746	Si Cl	Si Cl	Si Cl	Si Cl	L	S
041-140028	6	4,677,007	334,961	Si Cl	Si Cl	Si Cl	Si Cl	L	S
921-4204	3	4,678,157	335,239	Si Cl	Si Cl	Si Cl	Si Cl	L	S
73643	4	4,677,869	336,224	Si Cl	Si Cl	Si Cl	Si Cl	L	S
901-4119	3	4,678,306	337,362	Si Cl	Si Cl	Si Cl	Si Cl	L	S
04-111-060	1	4,677,738	335,500	Si Cl	Si Cl	Si Cl	Cl Si	L	S
04-111-060	7	4,678,848	333,325	Cl Si	Cl Si	Cl Si	Cl Si	L	S
04-111-060	14	4,680,648	331,648	Fill	Cl Si	Cl Si	Cl Si	L	S
04-111-060	23	4,682,323	328,529	Fill and Topsoil	Sa Si	Cl Si	Cl Si	M	M

Frost Susceptibility:
 - M - Moderate
 - L - Low

Erodibility:
 - VS - Very Slight
 - S - Slight
 - M - Moderate

- NOTES:
1. **Bold** soil classification indicates grain size analysis carried out on a sample at about this depth.
 2. Table to be read in conjunction with accompanying report.

APPENDIX C
RECORDS OF BOREHOLES
(GOLDER ASSOCIATES LTD. PROJECT NO. 04-1111-060)
DETROIT RIVER INTERNATIONAL CROSSING

PROJECT 04-1111-060

RECORD OF BOREHOLE No 1

1 OF 4

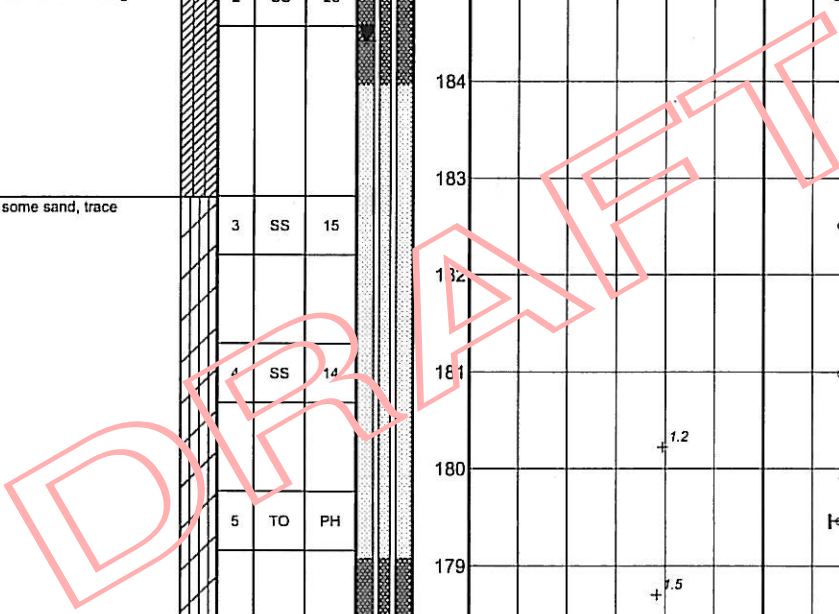
METRIC

G.W.P. _____ LOCATION 335500E, 4677738N ORIGINATED BY C.C.

DIST SW Region HWY 401 / 3 BOREHOLE TYPE POWER AUGER, HOLLOW STEM COMPILED BY T.M.

DATUM Geodetic DATE November 2, 2006 - November 5, 2006 CHECKED BY SJS

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT W _p	NATURAL MOISTURE CONTENT W	LIQUID LIMIT W _L	UNIT WEIGHT γ kN/m ³	REMARKS & GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa						
186.70	GROUND SURFACE													
0.00	TOPSOIL, clayey Firm Black		1	SS	5									
0.27	SILTY CLAY, some sand, trace gravel, fissured, oxidized along fissures Firm Brown													
185.18														
1.52	SILTY CLAY, some sand, trace gravel, fissured and oxidized along fissures Very stiff Brown		2	SS	28									
182.80														
3.90	CLAYEY SILT, some sand, trace gravel Stiff Grey		3	SS	15									
			4	SS	14									
			5	TO	PH			1.2					21.1	9 30 36 25 CICU, Oedometer
			6	SS	9			1.5						
			7	TO	PH			1.0						
			8	SS	11			1.7						
			9	TO	PH									
			10	TO	PH								20.5	2 28 41 29 CICU, Oedometer
			11	SS	10									



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+ 3, X 3: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

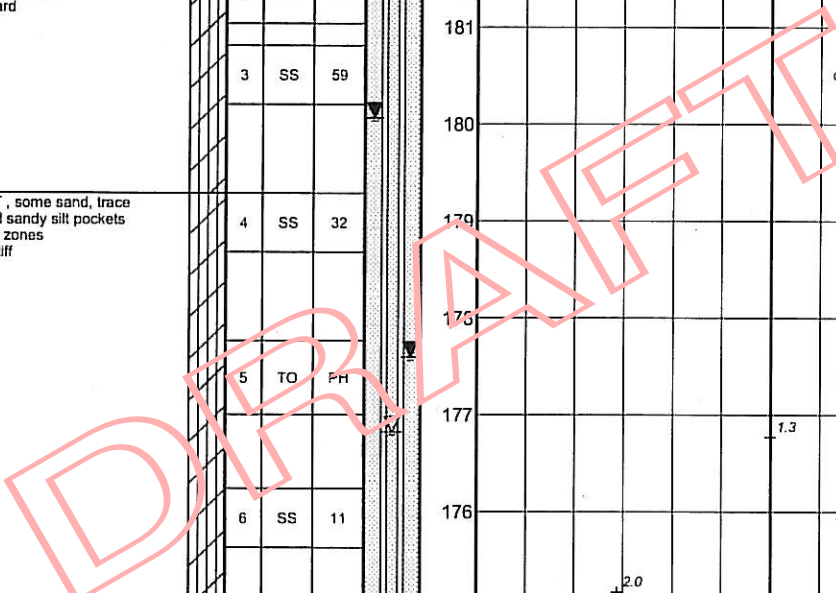
RECORD OF BOREHOLE No 7

1 OF 4

METRIC

PROJECT 04-1111-060 LOCATION 333325E, 4678848N ORIGINATED BY C.C.
 G.W.P. _____ DIST SW Region HWY 401/3 BOREHOLE TYPE POWER AUGER, HOLLOW STEM COMPILED BY T.M.
 DATUM Geodetic DATE November 10, 2006 - November 16, 2006 CHECKED BY SJB

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)		
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			20	40						60	80
183.17	GROUND SURFACE															
0.00	TOPSOIL, clayey															
0.25	Firm Black CLAYEY SILT, some sand, trace gravel Stiff Mottled brown and grey		1	SS	10											
181.65	CLAYEY SILT, some sand, trace gravel, oxidized fissures Very stiff to hard Brown		2	SS	23											
1.52			3	SS	59											
179.28	CLAYEY SILT, some sand, trace gravel, silt and sandy silt pockets and laminated zones Firm to very stiff Grey		4	SS	32											
3.89			5	TO	PH											
			6	SS	11											
			7	TO	PH											
			8	SS	12											
			9	TO	PH											
			10	SS	8											
			11	TO	PH											



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+³, X³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

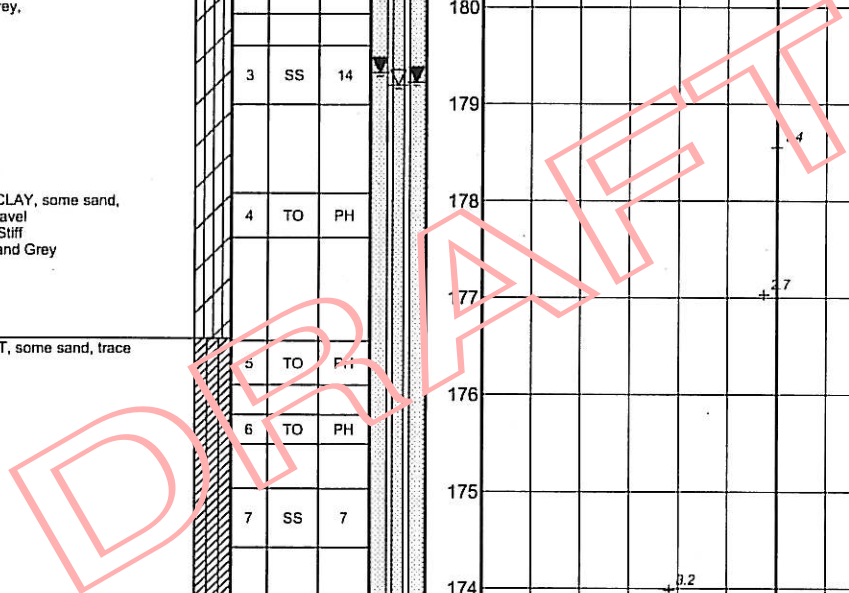
RECORD OF BOREHOLE No 14

1 OF 4

METRIC

PROJECT 04-1111-060 LOCATION 331648E, 4680648N ORIGINATED BY C.C.
 G.W.P. _____ DIST SW Region HWY 401/3 BOREHOLE TYPE POWER AUGER, HOLLOW STEM COMPILED BY T.M.
 DATUM Geodetic DATE November 18, 2006 - November 23, 2006 CHECKED BY SJB

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa						
						20	40	60	80	100	10	20	30	GR SA SI CL
182.06 0.00	GROUND SURFACE FILL, silty clay, pockets of crushed gravel, topsoil, sand partings, glass fragments Stiff Brown to grey	[Hatched]	1	SS	13									
180.99 1.07 180.89	FILL, silty sand and gravel Brown	[Hatched]												
1.37	CLAYEY SILT, laminated, some sand Soft to stiff Brown and grey,	[Grid]	2	SS	4									
	SILTY CLAY, some sand, trace gravel Soft to Stiff Brown and Grey	[Grid]	3	SS	14									
		[Grid]	4	TO	PH									
176.57 5.49	CLAYEY SILT, some sand, trace gravel Firm to stiff Grey	[Grid]	5	TO	PH									
		[Grid]	6	TO	PH									
		[Grid]	7	SS	7									
		[Grid]	8	TO	PH									
		[Grid]	9	TO	PH									
		[Grid]	10	SS	WR									
		[Grid]	11	TO	PH									
		[Grid]												



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+ 3, X 3: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

PROJECT 04-1111-060 **RECORD OF BOREHOLE No 23** 1 OF 3 **METRIC**
 G.W.P. _____ LOCATION 32B529E, 46B2323N ORIGINATED BY C.C.
 DIST SW Region HWY 401/3 BOREHOLE TYPE POWER AUGER, HOLLOW STEM COMPILED BY T.M.
 DATUM Geodetic DATE November 24, 2006 - November 26, 2006 CHECKED BY SJB

SOIL PROFILE			SAMPLES			GROUND WATER CONDITIONS	ELEVATION SCALE	DYNAMIC CONE PENETRATION RESISTANCE PLOT		PLASTIC LIMIT w _p	NATURAL MOISTURE CONTENT w	LIQUID LIMIT w _L	UNIT WEIGHT γ	REMARKS & GRAIN SIZE DISTRIBUTION (%)
ELEV DEPTH	DESCRIPTION	STRAT PLOT	NUMBER	TYPE	"N" VALUES			SHEAR STRENGTH kPa						
178.92	GROUND SURFACE													
0.00	FILL, silty sand, with gravel and silt pockets		1	AS	-									
178.92	Brown													
0.30	TOPSOIL, silty sand													
178.31	Black													
0.61	SANDY SILT, Compact													
177.24	Brown to grey,													
1.68	CLAYEY SILT, laminated, some sand, numerous silt and sandy silt partings		2	SS	15									
	Silt to very stiff													
	Grey		3	SS	6									
174.50	CLAYEY SILT, some sand, trace gravel, silty sand pockets		4	TO	PH									
4.42	Soft to very stiff													
	Grey		5	SS	4									
			6	SS	5									
			7	TO	PH									
			8	SS	1									
			9	TO	PH									
			10	SS	3									
			11	TO	PH									

DRAFT

1 23 37 39
CICU
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LDN_MTO_2006 04-1111-060.GPJ GLDR_LON.GDT 6/14/07

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+³, X³: Numbers refer to Sensitivity ○ 3% STRAIN AT FAILURE

APPENDIX D
SITE PHOTOGRAPHS
DETROIT RIVER INTERNATIONAL CROSSING

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario

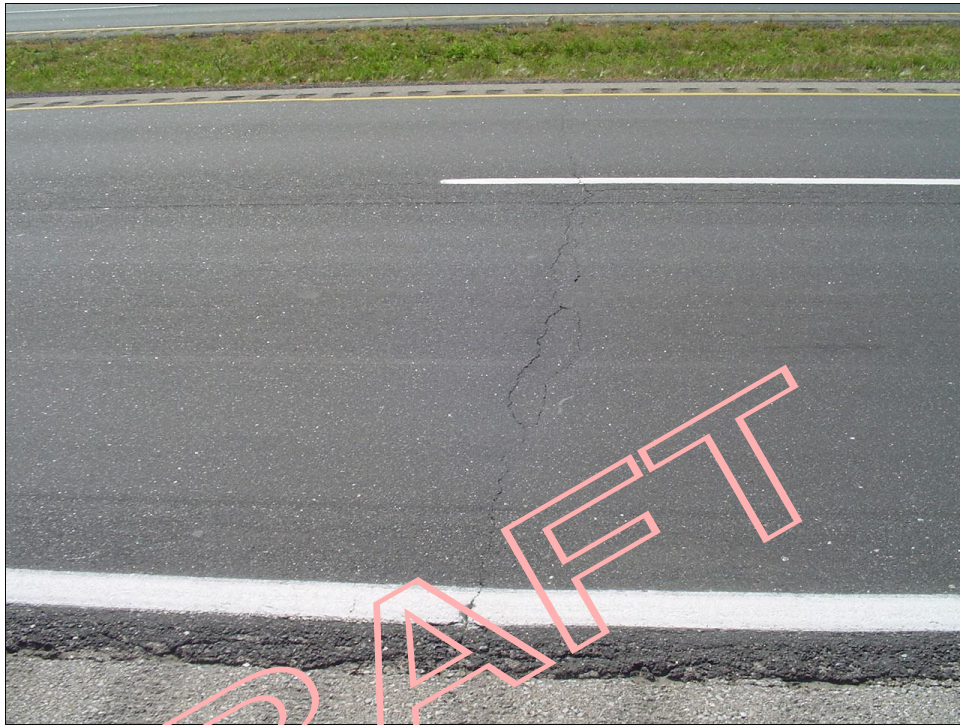


Photo 1 – Highway 401 westbound lanes between North Talbot Road and Huron Church Line. Typical transverse crack.



Photo 2 – Highway 401 westbound lanes just west of Talbot Road east. Note rutting and cracking at inner edge of pavement.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 3 – Howard Avenue - south of Talbot Line - looking south.



Photo 4 – Howard Avenue - south of Talbot Line - looking north.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 5 – Howard Avenue - north of Talbot Line - looking north.



Photo 6 – Howard Avenue - north of Talbot Line - looking south.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 7 – Cousineau Road - looking south.



Photo 8 – Sandwich Parkway - looking north.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 9 – Sandwich Parkway - looking south



Photo 10 – Cabana Road - looking north.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 11 – Cabana Road - looking south

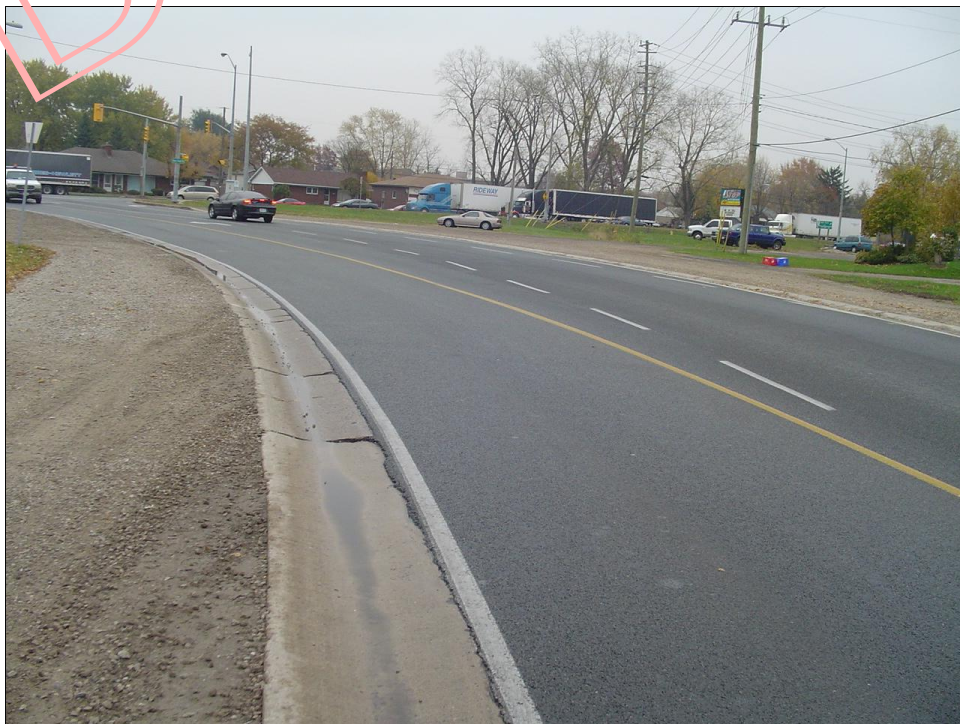


Photo 12 – Todd Lane - looking north

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 13 – Todd Lane - looking south



Photo 14 – Pulford Street - looking west.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 15 – Pulford Street - looking east.



Photo 16 – Pulford Street – concrete distress about 50 m south of Northway Ave.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 17 – Pulford Street - concrete distress about 20 m south of Northway Ave.



Photo 18 – Grand Marais Road West - looking east.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 19 – Grand Marais Road West - looking west.



Photo 20 – Lambton Street - looking east.

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 21 – Lambton Street - looking west



Photo 22 – Spring Garden Road - looking east

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 23 – Spring Garden Road - looking west



Photo 24 – Spring Garden Road – slight joint spalling

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 25 – Labelle Street - looking west



Photo 26 – Malden Road - south of E.C. Row Expressway - looking south

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 27 – Malden Road - north of E.C. Row Expressway - looking north



Photo 28– Malden Road - north of E.C. Row Expressway - looking south

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 29 – Malden Road – very severe transverse crack



Photo 30 – Malden Road – very severe longitudinal/meander crack

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 31 – Matchette Road - north of E.C. Row Expressway - looking south



Photo 32 – Matchette Road – north of E.C. Row Expressway – looking north

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 33 – Matchette Road – north of E.C. Row Expressway – alligator wheel path cracks



Photo 34 – Matchette Road – south of E.C. Row Expressway – looking north

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 35 – Matchette Road – south of E.C. Row Expressway – looking south



Photo 36 – Matchette Road – moderate longitudinal cracks

SITE PHOTOGRAPHS

Detroit River International Crossing Windsor, Ontario



Photo 37 – Ojibway Parkway - north of E.C. Row Expressway



Photo 38 – Ojibway Parkway - north of E.C. Row Expressway