



**Canada-United States-Ontario-Michigan
Border Transportation Partnership**

**Practical Alternatives Evaluation
Assessment Report**

Stormwater Management Plan

Appendices



July 2007
Revised December 2007

Appendices

Appendix A

Hydraulic Analysis Post Development Condition

Appendix A.1

Alternative 1A

Titcombe Drain Crossing
Basin Drain Crossing
Cahill Drain Crossing
Cahill / Wolfe Drainage Along Talbot Road

Titcombe Drain Worksheet for Circular Channel

Project Description	
Worksheet	Titcome_prelimin
Flow Element	Circular Channel
Method	Manning's Formu
Solve For	Full Flow Diametr

Input Data	
Mannings Coeffici	0.013
Channel Slope	005000 m/m
Discharge	3.2000 m ³ /s

Results	
Depth	1.27 m
Diameter	1,269.0 mm
Flow Area	1.3 m ²
Wetted Perimet	4.30 m
Top Width	0.00 m
Critical Depth	0.97 m
Percent Full	100.0 %
Critical Slope	005735 m/m
Velocity	2.53 m/s
Velocity Head	0.33 m
Specific Energy	1.60 m
Froude Number	0.00
Maximum Discr	3.4423 m ³ /s
Discharge Full	3.2000 m ³ /s
Slope Full	005000 m/m
Flow Type	N/A

Notes: Discharge of 3.2 m³/s was taken from taking 50% of the 100 year flow of subcatchment 140 of turkey creek watershed. Drainage area D/S of Titcombe is approximately 274 ha, 55% of the entire subcatch #140. Drainage area therefore U/S Titcombe crossing is app. 50% of the entire subcatch.
 From 1989 Maclaren report:
 Catch # 140
 DA = 496 Ha.
 100 yr existing = 6.4 m³/s
 100 yr efuture = 16.7 m³/s

Culvert Calculator Report

Basin Drain -All Alternatives - Reg Check

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	181.40 m	Headwater Depth/Height	1.20
Computed Headwater Elevat	180.32 m	Discharge	8.1000 m ³ /s
Inlet Control HW Elev.	180.32 m	Tailwater Elevation	179.70 m
Outlet Control HW Elev.	180.32 m	Control Type	Inlet Control

Grades			
Upstream Invert	178.50 m	Downstream Invert	178.20 m
Length	58.00 m	Constructed Slope	0.005172 m/m

Hydraulic Profile			
Profile	CompositeS1S2	Depth, Downstream	1.05 m
Slope Type	Steep	Normal Depth	1.05 m
Flow Regime	N/A	Critical Depth	1.14 m
Velocity Downstream	3.62 m/s	Critical Slope	0.004178 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.13 m
Section Size	2130 x 1520 mm	Rise	1.52 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	180.32 m	Upstream Velocity Head	0.57 m
Ke	0.20	Entrance Loss	0.11 m

Inlet Control Properties			
Inlet Control HW Elev.	180.32 m	Flow Control	Transition
Inlet Type	90° headwall w 45° bevels	Area Full	3.3 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Culvert Calculator Report

Lennon Drain - Alt1A-100yr-Existing

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	182.00 m	Headwater Depth/Height	1.40
Computed Headwater Elev.	181.91 m	Discharge	8.3000 m ³ /s
Inlet Control HW Elev.	181.91 m	Tailwater Elevation	180.85 m
Outlet Control HW Elev.	181.82 m	Control Type	Inlet Control

Grades			
Upstream Invert	180.20 m	Downstream Invert	179.30 m
Length	138.00 m	Constructed Slope	0.006522 m/m

Hydraulic Profile			
Profile	CompositePressureProfileS1S2	Depth, Downstream	0.82 m
Slope Type	N/A	Normal Depth	0.82 m
Flow Regime	N/A	Critical Depth	1.02 m
Velocity Downstream	3.92 m/s	Critical Slope	0.003567 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.59 m
Section Size	1219 mm x 2591 mm	Rise	1.22 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	181.82 m	Upstream Velocity Head	0.51 m
Ke	0.20	Entrance Loss	0.10 m

Inlet Control Properties			
Inlet Control HW Elev.	181.91 m	Flow Control	N/A
Inlet Type	90° headwall w 45° bevels	Area Full	3.2 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Culvert Calculator Report Cahill Alt 1A-Future

Comments: Unknown Flow

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	183.50 m	Headwater Depth/Height	1.97
Computed Headwater Elevat	183.26 m	Discharge	27.6000 m ³ /s
Inlet Control HW Elev.	183.26 m	Tailwater Elevation	180.23 m
Outlet Control HW Elev.	183.02 m	Control Type	Inlet Control

Grades			
Upstream Invert	180.31 m	Downstream Invert	179.33 m
Length	192.00 m	Constructed Slope	0.005104 m/m

Hydraulic Profile			
Profile	PressureProfile	Depth, Downstream	1.50 m
Slope Type	N/A	Normal Depth	N/A m
Flow Regime	N/A	Critical Depth	1.50 m
Velocity Downstream	4.09 m/s	Critical Slope	0.006085 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	4.50 m
Section Size	4500 x 1500 mm	Rise	1.50 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	183.02 m	Upstream Velocity Head	0.85 m
Ke	0.20	Entrance Loss	0.17 m

Inlet Control Properties			
Inlet Control HW Elev.	183.26 m	Flow Control	Submerged
Inlet Type	90° headwall w 45° bevels	Area Full	6.8 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Cahill/Wolfe Drain Along Talbot Road Worksheet for Rectangular Channel

Project Description	
Worksheet	Rectangular Channel - Cahill Drain (4.5 m)
Flow Element	Rectangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data	
Mannings Coefficient	0.013
Channel Slope	0.02400 m/m
Bottom Width	4.50 m
Discharge	12.1000 m ³ /s

Results	
Depth	0.94 m
Flow Area	4.2 m ²
Wetted Perimeter	6.38 m
Top Width	4.50 m
Critical Depth	0.90 m
Critical Slope	0.002689 m/m
Velocity	2.86 m/s
Velocity Head	0.42 m
Specific Energy	1.36 m
Froude Number	0.94
Flow Type	Subcritical

Cahill/Wolfe Drain Along Talbot Road Worksheet for Rectangular Channel

Project Description	
Worksheet	Rectangular Channel - Cahill Drain (4.5 m)
Flow Element	Rectangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data	
Mannings Coeffici	0.013
Channel Slope	002400 m/m
Bottom Width	4.50 m
Discharge	12.1000 m ³ /s

Results	
Depth	0.94 m
Flow Area	4.2 m ²
Wetted Perime	6.38 m
Top Width	4.50 m
Critical Depth	0.90 m
Critical Slope	0.002689 m/r
Velocity	2.86 m/s
Velocity Head	0.42 m
Specific Energ	1.36 m
Froude Numbe	0.94
Flow Type	Subcritical

Appendix A.2

Alternative 1B

Appendix A.2.1

Titcombe Drain Crossing

Titcombe Drain Worksheet for Circular Channel

Project Description	
Worksheet	Titcome_prelimin
Flow Element	Circular Channel
Method	Manning's Formu
Soive For	Full Flow Diametr

Input Data	
Mannings Coeffici	0.013
Channel Slope	005000 m/m
Discharge	3.2000 m ³ /s

Results	
Depth	1.27 m
Diameter	1,269.0 mm
Flow Area	1.3 m ²
Wetted Perimet	4.30 m
Top Width	0.00 m
Critical Depth	0.97 m
Percent Full	100.0 %
Critical Slope	005735 m/m
Velocity	2.53 m/s
Velocity Head	0.33 m
Specific Energy	1.60 m
Froude Number	0.00
Maximum Disct	3.4423 m ³ /s
Discharge Full	3.2000 m ³ /s
Slope Full	005000 m/m
Flow Type	N/A

Notes: Discharge of 3.2 m³/s was taken from taking 50% of the 100 year flow of subcatchment 140 of turkey creek watershed. Drainage area D/S of Titcombe is approximately 274 ha, 55% of the entire subcatch #140. Drainage area therefore U/S Titcombe crossing is app. 50% of the entire subcatch.
 From 1989 Maclaren report:
 Catch # 140
 DA = 496 Ha.
 100 yr existing = 6.4 m³/s
 100 yr efuture = 16.7 m³/s

Appendix A.2.2

Basin Drain Crossing

Culvert Calculator Report

Basin Drain -All Alternatives - Reg Check

Solve For: Headwater Elevation

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Computed Headwater Elevat	180.32 m	Discharge	8.1000 m ³ /s
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Outlet Control HW Elev.	180.32 m	Control Type	Inlet Control

Grades			
Upstream Invert	178.50 m	Downstream Invert	178.20 m
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Hydraulic Profile			
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Flow Regime	N/A	Critical Depth	1.14 m
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Section			
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Outlet Control Properties			
Outlet Control HW Elev.	180.32 m	Upstream Velocity Head	0.57 m
Ke	0.20	Entrance Loss	0.11 m

Inlet Control Properties			
Inlet Control HW Elev.	180.32 m	Flow Control	Transition
Inlet Type	90° headwall w 45° bevels	Area Full	3.3 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Appendix A.2.3

Cahill / Wolfe Drainage Along Talbot Road

Cahill/Wolfe Drain Along Talbot Road Worksheet for Rectangular Channel

Project Description	
Worksheet	Rectangular Channel - Cahill Drain (4.5 >
Flow Element	Rectangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data	
Mannings Coeffici	0.013
Channel Slope	002400 m/m
Bottom Width	4.50 m
Discharge	12.1000 m ³ /s

Results	
Depth	0.94 m
Flow Area	4.2 m ²
Wetted Perime	6.38 m
Top Width	4.50 m
Critical Depth	0.90 m
Critical Slope	0.002689 m/r
Velocity	2.86 m/s
Velocity Head	0.42 m
Specific Energ	1.36 m
Froude Numbe	0.94
Flow Type	Subcritical

Appendix A.2.4

Syphon Analysis

Appendix A.2.4.1

Turkey Creek

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*****
* U.S. Environmental Protection Agency *
* Storm Water Management Model (SWM) *
* Version 4.4GU *
* *
* CDM/OSU Beta *
* Release Date - November 23, 1999 *
* Camp Dresser & McKee and Oregon State Univ. *
* Chuck Moore and Wayne Huber *
* Compiled using Digital Visual Fortran 6.0 *
*****

```

Developed by

```

*****
* Metcalf & Eddy, Inc. *
* University of Florida *
* Water Resources Engineers, Inc. *
* (Now Camp, Dresser and McKee, Inc.) *
* September 1970 *
*****

```

Distributed and Maintained by

```

*****
* U.S. Environmental Protection Agency *
* Center for Exposure Assessment Modeling (CEAM) *
* Athens Environmental Research Laboratory *
* 960 College Station Road *
* Athens, GA 30605-2720 *
*****

```

```

*****
* This is a new release of SWMM. If any *
* problems occur executing this model *
* system, contact Mr. Frank Stancil, *
* U.S. Environmental Protection Agency. *
* 706/355-8328 (voice) *
* e-mail: stancil@athens.ath.epa.gov *
* Or contact Wayne C. Huber at Oregon St. U. *
* 541/737-6150 or wayne.huber@orst.edu *
* Or Michael F. Schmidt at Camp Dresser & *
* McKee (904) 281-0170 SCHMIDTF@CDM.COM *
*****

```



```

*****
* This is an implementation of EPA SWMM 4.4GU *
* "Nature is full of infinite causes which *
* have never occurred in experience" da Vinci *
*****

```

```

#####
# File names by SWMM Block #
# JIN -> Input to a Block #
# JOUT -> Output from a Block #
#####

```

```

JIN for Block # 1 File # 0 JIN.UF
JOUT for Block # 1 File # 9 PCTmpl.int

```

```

#####
# Scratch file names for this simulation. #
#####

```

```

NSCRAT # 1 File # 21 SCRT1.UF
NSCRAT # 2 File # 22 SCRT2.UF
NSCRAT # 3 File # 23 SCRT3.UF
NSCRAT # 4 File # 24 SCRT4.UF
NSCRAT # 5 File # 25 SCRT5.UF
NSCRAT # 6 File # 26 SCRT6.UF
NSCRAT # 7 File # 27 SCRT7.UF
NSCRAT # 8 File # 28 SCRT8.UF

```

```

*****
+ Parameter Values on the Tapes Common Block *
*****

```

```

Number of Subcatchments in the Runoff Block (NW)..... 1000
Number of Channel/Pipes in the Runoff Block (NG)..... 1000
Number of Connections to Runoff Channels/Inlets (NCP). 6
Number of Water Quality Constituents (MQUAL)..... 20
Number of Runoff Land Uses per Subcatchment (NLU)..... 20
Number of Groundwater Subcatchments in Runoff (HGW)... 100
Number of Interface Locations for all Blocks (NIE).... 1000
Number of Elements in the Transport Block (NET)..... 500
Number of Storage Junctions in Transport (NTSE)..... 100
Number of Transport interface input locations (NTHI).. 500
Number of Transport interface output locations (NTHO). 500
Number of Transport input locations on R lines (NTHR). 80

```

Number of Transport printed output locations (NTOA)... 80
 Number of Tabular Flow Splitters in Transport (NTSP).. 50
 Number of Elements in the Extran Block (NEE)..... 4000
 Number of Pumps in Extran (NEP)..... 75
 Number of Orifices in Extran (NEO)..... 200
 Number of Tide Gates/Free Outfalls in Extran (NTG).... 200
 Number of Extran Weirs (NEW)..... 400
 Number of Extran Printout Locations (NPO)..... 150
 Number of Tide Elements in Extran (NTE)..... 50
 Number of Natural Channels (NNC)..... 1200
 Number of Storage Junctions in Extran (NVSE)..... 1000
 Number of Time History Data Points in Extran (NTVAL).. 500
 Number of Data Points for Variable Storage Elements
 in the Extran Block (NVST)..... 25
 Number of Input Hydrographs in Extran (NEH)..... 500
 Number of Allowable Channel Connections to
 Junctions in the Extran Block (NCHN)..... 15
 Number Rain Gages in Rain and Runoff (MAXRG)..... 200
 Number PRATE/VRATE Points for Extran Pump
 Input (MAXPRA)..... 10
 Number of Variable Orifices in Extran (NVORF)..... 50
 Number of Variable Orifice Data Points (NVOTIM)..... 50
 Number of Allowable Precip. Values/yr in Rain (LIMRN). 5000
 Number of Storm Events for Rain Analysis (LSTORM).....20000
 Number of Plugs for Plug-flow in S/T (NPLUG)..... 3000
 Number Conduits for Extran Results to ASCII
 File (MXFLOW)..... 400

 * Entry made to the EXTENDED TRANSPORT MODEL (EXTRAN) *
 * developed 1973 by Camp, Dresser and McKee (CDM) with *
 * modifications 1977-1991 by the University of Florida. *
 *
 * Most recent update: March 1999 by CDM, Oregon *
 * State University, and XP Software, Inc. *
 *
 * "Smooth runs the water where the brook is deep." *
 * Shakespeare, Henry VI, II, III, 1 *

WASHINGTON, D.C.

ANALYSIS MODULE

CAMP DRESSER & MCKEE INC.
ANNANDALE, VIRGINIA

Detroit River International Crossing
Turkey Creek Existing Condition - 100-Year Flow

Control information for simulation

Integration cycles..... 1440

Length of integration step is..... 5.00 seconds

Simulation length..... 2.00 hours

Do not create equiv. pipes(NEQUAL). 0

Use metric units for I/O..... 1

Printing starts in cycle..... 1

Intermediate printout intervals of. 1 cycles

Intermediate printout intervals of. 0.08 minutes

Summary printout intervals of..... 1 cycles

Summary printout time interval of.. 0.08 minutes

Hot start file parameter (JREDO)... 0

Initial time (TZERO)..... 0.00 hours

This is time displacement from JIN interface file starting date/time when interface file is used.

This also describes starting hour in K3 line hydrograph input when K3 lines are used.

Initial date (IDATZ)..... 19060714 (yr/mo/day)

NOTE: Initial date from JIN interface file will be used, if accessed, unless IDATZ is negative.

Iteration variables: ITMAX..... 30

SURTOL..... 0.0500

Default surface area of junctions.. 1.22 square meters.

EXTRAN VERSION 3.3 SOLUTION. (ISOL = 0).

Sum of junction flow is zero during surcharge.

NORMAL FLOW OPTION WHEN THE WATER SURFACE SLOPE IS LESS THAN THE GROUND SURFACE SLOPE (KSUPER=0)....

NJSW INPUT HYDROGRAPH JUNCTIONS.... 0

INTERMEDIATE HEADER LINES ARE PRINTED AS IN ORIGINAL PROGRAM

IDS ARE WRITTEN AS IN ORIGINAL PROGRAM

CONDUIT LENGTHS ON C1 LINE MUST EQUAL IRREGULAR SECTION LENGTH ENTERED ON THE C3 OR X1 LINES (IWLEN = 0)

JELEV = 0 (DEFAULT). STANDARD INPUTS ARE DEPTHS NOT ELEVATIONS

JDOWN = 0 - Minimum of normal or critical depth will be used at free outfalls (II).

Characteristic depth for M2 and S2 water surface profiles will be computed as in previous versions of EXTRAN (IM2 = 0).

SEDIMENT DEPTHS WILL NOT BE READ FROM C1 LINES

Intermediate continuity output will not be created

1-----

ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
 ***** ANALYSIS MODULE ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Turkey Creek Existing Condition - 100-Year Flow

1 *****
 * Conduit Data *

INP NUM	CONDUIT NUMBER	LENGTH (M)	CONDUIT CLASS	AREA (SQ M)	MANNING MAX WIDTH COEF.	DEPTH (M)	JUNCTIONS AT THE ENDS	INVERT HEIGHT ABOVE JUNCTIONS	TRAPEZOID SIDE SLOPES
1	1	100.	TRAPEZOID	96.91	0.01500	4.64	1	2.40	2.40
2	2	100.	TRAPEZOID	96.91	0.01500	4.64	2	2.40	2.40
3	3	100.	TRAPEZOID	96.91	0.01500	4.64	3	2.40	2.40
4	4	60.	TRAPEZOID	96.91	0.01500	4.64	4	2.40	2.40
5	5	40.	TRAPEZOID	96.91	0.01500	4.64	5	2.40	2.40

6	100. TRAPEZOID	96.91	0.01500	9.75	4.64	6	7	2.40	2.40
7	100. TRAPEZOID	96.91	0.01500	9.75	4.64	7	8	2.40	2.40
8	100. TRAPEZOID	96.91	0.01500	9.75	4.64	8	9	2.40	2.40
9	100. TRAPEZOID	96.91	0.01500	9.75	4.64	9	10	2.40	2.40
10	100. TRAPEZOID	96.91	0.01500	9.75	4.64	10	11	2.40	2.40

 + Conduit Volume +

Input full depth volume..... 8.7220E+04 cubic meters

1-----
 ENVIRONMENTAL PROTECTION AGENCY +*** EXTENDED TRANSPORT PROGRAM +*** WATER RESOURCES DIVISION
 WASHINGTON, D.C. +*** CAMP DRESSER & MCREE INC. +***
 +*** ANALYSIS MODULE +*** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Turkey Creek Existing Condition - 100-Year Flow

1

 + Junction Data +

INP NUM	JUNCTION NUMBER	GROUND ELEV.	CROWN ELEV.	INVERT ELEV.	QINST CMS	INITIAL DEPTH(M)	CONNECTING CONDUITS
1	1	182.00	180.68	176.04	62.60	0.00	1
2	2	182.00	180.64	176.00	0.00	0.00	1 2
3	3	182.00	180.60	175.96	0.00	0.00	2 3
4	4	182.00	180.56	175.92	0.00	0.00	3 4
5	5	182.00	180.54	175.90	0.00	0.00	4 5
6	6	182.00	180.52	175.88	0.00	0.00	5 6
7	7	182.00	180.48	175.84	0.00	0.00	6 7
8	8	182.00	180.44	175.80	0.00	0.00	7 8
9	9	182.00	180.40	175.76	0.00	0.00	8 9
10	10	182.00	180.36	175.72	0.00	0.00	9 10
11	11	182.00	180.32	175.68	0.00	0.00	10

* FREE OUTFALL DATA (DATA GROUP I1) *
* BOUNDARY CONDITION ON DATA GROUP J1 *

OUTFALL AT JUNCTION... 11 HAS BOUNDARY CONDITION NUMBER... 1

1-----
ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
WASHINGTON, D.C. *****
***** ANALYSIS MODULE *****
ANNANDALE, VIRGINIA

Detroit River International Crossing
Turkey Creek Existing Condition - 100-Year Flow

* INTERNAL CONNECTIVITY INFORMATION *

CONDUIT JUNCTION JUNCTION

90011 11 0

* BOUNDARY CONDITION INFORMATION *
* DATA GROUPS J1-J4 *

BC NUMBER.. 1 CONTROL WATER SURFACE ELEVATION IS.. 178.80 METERS.
TZERO = 1906195 0.0000000E+00

* INITIAL MODEL CONDITION *
* INITIAL TIME = 0.00 HOURS *

JUNCTION / DEPTH / ELEVATION ==> "!" JUNCTION IS SURCHARGED.
1/ 0.00 / 176.04 2/ 0.00 / 176.00 3/ 0.00 / 175.96
4/ 0.00 / 175.92 5/ 0.00 / 175.90 6/ 0.00 / 175.88
7/ 0.00 / 175.84 8/ 0.00 / 175.80 9/ 0.00 / 175.76
10/ 0.00 / 175.72 11/ 0.00 / 175.68

CONDUIT/ FLOW ==> "*" CONDUIT USES THE NORMAL FLOW OPTION.

1/	0.00	2/	0.00	3/	0.00	4/	0.00
5/	0.00	6/	0.00	7/	0.00	8/	0.00
9/	0.00	10/	0.00	90011/	0.00		

CONDUIT/ VELOCITY

1/	0.00	2/	0.00	3/	0.00	4/	0.00
5/	0.00	6/	0.00	7/	0.00	8/	0.00
9/	0.00	10/	0.00				

CONDUIT/ CROSS SECTIONAL AREA

1/	0.00	2/	0.00	3/	0.00	4/	0.00
5/	0.00	6/	0.00	7/	0.00	8/	0.00
9/	0.00	10/	0.00				

CONDUIT/ HYDRAULIC RADIUS

1/	0.00	2/	0.00	3/	0.00	4/	0.00
5/	0.00	6/	0.00	7/	0.00	8/	0.00
9/	0.00	10/	0.00				

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/	176.04/	176.00	2/	176.00/	175.96	3/	175.96/	175.92
4/	175.92/	175.90	5/	175.90/	175.88	6/	175.88/	175.84
7/	175.84/	175.80	8/	175.80/	175.76	9/	175.76/	175.72
10/	175.72/	175.68						

 * FINAL MODEL CONDITION *
 * FINAL TIME = 2.00 HOURS *

>>> ENDING DATE AND TIME OF EXTRAN RUN ARE:

JULIAN DATE: 1906195
 YR/MO/DA: 1906/ 7/14
 TIME OF DAY: 2.000 HRS

JUNCTION / DEPTH / ELEVATION ==> "*" JUNCTION IS SURCHARGED.

1/	2.86 /	178.90	2/	2.89 /	178.89	3/	2.92 /	178.88
4/	2.95 /	178.87	5/	2.96 /	178.86	6/	2.97 /	178.85
7/	3.00 /	178.84	8/	3.03 /	178.83	9/	3.06 /	178.82
10/	3.09 /	178.81	11/	3.12 /	178.80			

CONDUIT/ FLOW ==> "*" CONDUIT USES THE NORMAL FLOW OPTION.

1/	62.60	2/	62.59	3/	62.61	4/	62.60
----	-------	----	-------	----	-------	----	-------

5/ 62.59 6/ 62.60 7/ 62.60 8/ 62.59

9/ 62.60 10/ 62.59 90011/ 62.59

CONDUIT/ VELOCITY

1/ 1.31 2/ 1.29 3/ 1.27 4/ 1.26
 5/ 1.25 6/ 1.24 7/ 1.22 8/ 1.20
 9/ 1.19 10/ 1.17

CONDUIT/ CROSS SECTIONAL AREA

1/ 47.94 2/ 48.57 3/ 49.22 4/ 49.69
 5/ 50.02 6/ 50.55 7/ 51.24 8/ 51.95
 9/ 52.67 10/ 53.41

CONDUIT/ FINAL VOLUME

1/ 4793.90 2/ 4856.83 3/ 4921.51 4/ 2981.39
 5/ 2000.85 6/ 5055.19 7/ 5124.48 8/ 5195.12
 9/ 5267.39 10/ 5340.97

CONDUIT/ HYDRAULIC RADIUS

1/ 1.94 2/ 1.95 3/ 1.97 4/ 1.98
 5/ 1.99 6/ 2.00 7/ 2.01 8/ 2.03
 9/ 2.05 10/ 2.06

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/ 178.90/ 178.89 2/ 178.89/ 178.88 3/ 178.88/ 178.87
 4/ 178.87/ 178.86 5/ 178.86/ 178.85 6/ 178.85/ 178.84
 7/ 178.84/ 178.83 8/ 178.83/ 178.82 9/ 178.82/ 178.81
 10/ 178.81/ 178.80

 # Surcharge Iteration Summary #
 #####

Maximum number of iterations in a time step..... 22
 Total number of iterations in the simulation.. 3151
 Average number of iterations per time step..... 2.19
 Surcharge iterations during the simulation..... 271
 Maximum surcharge flow error during simulation.. 4.73E+00 cms
 Total number of time steps during simulation.. 1440

* CONDUIT COURANT CONDITION SUMMARY *
 * TIME IN MINUTES DELT > COURANT TIME STEP *
 * SEE BELOW FOR EXPLANATION OF COURANT TIME STEP. *

CONDUIT #	TIME (MN)	CONDUIT #	TIME (MN)	CONDUIT #	TIME (MN)	CONDUIT #	TIME (MN)
1	0.00	2	0.00	3	0.00	4	0.00
5	0.33	6	0.00	7	0.00	8	0.00
9	0.00	10	0.00				

1
 * CONDUIT COURANT CONDITION SUMMARY *
 * COURANT = CONDUIT LENGTH *
 * TIME STEP = VELOCITY + SQRT (GRVT*AREA/WIDTH) *
 * AVERAGE COURANT CONDITION TIME STEP (SECONDS) *

CONDUIT #	TIME (SEC)	CONDUIT #	TIME (SEC)	CONDUIT #	TIME (SEC)	CONDUIT #	TIME (SEC)
1	17.23	2	17.25	3	17.91	4	12.67
5	8.85	6	22.33	7	17.08	8	17.08
9	17.08	10	16.96				

1
 * EXTRAN CONTINUITY BALANCE AT THE LAST TIME STEP *
 * JUNCTION INFLOW, OUTFLOW OR STREET FLOODING *

JUNCTION	INFLOW, CU M
1	4.5072E+05

JUNCTION	OUTFLOW, CU M
1	2.6693E+03

```

2 5.6062E+02
3 2.0810E+02
4 1.8592E+03
5 5.6534E+03
6 1.3183E+03
7 9.4449E+02
8 2.9801E+03
9 1.3814E+03
10 1.5085E+03
11 3.9284E+05

*****
* INITIAL SYSTEM VOLUME = 9.0000E-03 CU M *
* TOTAL SYSTEM INFLOW VOLUME = 4.5072E+05 CU M *
* INFLOW + INITIAL VOLUME = 4.5072E+05 CU M *
*****
* TOTAL SYSTEM OUTFLOW = 4.1192E+05 CU M *
* VOLUME LEFT IN SYSTEM = 4.5538E+04 CU M *
* OUTFLOW + FINAL VOLUME = 4.5746E+05 CU M *
*****
* ERROR IN CONTINUITY, PERCENT = -1.50 *
*****

```

```

TEST WRITE OF ALTERNATIVE CONTINUITY ERROR CALCULATION
VOLUME LEFT IN SYSTEM = 6.7482E+04 CU. FT.
ERROR IN CONTINUITY PERCENT = -63.64

```

----- SUMMARY OF FULL FLOW CHANNEL WARNINGS -----

OPEN CHANNEL NUMBER	TIME STEP OF FIRST OCCURRENCE	TIME OF FIRST OCCURRENCE (HOURS)	TIME STEP OF LAST OCCURRENCE	TIME OF LAST OCCURRENCE (HOURS)
1	1	0.00	84	0.12
2	38	0.05	82	0.11
3	31	0.04	81	0.11
4	27	0.04	81	0.11
5	26	0.04	74	0.10
6	24	0.03	70	0.10
7	20	0.03	70	0.10
8	17	0.02	59	0.08
9	15	0.02	61	0.08
10	15	0.02	61	0.08

THE PROGRAM USES FULL DEPTH CHANNEL CHARACTERISTICS TO COMPUTE FLOW THROUGH THE TRAPEZOIDAL, IRREGULAR, OR PARABOLIC/POWER FUNCTION CONDUIT WHEN THE COMPUTED DEPTHS EXCEED MAXIMUM DEPTH. THIS WILL AFFECT THE MAXIMUM COMPUTED HEAD AND FLOWS IN THE MODEL. IT IS HIGHLY RECOMMENDED THAT THE MODELED CROSS SECTIONS BE EXTENDED TO ELIMINATE THESE FULL FLOW CHANNEL WARNINGS

1

 * JUNCTION SUMMARY STATISTICS *

Detroit River International Crossing
 Turkey Creek Existing Condition - 100-Year Flow

JUNCTION NUMBER	GROUND ELEVATION (M)	UPPERMOST PIPE CROWN ELEVATION (M)	MEAN JUNCTION ELEVATION (M)	JUNCTION AVERAGE ELEV. CHANGE (%)	MAXIMUM JUNCTION ELEV. (M)	TIME OF OCCURRENCE HR. MIN.	METERS OF SURCHARGE AT MAX ELEVATION	METERS MAX. DEPTH IS BELOW GROUND ELEVATION	LENGTH OF SURCHARGE (MIN)	LENGTH OF FLOODING (MIN)	MAXIMUM JUNCTION AREA (SQ.MET)
1	182.00	180.68	178.96	0.3566	182.00	0 3	1.32	0.00	0.8	0.3	3.522E+04
2	182.00	180.64	178.94	0.3323	182.00	0 3	1.36	0.00	0.5	0.1	6.990E+04
3	182.00	180.60	178.91	0.2984	182.00	0 5	1.40	0.00	0.4	0.1	5.033E+04
4	182.00	180.56	178.89	0.3581	182.00	0 2	1.44	0.00	0.9	0.2	4.026E+04
5	182.00	180.54	178.88	0.3984	182.00	0 2	1.46	0.00	0.7	0.3	3.403E+04
6	182.00	180.52	178.87	0.3937	182.00	0 2	1.48	0.00	0.3	0.3	4.789E+04
7	182.00	180.48	178.85	0.3333	182.00	0 2	1.52	0.00	0.3	0.2	3.544E+04
8	182.00	180.44	178.84	0.3701	182.00	0 1	1.56	0.00	0.7	0.3	2.120E+04
9	182.00	180.40	178.83	0.3194	182.00	0 1	1.60	0.00	0.3	0.2	4.340E+04
10	182.00	180.36	178.82	0.3523	182.00	0 1	1.64	0.00	0.3	0.3	3.099E+04
11	182.00	180.32	178.81	0.0960	180.32	0 1	0.00	1.68	0.4	0.0	1.458E+04

1

 * CONDUIT SUMMARY STATISTICS *

Detroit River International Crossing
 Turkey Creek Existing Condition - 100-Year Flow

CONDUIT DESIGN FLOW	DESIGN FLOW VELOCITY	VERTICAL DEPTH	CONDUIT VERTICAL DEPTH	MAXIMUM COMPUTED FLOW	TIME OF OCCURRENCE	MAXIMUM COMPUTED VELOCITY	TIME OF OCCURRENCE	RATIO OF MAX. TO DESIGN	MAXIMUM DEPTH ABOVE INV. AT CONDUIT ENDS UPSTREAM	LENGTH OF CONDUIT ABOVE DOWNSTREAM	NORM SLOPE OF FLOW
182.00	1.32	0.00	0.00	182.00	0 3	1.32	0 3	1.00	0.8	0.3	3.522E+04
182.00	1.36	0.00	0.00	182.00	0 3	1.36	0 3	1.00	0.5	0.1	6.990E+04
182.00	1.40	0.00	0.00	182.00	0 5	1.40	0 5	1.00	0.4	0.1	5.033E+04
182.00	1.44	0.00	0.00	182.00	0 2	1.44	0 2	1.00	0.9	0.2	4.026E+04
182.00	1.46	0.00	0.00	182.00	0 2	1.46	0 2	1.00	0.7	0.3	3.403E+04
182.00	1.48	0.00	0.00	182.00	0 2	1.48	0 2	1.00	0.3	0.3	4.789E+04
182.00	1.52	0.00	0.00	182.00	0 2	1.52	0 2	1.00	0.3	0.2	3.544E+04
182.00	1.56	0.00	0.00	182.00	0 1	1.56	0 1	1.00	0.7	0.3	2.120E+04
182.00	1.60	0.00	0.00	182.00	0 1	1.60	0 1	1.00	0.3	0.2	4.340E+04
182.00	1.64	0.00	0.00	182.00	0 1	1.64	0 1	1.00	0.3	0.3	3.099E+04
180.32	0.00	1.68	0.00	180.32	0 1	0.00	0 1	1.68	0.4	0.0	1.458E+04

NUMBER	(CMS)	(M/S)	(M)	(CMS)	HR.	MIN.	(MPS)	HR.	MIN.	FLOW	(M)	(M)	(MIN)	(M/M)
1	2.60E+02	2.69	4.640	-1.95E+02	0	3	4.06	0	0	-0.75	5.96	5.53	0.5	0.00040
2	2.60E+02	2.69	4.640	-2.28E+02	0	3	4.15	0	1	-0.88	5.53	6.04	0.3	0.00040
3	2.60E+02	2.69	4.640	-2.47E+02	0	3	4.14	0	1	-0.95	6.04	6.08	0.3	0.00040
4	2.38E+02	2.45	4.640	-4.24E+02	0	2	-4.77	0	2	-1.78	6.08	6.10	0.3	0.00033
5	2.91E+02	3.00	4.640	-7.58E+02	0	2	-8.07	0	2	-2.60	6.10	6.12	0.1	0.00050
6	2.60E+02	2.69	4.640	-5.79E+02	0	2	-6.64	0	2	-2.22	6.12	6.08	0.2	0.00040
7	2.60E+02	2.69	4.640	-7.19E+02	0	1	-8.32	0	2	-2.76	6.08	6.20	0.3	0.00040
8	2.60E+02	2.69	4.640	-7.36E+02	0	1	-8.37	0	1	-2.83	6.20	6.24	0.2	0.00040
9	2.60E+02	2.69	4.640	-7.29E+02	0	1	-7.76	0	1	-2.80	6.24	6.28	0.3	0.00040
10	2.60E+02	2.69	4.640	-7.66E+02	0	1	-7.96	0	1	-2.94	6.28	4.64	0.2	0.00040
90011	UNDEF	UNDEF	UNDEF	-7.66E+02	0	1								

1

 * SUBCRITICAL AND CRITICAL FLOW ASSUMPTIONS FROM *
 * SUBROUTINE HEAD. SEE FIGURE 5-4 IN THE EXTRAN *
 * MANUAL FOR FURTHER INFORMATION. *

CONDUIT NUMBER	LENGTH OF DRY FLOW (MIN)	LENGTH OF SUBCRITICAL FLOW (MIN)	LENGTH OF UPSTR. OF CRITICAL FLOW (MIN)	LENGTH OF DOWNSTR. CRITICAL FLOW (MIN)	MEAN FLOW (CMS)	AVERAGE % CHANGE	TOTAL FLOW CUBIC MET	HYDRAULIC RADIUS (MET)	MAXIMUM CROSS SECT AREA (SQ.M)
1	0.00	120.00	0.00	0.00	62.38	0.4018	4.4914E+05	2.8606	96.9110
2	0.08	119.92	0.00	0.00	61.55	0.4045	4.4315E+05	2.8317	95.0900
3	0.08	119.92	0.00	0.00	60.79	0.5007	4.3772E+05	2.8606	96.9110
4	0.25	119.75	0.00	0.00	60.30	0.7906	4.3419E+05	2.8499	96.2355
5	0.58	119.42	0.00	0.00	59.26	0.7936	4.2664E+05	2.8606	96.9110
6	0.92	119.08	0.00	0.00	58.48	0.6124	4.2108E+05	2.7665	91.0556
7	1.00	119.00	0.00	0.00	57.33	0.7272	4.1280E+05	2.8039	93.3605
8	0.58	119.42	0.00	0.00	56.72	0.7770	4.0839E+05	2.7152	87.9453
9	0.17	119.83	0.00	0.00	55.68	0.6837	4.0092E+05	2.8136	93.9603
10	0.00	120.00	0.00	0.00	54.56	0.6524	3.9284E+05	2.8606	96.9110
90011	UNDEFINED	UNDEFINED	UNDEFINED	UNDEFINED	54.56		3.9284E+05		

 * AVERAGE % CHANGE IN JUNCTION OR CONDUIT IS DEFINED AS: *
 * CONDUIT % CHANGE ==> 100.0 (Q(n+1) - Q(n)) / Qfull *
 * JUNCTION % CHANGE ==> 100.0 (Y(n+1) - Y(n)) / Yfull *

The Conduit with the largest average change... 5 had 0.794 percent

The Junction with the largest average change... 5 had 0.398 percent

==> Extended Transport model simulation ended normally.

==> SWMM 4.4GU simulation ended normally.

Always check output file for possible warning messages.

==> Your input file was named : PCTmpl.dat

==> Your output file was named: PCTmpl.out

```
*****
* SWMM 4.4GU Simulation Date and Time Summary *
*****
* Starting Date... November 16, 2006 *
* Time... 16:24:48.900 *
* Ending Date... November 16, 2006 *
* Time... 16:24:53.510 *
* Elapsed Time... 0.077 minutes. *
* Elapsed Time... 4.611 seconds. *
*****
```

```

*****
* U.S. Environmental Protection Agency *
* Storm Water Management Model (SWM2) *
* Version 4.4GU *
* *
* CDM/OSU Beta *
* Release Date - November 23, 1999 *
* Camp Dresser & McKee and Oregon State Univ. *
* Chuck Moore and Wayne Huber *
* Compiled using Digital Visual Fortran 6.0 *
*****

```

Developed by

```

*****
* Metcalf & Eddy, Inc. *
* University of Florida *
* Water Resources Engineers, Inc. *
* (Now Camp, Dresser and McKee, Inc.) *
* September 1970 *
*****

```

Distributed and Maintained by

```

*****
* U.S. Environmental Protection Agency *
* Center for Exposure Assessment Modeling (CEAM) *
* Athens Environmental Research Laboratory *
* 960 College Station Road *
* Athens, GA 30605-2720 *
*****

```

```

*****
* This is a new release of SWMM. If any *
* problems occur executing this model *
* system, contact Mr. Frank Stancil, *
* U.S. Environmental Protection Agency. *
* 706/355-8328 (voice) *
* e-mail: stancil@athens.ath.epa.gov *
* Or contact Wayne C. Huber at Oregon St. U. *
* 541/737-6150 or wayne.huber@orst.edu *
* Or Michael F. Schmidt at Camp Dresser & *
* McKee (904) 281-0170 SCHMIDTM@CDM.COM *
*****

```

```

*****
* This is an implementation of EPA SWMM 4.4GU *
* "Nature is full of infinite causes which *
* have never occurred in experience" da Vinci *
*****

```

```

#####
# File names by SWMM Block #
# JIN -> Input to a Block #
# JOUT -> Output from a Block #
#####

```

```

JIN for Block # 1 File # 0 JIN.UF
JOUT for Block # 1 File # 9 PCTmpl.int

```

```

#####
# Scratch file names for this simulation. #
#####

```

```

NSCRAT # 1 File # 21 SCRT1.UF
NSCRAT # 2 File # 22 SCRT2.UF
NSCRAT # 3 File # 23 SCRT3.UF
NSCRAT # 4 File # 24 SCRT4.UF
NSCRAT # 5 File # 25 SCRT5.UF
NSCRAT # 6 File # 26 SCRT6.UF
NSCRAT # 7 File # 27 SCRT7.UF
NSCRAT # 8 File # 28 SCRT8.UF

```

```

*****
* Parameter Values on the Tapes Common Block *
*****

```

```

Number of Subcatchments in the Runoff Block (RW)..... 1000
Number of Channel/Pipes in the Runoff Block (NG)..... 1000
Number of Connections to Runoff Channels/Inlets (NCP). 6
Number of Water Quality Constituents (MQUAL)..... 20
Number of Runoff Land Uses per Subcatchment (NLU).... 20
Number of Groundwater Subcatchments in Runoff (NGW)... 100
Number of Interface Locations for all Blocks (NIE).... 1000
Number of Elements in the Transport Block (NET)..... 500
Number of Storage Junctions in Transport (NTSE)..... 100
Number of Transport Interface input locations (NTHI).. 500
Number of Transport interface output locations (NTHO). 500
Number of Transport input locations on R lines (NTHR). 80

```

Number of Transport printed output locations (NTOA).... 80
 Number of Tubular Flow Splitters in Transport (WTSP).. 50
 Number of Elements in the Extran Block (NEE)..... 4000
 Number of Pumps in Extran (NEP)..... 75
 Number of Orifices in Extran (NEO)..... 200
 Number of Tide Gates/Free Outfalls in Extran (NTG).... 200
 Number of Extran Weirs (NEW)..... 400
 Number of Extran Printout Locations (NPO)..... 150
 Number of Tide Elements in Extran (NTE)..... 50
 Number of Natural Channels (NNC)..... 1200
 Number of Storage Junctions in Extran (NVSE)..... 1000
 Number of Time History Data Points in Extran (NTVAL).. 500
 Number of Data Points for Variable Storage Elements
 in the Extran Block (NVST)..... 25
 Number of Input Hydrographs in Extran (NEH)..... 500
 Number of Allowable Channel Connections to
 Junctions in the Extran Block (NCHN)..... 15
 Number Rain Gages in Rain and Runoff (MAXRG)..... 200
 Number PRATE/VRATE Points for Extran Pump
 Input (MAXPRA)..... 10
 Number of Variable Orifices in Extran (NVORE)..... 50
 Number of Variable Orifice Data Points (NVOTIM)..... 50
 Number of Allowable Precip. Values/yr in Rain (LIMRN). 5000
 Number of Storm Events for Rain Analysis (LSTORM).....20000
 Number of Plugs for Plug-flow in S/T (NPLUG)..... 3000
 Number Conduits for Extran Results to ASCII
 File (MXFLOW)..... 400

 * Entry made to the EXTENDED TRANSPORT MODEL (EXTRAN) *
 * developed 1973 by Camp, Dresser and McKee (CDM) with *
 * modifications 1977-1991 by the University of Florida. *
 *
 * Most recent update: March 1999 by CDM, Oregon *
 * State University, and XP Software, Inc. *
 *
 * "Smooth runs the water where the brook is deep." *
 * Shakespeare, Henry VI, II, III, I *

WASHINGTON, D.C.

ANALYSIS MODULE

CAMP DRESSER & MCKEE INC.

ANNANDALE, VIRGINIA

Detroit River International Crossing
Turkey Creek Alternative 1B,2B Syphon - 100-Year Flow

Control information for simulation

Integration cycles..... 1440

Length of integration step is..... 5.00 seconds

Simulation length..... 2.00 hours

Do not create equiv. pipes(NEQUAL). 0

Use metric units for I/O..... 1

Printing starts in cycle..... 1

Intermediate printout intervals of. 1 cycles

Intermediate printout intervals of. 0.08 minutes

Summary printout intervals of..... 1 cycles

Summary printout time interval of.. 0.08 minutes

Hot start file parameter (JREDO)... 0

Initial time (TZERO)..... 0.00 hours

This is time displacement from JIN interface file starting date/time when interface file is used.

This also describes starting hour in K3 line hydrograph input when K3 lines are used.

Initial date (IDATZ)..... 19060714 (yr/mo/day)

NOTE: Initial date from JIN interface file will be used, if accessed, unless IDATZ is negative.

Iteration variables: ITHMAX..... 30

SURTOL..... 0.0500

Default surface area of junctions.. 1.22 square meters.

EXTRAN VERSION 3.3 SOLUTION. (ISOL = 0).

Sum of junction flow is zero during surcharge.

NORMAL FLOW OPTION WHEN THE WATER
SURFACE SLOPE IS LESS THAN THE
GROUND SURFACE SLOPE (KSUPER=0).....

NJSW INPUT HYDROGRAPH JUNCTIONS..... 0

INTERMEDIATE HEADER LINES ARE PRINTED AS IN ORIGINAL PROGRAM

IDS ARE WRITTEN AS IN ORIGINAL PROGRAM

CONDUIT LENGTHS ON C1 LINE MUST EQUAL IRREGULAR SECTION LENGTH ENTERED ON THE C3 OR X1 LINES (IWLEN = 0)

JELEV = 0 (DEFAULT). STANDARD INPUTS ARE DEPTHS NOT ELEVATIONS

JDOWN = 0 - Minimum of normal or critical depth will be used at free outfalls (II).

Characteristic depth for M2 and S2 water surface profiles will be computed as in previous versions of EXTRAN (IM2 = 0).

SEDIMENT DEPTHS WILL NOT BE READ FROM C1 LINES

Intermediate continuity output will not be created

 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
 ***** ANALYSIS MODULE ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Turkey Creek Alternative 1B,2B Syphon - 100-Year Flow

1 *****
 * Conduit Data *

INP NUM	CONDUIT NUMBER	LENGTH (ft)	CONDUIT CLASS	AREA (SQ M)	COEF.	MANNING MAX WIDTH (M)	DEPTH (M)	JUNCTIONS AT THE ENDS	INVERT HEIGHT ABOVE JUNCTIONS	TRAPEZOID SIDE SLOPES
1	1	100.	TRAPEZOID	96.91	0.01500	9.75	4.64	1	2	2.40 2.40
2	2	100.	TRAPEZOID	96.91	0.01500	9.75	4.64	2	3	2.40 2.40
3	3	100.	TRAPEZOID	96.91	0.01500	9.75	4.64	3	4	2.40 2.40
4	4	100.	TRAPEZOID	96.91	0.01500	9.75	4.64	4	5	2.40 2.40
5	5	15.	RECTANGLE	60.00	0.01300	15.00	4.00	5	50	

6	50	23. RECTANGLE	50.00	0.01300	25.00	2.00	50	51
7	51	40. RECTANGLE	50.00	0.01300	25.00	2.00	51	55
8	55	23. RECTANGLE	50.00	0.01300	25.00	2.00	55	56
9	56	15. RECTANGLE	80.00	0.01300	20.00	4.00	56	6
10	6	100. TRAPEZOID	96.91	0.01500	9.75	4.64	6	7
11	7	100. TRAPEZOID	96.91	0.01500	9.75	4.64	7	8
12	8	100. TRAPEZOID	96.91	0.01500	9.75	4.64	8	9
13	9	100. TRAPEZOID	96.91	0.01500	9.75	4.64	9	10
14	10	100. TRAPEZOID	96.91	0.01500	9.75	4.64	10	11

====> WARNING !!! (C*DELTA/LEN) IN CONDUIT 5 IS 2.1 AT FULL DEPTH.
 ====> WARNING !!! (C*DELTA/LEN) IN CONDUIT 56 IS 2.1 AT FULL DEPTH.

 * Conduit Volume *

Input full depth volume..... 9.3620E+04 cubic meters
 Conduit #... 51 has zero slope. 0.001 feet added to upstream invert.

====> Warning !! The upstream and downstream junctions for the following conduits
 have been reversed to correspond to the positive flow and decreasing
 slope EXTRAN convention. A negative flow in the output thus means
 the flow was from your original upstream junction to your original
 downstream junction. Any initial flow has been multiplied by -1.

1. Conduit #... 55 has been changed.

1-----
 ENVIRONMENTAL PROTECTION AGENCY *** EXTENDED TRANSPORT PROGRAM *** WATER RESOURCES DIVISION
 WASHINGTON, D.C. *** *** CAMP DRESSER & MCKEE INC. ***
 *** ANALYSIS MODULE *** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Turkey Creek Alternative 1B,2B Syphon - 100-Year Flow

1 *****
 * Junction Data *

INP JUNCTION GROUND CROSS INVERT INITIAL CONNECTING CONDUITS
 NUM NUMBER ELEV. ELEV. ELEV. CMS DEPTH(H)

1	1	182.00	180.71	176.07	62.60	0.00	1
2	2	182.00	180.67	176.03	0.00	0.00	1
3	3	182.00	180.63	175.99	0.00	0.00	2
4	4	182.00	180.59	175.95	0.00	0.00	3
5	5	182.00	180.55	175.91	0.00	0.00	4
6	50	182.00	179.90	175.90	0.00	0.00	5
7	51	182.00	166.31	164.30	0.00	0.00	50
8	55	182.00	166.30	164.30	0.00	0.00	51
9	56	182.00	179.87	175.87	0.00	0.00	55
10	6	182.00	180.50	175.86	0.00	0.00	56
11	7	182.00	180.46	175.82	0.00	0.00	6
12	8	182.00	180.42	175.78	0.00	0.00	7
13	9	182.00	180.38	175.74	0.00	0.00	8
14	10	182.00	180.34	175.70	0.00	0.00	9
15	11	182.00	180.30	175.66	0.00	0.00	10

 * FREE OUTFALL DATA (DATA GROUP I1) *
 * BOUNDARY CONDITION ON DATA GROUP J1 *

OUTFALL AT JUNCTION... 11 HAS BOUNDARY CONDITION NUMBER... 1

1-----
 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
 ***** ANALYSIS MODULE ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Turkey Creek Alternative 1B,2B Syphon - 100-Year Flow

 * INTERNAL CONNECTIVITY INFORMATION *

CONDUIT	JUNCTION	JUNCTION
90015	11	0

1

 * BOUNDARY CONDITION INFORMATION *

***** DATA GROUPS J1-J4 *****

BC NUMBER.. 1 CONTROL WATER SURFACE ELEVATION IS.. 178.80 METERS.
 TZERO = 1906195 0.0000000E+00

 * INITIAL MODEL CONDITION *
 * INITIAL TIME = 0.00 HOURS *

JUNCTION / DEPTH / ELEVATION ==> "JUNCTION IS SURCHARGED."
 1/ 0.00 / 176.07 2/ 0.00 / 176.03 3/ 0.00 / 175.99
 4/ 0.00 / 175.95 5/ 0.00 / 175.91 50/ 0.00 / 175.90
 51/ 0.00 / 164.30 55/ 0.00 / 164.30 56/ 0.00 / 175.87
 6/ 0.00 / 175.86 7/ 0.00 / 175.82 8/ 0.00 / 175.78
 9/ 0.00 / 175.74 10/ 0.00 / 175.70 11/ 0.00 / 175.66

CONDUIT/ FLOW ==> "CONDUIT USES THE NORMAL FLOW OPTION."
 1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
 5/ 0.00 50/ 0.00 51/ 0.00 55/ 0.00
 56/ 0.00 6/ 0.00 7/ 0.00 8/ 0.00
 9/ 0.00 10/ 0.00 90015/ 0.00

CONDUIT/ VELOCITY
 1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
 5/ 0.00 50/ 0.00 51/ 0.00 55/ 0.00
 56/ 0.00 6/ 0.00 7/ 0.00 8/ 0.00
 9/ 0.00 10/ 0.00

CONDUIT/ CROSS SECTIONAL AREA
 1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
 5/ 0.00 50/ 0.00 51/ 0.00 55/ 0.00
 56/ 0.00 6/ 0.00 7/ 0.00 8/ 0.00
 9/ 0.00 10/ 0.00

CONDUIT/ HYDRAULIC RADIUS
 1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
 5/ 0.00 50/ 0.00 51/ 0.00 55/ 0.00
 56/ 0.00 6/ 0.00 7/ 0.00 8/ 0.00
 9/ 0.00 10/ 0.00

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/	176.07/	176.03	2/	176.03/	175.99	3/	175.99/	175.95
4/	175.95/	175.91	5/	175.91/	175.90	50/	175.90/	164.30
51/	164.31/	164.30	55/	175.87/	164.30	56/	175.87/	175.86
6/	175.86/	175.82	7/	175.82/	175.78	8/	175.78/	175.74
9/	175.74/	175.70	10/	175.70/	175.66			

 * FINAL MODEL CONDITION *
 * FINAL TIME = 2.00 HOURS *

>>> ENDING DATE AND TIME OF EXTRAN RUN ARE:

JULIAN DATE: 1906195
 YR/MO/DA: 1906/ 7/14
 TIME OF DAY: 2.000 HRS

JUNCTION / DEPTH / ELEVATION ===> "*" JUNCTION IS SURCHARGED.

1/	2.86 /	178.93	2/	2.89 /	178.92	3/	2.91 /	178.90
4/	2.94 /	178.89	5/	2.97 /	178.88	50/	2.98 /	178.88
51/	14.56* /	178.87	55/	14.56* /	178.86	56/	2.98 /	178.85
6/	3.00 /	178.86	7/	3.02 /	178.84	8/	3.05 /	178.83
9/	3.08 /	178.82	10/	3.11 /	178.81	11/	3.14 /	178.80

CONDUIT/ FLOW ===> "*" CONDUIT USES THE NORMAL FLOW OPTION.

1/	62.57	2/	62.61	3/	62.59	4/	62.59
5/	62.18	50/	62.89	51/	63.09	55/	-60.48
56/	66.50	6/	62.31	7/	62.58	8/	62.68
9/	62.48	10/	62.60	90015/	62.60		

CONDUIT/ VELOCITY

1/	1.31	2/	1.29	3/	1.27	4/	1.26
5/	1.39	50/	1.26	51/	1.26	55/	-1.21
56/	1.11	6/	1.22	7/	1.21	8/	1.20
9/	1.18	10/	1.16				

CONDUIT/ CROSS SECTIONAL AREA

1/	47.85	2/	48.47	3/	49.12	4/	49.77
5/	44.61	50/	50.00	51/	50.00	55/	50.00
56/	59.73	6/	51.03	7/	51.69	8/	52.43
9/	53.15	10/	53.90				

CONDUIT/ FINAL VOLUME

1/	4785.28	2/	4847.46	3/	4912.26	4/	4977.36
5/	669.11	50/	1150.00	51/	2000.00	55/	1150.00

56/ 895.95 6/ 5102.61 7/ 5169.46 8/ 5243.20
 9/ 5314.87 10/ 5389.59

CONDUIT/ HYDRAULIC RADIUS

1/ 1.94 2/ 1.95 3/ 1.97 4/ 1.98
 5/ 2.13 50/ 0.93 51/ 0.93 55/ 0.93
 56/ 2.30 6/ 2.01 7/ 2.02 8/ 2.04
 9/ 2.06 10/ 2.07

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/ 178.93/ 178.92 2/ 178.92/ 178.90 3/ 178.90/ 178.89
 4/ 178.89/ 178.88 5/ 178.88/ 178.88 50/ 178.88/ 178.87
 51/ 178.87/ 178.86 55/ 178.85/ 178.86 56/ 178.85/ 178.86
 6/ 178.86/ 178.84 7/ 178.84/ 178.83 8/ 178.83/ 178.82
 9/ 178.82/ 178.81 10/ 178.81/ 178.80

 # Surcharge Iteration Summary #
 #####

Maximum number of iterations in a time step..... 31
 Total number of iterations in the simulation.. 4109
 Average number of iterations per time step..... 2.85
 Surcharge iterations during the simulation..... 1229
 Maximum surcharge flow error during simulation.. 6.92E+01 cms
 Total number of time steps during simulation.. 1440

1

 * CONDUIT COURANT CONDITION SUMMARY *
 * TIME IN MINUTES DELT > COURANT TIME STEP *

 * SEE BELOW FOR EXPLANATION OF COURANT TIME STEP. *

CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)
1	0.00	2	0.00	3	0.00	4	0.00
5	118.58	50	118.25	51	117.75	55	117.92
56	118.08	6	0.00	7	0.00	8	0.00
9	0.00	10	0.00				

```

1 *****
* CONDUIT COURANT CONDITION SUMMARY *
*****
* COURANT = CONDUIT LENGTH *
* TIME STEP = ----- *
* VELOCITY + SQRT(GRVT*AREA/WIDTH) *
*****
* AVERAGE COURANT CONDITION TIME STEP(SECONDS) *
*****

```

CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)
1	17.24	2	17.25	3	17.95	4	21.36
5	2.74	50	2.16	51	3.16	55	1.82
56	2.35	6	17.12	7	17.09	8	17.09
9	17.11	10	17.02				

```

1 *****
* EXTRAN CONTINUITY BALANCE AT THE LAST TIME STEP *
*****
*****
* JUNCTION INFLOW, OUTFLOW OR STREET FLOODING *
*****

```

```

JUNCTION INFLOW, CU M
-----
1 4.5072E+05

```

```

JUNCTION OUTFLOW, CU M
-----
1 9.2710E+02
3 2.9048E+02
4 5.0193E+01
5 4.3016E+02
50 7.5294E+02
56 7.3184E+01
6 7.2493E+03
7 4.5054E+02
8 1.6235E+03
9 1.2573E+03
10 3.8129E+02
11 3.8971E+05

```



```

*****
* INITIAL SYSTEM VOLUME = 1.0160E-02 CU M *
* TOTAL SYSTEM INFLOW VOLUME = 4.5072E+05 CU M *
* INFLOW + INITIAL VOLUME = 4.5072E+05 CU M *
*****
* TOTAL SYSTEM OUTFLOW = 4.0319E+05 CU M *
* VOLUME LEFT IN SYSTEM = 5.1608E+04 CU M *
* OUTFLOW + FINAL VOLUME = 4.5480E+05 CU M *
*****
* ERROR IN CONTINUITY, PERCENT = -0.91 *
*****

```

```

TEST WRITE OF ALTERNATIVE CONTINUITY ERROR CALCULATION
VOLUME LEFT IN SYSTEM = 9.8749E+04 CU. FT.
ERROR IN CONTINUITY PERCENT = -113.64

```

----- SUMMARY OF FULL FLOW CHANNEL WARNINGS -----

OPEN CHANNEL NUMBER	TIME STEP OF FIRST OCCURRENCE	TIME OF FIRST OCCURRENCE (HOURS)	TIME STEP OF LAST OCCURRENCE	TIME OF LAST OCCURRENCE (HOURS)
1	1	0.00	91	0.13
2	74	0.10	91	0.13
3	74	0.10	92	0.13
4	74	0.10	92	0.13
6	24	0.03	82	0.11
7	20	0.03	25	0.03
8	17	0.02	57	0.08
9	15	0.02	63	0.09
10	15	0.02	63	0.09

THE PROGRAM USES FULL DEPTH CHANNEL CHARACTERISTICS TO COMPUTE FLOW THROUGH THE TRAPEZOIDAL, IRREGULAR, OR PARABOLIC/POWER FUNCTION CONDUIT WHEN THE COMPUTED DEPTHS EXCEED MAXIMUM DEPTH. THIS WILL AFFECT THE MAXIMUM COMPUTED HEAD AND FLOWS IN THE MODEL. IT IS HIGHLY RECOMMENDED THAT THE MODELED CROSS SECTIONS BE EXTENDED TO ELIMINATE THESE FULL FLOW CHANNEL WARNINGS

```

1
*****
* JUNCTION SUMMARY STATISTICS *
*****

```

Detroit River International Crossing
 Turkey Creek Alternative 1B,2B Syphon - 100-Year Flow

JUNCTION NUMBER	GROUND ELEVATION (M)	UPPERMOST PIPE CROWN ELEVATION (M)	MEAN JUNCTION ELEVATION (M)	JUNCTION AVERAGE ELEV. CHANGE (%)	MAXIMUM JUNCTION ELEV. (M)	TIME OF OCCURRENCE HR. MIN.	METERS OF SURCHARGE AT MAX ELEVATION	METERS MAX. DEPTH IS BELOW GROUND ELEVATION	LENGTH OF SURCHARGE (MIN)	LENGTH OF FLOODING (MIN)	MAXIMUM JUNCTION AREA (SQ.MET)
1	182.00	180.71	178.98	0.3684	182.00	0 5	1.29	0.00	0.8	0.3	4.880E+04
2	182.00	180.67	178.95	0.2199	181.93	0 7	1.26	0.07	0.3	0.0	9.721E+04
3	182.00	180.63	178.92	0.2144	182.00	0 7	1.37	0.00	0.3	0.1	9.732E+04
4	182.00	180.59	178.90	0.2467	182.00	0 6	1.41	0.00	0.2	0.1	9.882E+04
5	182.00	180.55	178.88	0.3865	182.00	0 6	1.45	0.00	0.3	0.1	5.312E+04
50	182.00	179.90	178.87	0.6565	182.00	0 2	2.10	0.00	2.8	0.1	1.240E+04
51	182.00	166.31	178.66	1.6342	181.41	0 2	15.11	0.59	117.9	0.0	2.441E+04
55	182.00	166.30	178.65	2.0175	180.96	0 6	14.65	1.04	117.9	0.0	2.441E+04
56	182.00	179.87	178.85	0.6963	182.00	0 2	2.13	0.00	2.4	0.1	1.356E+04
6	182.00	180.50	178.86	0.4825	182.00	0 2	1.50	0.00	0.6	0.3	5.323E+04
7	182.00	180.46	178.84	0.3419	182.00	0 2	1.54	0.00	0.2	0.1	4.803E+04
8	182.00	180.42	178.83	0.3371	182.00	0 1	1.58	0.00	0.2	0.2	1.597E+04
9	182.00	180.38	178.82	0.3148	182.00	0 1	1.62	0.00	0.5	0.1	4.363E+04
10	182.00	180.34	178.81	0.3738	182.00	0 1	1.66	0.00	0.2	0.2	9.404E+04
11	182.00	180.30	178.81	0.0957	180.30	0 1	0.00	1.70	0.3	0.0	4.259E+04

1

 * CONDUIT SUMMARY STATISTICS *

Detroit River International Crossing
 Turkey Creek Alternative 1B,2B Syphon - 100-Year Flow

CONDUIT NUMBER	DESIGN FLOW (CMS)	DESIGN VELOCITY (M/S)	CONDUIT VERTICAL DEPTH (M)	MAXIMUM COMPUTED FLOW (CMS)	TIME OF OCCURRENCE HR. MIN.	MAXIMUM COMPUTED VELOCITY (MPS)	TIME OF OCCURRENCE HR. MIN.	RATIO OF MAX. TO DESIGN FLOW	MAX. INV. UPSTREAM (M)	MAXIMUM DEPTH ABOVE DOWNSTREAM (M)	LENGTH OF CONDUIT ENDS (MIN)	NORM SLOPE
1	2.60E+02	2.69	4.640	1.67E+02	0 0	4.06	0 0	0.64	5.93	5.88	0.8	0.00040
2	2.60E+02	2.69	4.640	1.42E+02	0 5	4.15	0 1	0.55	5.88	6.01	0.5	0.00040
3	2.60E+02	2.69	4.640	1.46E+02	0 9	3.94	0 1	0.56	6.01	6.05	0.4	0.00040
4	2.60E+02	2.69	4.640	1.56E+02	0 8	3.72	0 1	0.60	6.05	6.09	0.3	0.00040
5	1.62E+02	2.69	4.000	2.00E+02	0 7	6.76	0 2	1.24	6.09	6.10	0.1	0.00067

CONDUIT NUMBER	LENGTH OF DRY FLOW (MIN)	LENGTH OF SUBCRITICAL FLOW (MIN)	LENGTH OF UPSTR. OF DOWNSTR. CRITICAL FLOW (MIN)	LENGTH OF DOWNSTR. CRITICAL FLOW (MIN)	MEAN FLOW (CMS)	AVERAGE % CHANGE	TOTAL FLOW CUBIC MET. RADIUS (MET)	MAXIMUM HYDRAULIC CROSS SECT AREA (SQ.M)	MAXIMUM CROSS SECT AREA (SQ.M)					
50	2.59E+03	51.89	2.000	7.04E+02	0	2	8.03	0	1	0.27	6.10	17.09	0.6	0.50413
51	1.83E+01	0.37	2.000	-2.30E+02	0	2	-4.60	0	2	-12.55	17.09	16.65	0.1	0.00003
55	2.59E+03	51.82	2.000	6.77E+02	0	2	6.23	0	2	0.26	6.12	16.65	0.5	0.50283
56	2.23E+02	2.79	4.000	-6.89E+02	0	2	-8.61	0	2	-3.09	6.12	6.14	0.1	0.00067
6	2.60E+02	2.69	4.640	-6.33E+02	0	2	-7.33	0	2	-2.43	6.14	6.10	0.0	0.00040
7	2.60E+02	2.69	4.640	-7.32E+02	0	1	-8.48	0	2	-2.81	6.10	6.22	0.1	0.00040
8	2.60E+02	2.69	4.640	-7.07E+02	0	1	-7.30	0	1	-2.72	6.22	5.85	0.2	0.00040
9	2.60E+02	2.69	4.640	-7.86E+02	0	1	-8.37	0	1	-3.02	5.85	6.30	0.2	0.00040
10	2.60E+02	2.69	4.640	-7.57E+02	0	1	-7.86	0	1	-2.91	6.30	4.64	0.3	0.00040
90015	UNDEF	UNDEF	UNDEF	-7.57E+02	0	1								

1

 * SUBCRITICAL AND CRITICAL FLOW ASSUMPTIONS FROM *
 * SUBROUTINE HEAD. SEE FIGURE 5-4 IN THE EXTRAN *
 * MANUAL FOR FURTHER INFORMATION. *

CONDUIT NUMBER	LENGTH OF DRY FLOW (MIN)	LENGTH OF SUBCRITICAL FLOW (MIN)	LENGTH OF UPSTR. OF DOWNSTR. CRITICAL FLOW (MIN)	LENGTH OF DOWNSTR. CRITICAL FLOW (MIN)	MEAN FLOW (CMS)	AVERAGE % CHANGE	TOTAL FLOW CUBIC MET. RADIUS (MET)	MAXIMUM HYDRAULIC CROSS SECT AREA (SQ.M)	MAXIMUM CROSS SECT AREA (SQ.M)
1	0.00	120.00	0.00	0.00	62.60	0.3486	4.5074E+05	2.8606	96.9110
2	0.08	119.92	0.00	0.00	61.93	0.3652	4.4592E+05	2.8606	96.9110
3	0.08	119.92	0.00	0.00	61.20	0.3023	4.4067E+05	2.8205	94.3929
4	0.25	119.75	0.00	0.00	60.47	0.4788	4.3537E+05	2.8606	96.9110
5	0.58	119.42	0.00	0.00	59.97	1.8271	4.3180E+05	2.4111	60.0000
50	1.08	118.92	0.00	0.00	60.08	0.1324	4.3261E+05	1.0787	50.0000
51	1.25	118.75	0.00	0.00	59.40	10.9593	4.2765E+05	0.9660	50.0000
55	1.67	118.33	0.00	0.00	-58.79	0.1639	-4.2326E+05	1.4793	50.0000
56	1.58	118.42	0.00	0.00	58.78	3.8133	4.2321E+05	2.6418	80.0000
6	1.33	118.67	0.00	0.00	57.56	0.9117	4.1440E+05	2.7102	87.6460
7	1.00	119.00	0.00	0.00	56.62	0.7945	4.0769E+05	2.6881	86.3271
8	0.58	119.42	0.00	0.00	55.67	0.7054	4.0084E+05	2.8606	96.9110
9	0.17	119.83	0.00	0.00	55.10	0.7921	3.9673E+05	2.8136	93.9603
10	0.00	120.00	0.00	0.00	54.13	0.6719	3.8971E+05	2.8606	96.9110
90015	UNDEF	UNDEF	UNDEF	UNDEF	54.13		3.8971E+05		

 * AVERAGE % CHANGE IN JUNCTION OR CONDUIT IS DEFINED AS: *
 * CONDUIT % CHANGE ==> 100.0 (Q(n+1) - Q(n)) / Qfull *
 * JUNCTION % CHANGE ==> 100.0 (Y(n+1) - Y(n)) / Yfull *

The Conduit with the largest average change... 51 had 10.959 percent
The Junction with the largest average change... 55 had 2.018 percent

==> Extended Transport model simulation ended normally.

==> SWMM 4.4GU simulation ended normally.

Always check output file for possible warning messages.

==> Your input file was named : PCTmpl.dat

==> Your output file was named: PCTmpl.out

```
*****  
* SWMM 4.4GU Simulation Date and Time Summary *  
*****  
* Starting Date... November 17, 2006 *  
* Time... 9:34: 9.480 *  
* Ending Date... November 17, 2006 *  
* Time... 9:34:15.420 *  
* Elapsed Time... 0.099 minutes. *  
* Elapsed Time... 5.941 seconds. *  
*****
```

Appendix A.2.4.2

Lennon Drain

```

*****
* U.S. Environmental Protection Agency *
* Storm Water Management Model (SWMM) *
* Version 4.4GU *
* *
* CDM/OSU Beta *
* Release Date - November 23, 1999 *
* Camp Dresser & McKee and Oregon State Univ. *
* Chuck Moore and Wayne Huber *
* Compiled using Digital Visual Fortran 6.0 *
*****

```

Developed by

```

*****
* Metcalf & Eddy, Inc. *
* University of Florida *
* Water Resources Engineers, Inc. *
* (Now Camp, Dresser and McKee, Inc.) *
* September 1970 *
*****

```

Distributed and Maintained by

```

*****
* U.S. Environmental Protection Agency *
* Center for Exposure Assessment Modeling (CEAM) *
* Athens Environmental Research Laboratory *
* 960 College Station Road *
* Athens, GA 30605-2720 *
*****

```

```

*****
* This is a new release of SWMM. If any *
* problems occur executing this model *
* system, contact Mr. Frank Stancil, *
* U.S. Environmental Protection Agency. *
* 706/355-8328 (voice) *
* e-mail: stancil@athens.ath.epa.gov *
* Or contact Wayne C. Huber at Oregon St. U. *
* 541/737-6150 or wayne.huber@orst.edu *
* Or Michael F. Schmidt at Camp Dresser & *
* McKee (904) 281-0170 SCHMIDTF@CDM.COM *
*****

```

```

*****
* This is an implementation of EPA SWMM 4.4GU *
* "Nature is full of infinite causes which *
* have never occurred in experience" da Vinci *
*****

```

```

#####
# File names by SWMM Block #
# JIN -> Input to a Block #
# JOUT -> Output from a Block #
#####

```

```

JIN for Block # 1 File # 0 JIN.UF
JOUT for Block # 1 File # 9 PCTmpl.int

```

```

#####
# Scratch file names for this simulation. #
#####

```

```

NSCRAT # 1 File # 21 SCRT1.UF
NSCRAT # 2 File # 22 SCRT2.UF
NSCRAT # 3 File # 23 SCRT3.UF
NSCRAT # 4 File # 24 SCRT4.UF
NSCRAT # 5 File # 25 SCRT5.UF
NSCRAT # 6 File # 26 SCRT6.UF
NSCRAT # 7 File # 27 SCRT7.UF
NSCRAT # 8 File # 28 SCRT8.UF

```

```

*****
* Parameter Values on the Tapes Common Block *
*****

```

```

Number of Subcatchments in the Runoff Block (NW)..... 1000
Number of Channel/Pipes in the Runoff Block (NG)..... 1000
Number of Connections to Runoff Channels/Inlets (INCP). 6
Number of Water Quality Constituents (NQUAL)..... 20
Number of Runoff Land Uses per Subcatchment (NLU)..... 20
Number of Groundwater Subcatchments in Runoff (NGW)... 100
Number of Interface Locations for all Blocks (NIE).... 1000
Number of Elements in the Transport Block (NET)..... 500
Number of Storage Junctions in Transport (NTSE)..... 100
Number of Transport interface input locations (NTHI).. 500
Number of Transport interface output locations (NTHO). 500
Number of Transport input locations on R lines (NTHR). 80

```

Number of Transport printed output locations (NTOA)... 80
 Number of Tubular Flow Splitters in Transport (NTSP).. 50
 Number of Elements in the Extran Block (NEE)..... 4000
 Number of Pumps in Extran (NEP)..... 75
 Number of Orifices in Extran (NEO)..... 200
 Number of Tide Gates/Free Outfalls in Extran (NTG)... 200
 Number of Extran Weirs (NEW)..... 400
 Number of Extran Printout Locations (NPO)..... 150
 Number of Tide Elements in Extran (NTE)..... 50
 Number of Natural Channels (NNC)..... 1200
 Number of Storage Junctions in Extran (NVSE)..... 1000
 Number of Time History Data Points in Extran (NTVAL).. 500
 Number of Data Points for Variable Storage Elements
 in the Extran Block (NVST)..... 25
 Number of Input Hydrographs in Extran (NEH)..... 500
 Number of Allowable Channel Connections to
 Junctions in the Extran Block (NCHN)..... 15
 Number Rain Gages in Rain and Runoff (MAXRG)..... 200
 Number PRATE/VRATE Points for Extran Pump
 Input (MAXPRA)..... 10
 Number of Variable Orifices in Extran (NVORE)..... 50
 Number of Variable Orifice Data Points (NVOTIM)..... 50
 Number of Allowable Precip. Values/yr in Rain (LIMRN). 5000
 Number of Storm Events for Rain Analysis (LSTORM).....20000
 Number of Plugs for Plug-flow in S/T (NPLUG)..... 3000
 Number Conduits for Extran Results to ASCII
 File (MXFLOW)..... 400

 + Entry made to the EXTENDED TRANSPORT MODEL (EXTRAN) +
 + developed 1973 by Camp, Dresser and McKee (CDM) with +
 + modifications 1977-1991 by the University of Florida. +
 +
 + Most recent update: March 1999 by CDM, Oregon +
 + State University, and XP Software, Inc. +
 +
 + "Smooth runs the water where the brook is deep." +
 + Shakespeare, Henry VI, II, III, 1 +

WASHINGTON, D.C.

ANALYSIS MODULE

CAMP DRESSER & MCKEE INC.
ANNANDALE, VIRGINIA

Detroit River International Crossing
Lennon Drain Existing Condition - 100-Year Flow

Control information for simulation

Integration cycles..... 4000

Length of integration step is..... 15.00 seconds

Simulation length..... 16.67 hours

Do not create equiv. pipes(NEQUAL). 0

Use metric units for I/O..... 1

Printing starts in cycle..... 1

Intermediate printout intervals of. 1 cycles

Intermediate printout intervals of. 0.25 minutes

Summary printout intervals of..... 1 cycles

Summary printout time interval of.. 0.25 minutes

Hot start file parameter (JREDO)... 0

Initial time (TZERO)..... 0.00 hours

This is time displacement from JIN interface file starting date/time when
interface file is used.

This also describes starting hour in K3 line hydrograph input when K3
lines are used.

Initial date (IDATZ)..... 19060714 (yr/mo/day)

NOTE: Initial date from JIN interface file will be used, if accessed,
unless IDATZ is negative.

Iteration variables: ITHAX..... 30

SURTOL..... 0.0500

Default surface area of junctions.. 1.22 square meters.

EXTRAN VERSION 3.3 SOLUTION. (ISOL = 0).

Sum of junction flow is zero during surcharge.

NORMAL FLOW OPTION WHEN THE WATER SURFACE SLOPE IS LESS THAN THE GROUND SURFACE SLOPE (KSUPER=0).....

NJSW INPUT HYDROGRAPH JUNCTIONS..... 0

INTERMEDIATE HEADER LINES ARE PRINTED AS IN ORIGINAL PROGRAM

IDS ARE WRITTEN AS IN ORIGINAL PROGRAM

CONDUIT LENGTHS ON C1 LINE MUST EQUAL IRREGULAR SECTION LENGTH ENTERED ON THE C3 OR X1 LINES (IWLEN = 0)

JLEEV = 0 (DEFAULT). STANDARD INPUTS ARE DEPTHS NOT ELEVATIONS

JDOWN = 0 - Minimum of normal or critical depth will be used at free outfalls (I1).

Characteristic depth for M2 and S2 water surface profiles will be computed as in previous versions of EXTRAN (IM2 = 0).

SEDIMENT DEPTHS WILL NOT BE READ FROM C1 LINES

Intermediate continuity output will not be created

1-----
 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
 ***** ANALYSIS MODULE ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Lennon Drain Existing Condition - 100-Year Flow

INP NUM	CONDUIT NUMBER	LENGTH (M)	CONDUIT CLASS	AREA (SQ M)	MANNING COEF.	MAX WIDTH (M)	DEPTH (M)	JUNCTIONS AT THE ENDS	INVERT HEIGHT ABOVE JUNCTIONS	TRAPEZOID SIDE SLOPES
1	1	70.	TRAPEZOID	803.64	0.03500	350.00	2.20	1	2	5.20 8.70
2	2	70.	TRAPEZOID	803.64	0.03500	350.00	2.20	2	3	5.20 8.70
3	3	100.	TRAPEZOID	803.64	0.03500	350.00	2.20	3	4	5.20 8.70
4	4	100.	TRAPEZOID	803.64	0.03500	350.00	2.20	4	5	5.20 8.70
5	5	60.	TRAPEZOID	46.22	0.03500	2.50	2.30	5	6	6.60 8.70

6	6	75. RECTANGLE	3.17	0.01500	2.60	1.22	6	7	
7	7	40. TRAPEZOID	21.80	0.03500	2.30	2.00	7	8	2.90 5.70
8	8	75. TRAPEZOID	18.18	0.03500	2.00	1.80	8	9	4.30 4.70
9	9	75. TRAPEZOID	18.18	0.03500	2.00	1.80	9	10	4.30 4.70
10	10	75. TRAPEZOID	18.18	0.03500	2.00	1.80	10	11	4.30 4.70

====> WARNING !!! (C*DELTA/LEN) IN CONDUIT 7 IS 1.2 AT FULL DEPTH.

 + Conduit Volume +

Input full depth volume..... 2.8121E+05 cubic meters

1-----
 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
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Detroit River International Crossing
 Lennon Drain Existing Condition - 100-Year Flow

1

 + Junction Data +

INP NUM	JUNCTION NUMBER	GROUND ELEV.	CROWN ELEV.	INVERT ELEV.	QINST CMS	INITIAL DEPTH (M)	CONNECTING CONDUITS
1	1	183.00	182.61	180.41	11.80	0.00	1
2	2	183.00	182.47	180.27	0.00	0.00	1 2
3	3	183.00	182.33	180.13	0.00	0.00	2 3
4	4	183.00	182.13	179.93	0.00	0.00	3 4
5	5	183.00	182.03	179.73	0.00	0.00	4 5
6	6	183.00	181.91	179.61	0.00	0.00	5 6
7	7	183.00	181.46	179.46	0.00	0.00	6 7
8	8	183.00	181.38	179.38	0.00	0.00	7 8
9	9	183.00	181.03	179.23	0.00	0.00	8 9
10	10	183.00	180.88	179.08	0.00	0.00	9 10
11	11	183.00	180.73	178.93	0.00	0.00	10

```

*****
*   FREE OUTFALL DATA (DATA GROUP I1)   *
*   BOUNDARY CONDITION ON DATA GROUP J1 *
*****

```

OUTFALL AT JUNCTION... I1 HAS BOUNDARY CONDITION NUMBER... 1

```

I-----
ENVIRONMENTAL PROTECTION AGENCY   ****   EXTENDED TRANSPORT PROGRAM   ****
WASHINGTON, D.C.                ****
                                  ****   ANALYSIS MODULE             ****
                                  ****   WATER RESOURCES DIVISION
                                  ****   CAMP DRESSER & MCKEE INC.
                                  ****   ANNANDALE, VIRGINIA

```

Detroit River International Crossing
Lennon Drain Existing Condition - 100-Year Flow

```

*****
*   INTERNAL CONNECTIVITY INFORMATION   *
*****

```

```

CONDUIT   JUNCTION   JUNCTION
-----
90011     I1         0

```

```

*****
*   BOUNDARY CONDITON INFORMATION     *
*   DATA GROUPS J1-J4                *
*****

```

BC NUMBER.. 1 HAS NO CONTROL WATER SURFACE.
TZERO = 1906195 0.00000000E+00

```

*****
*   INITIAL MODEL CONDITION           *
*   INITIAL TIME = 0.00 HOURS         *
*****

```

```

JUNCTION / DEPTH / ELEVATION   ==>  "" JUNCTION IS SURCHARGED.
1/ 0.00 / 180.41             2/ 0.00 / 180.27             3/ 0.00 / 180.13
4/ 0.00 / 179.93             5/ 0.00 / 179.73             6/ 0.00 / 179.61
7/ 0.00 / 179.46             8/ 0.00 / 179.38             9/ 0.00 / 179.23
10/ 0.00 / 179.08            11/ 0.00 / 178.93

```

```

CONDUIT/ FLOW ==> "*" CONDUIT USES THE NORMAL FLOW OPTION.
1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
5/ 0.00 6/ 0.00 7/ 0.00 8/ 0.00
9/ 0.00 10/ 0.00 90011/ 0.00
    
```

```

CONDUIT/ VELOCITY
1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
5/ 0.00 6/ 0.00 7/ 0.00 8/ 0.00
9/ 0.00 10/ 0.00
    
```

```

CONDUIT/ CROSS SECTIONAL AREA
1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
5/ 0.00 6/ 0.00 7/ 0.00 8/ 0.00
9/ 0.00 10/ 0.00
    
```

```

CONDUIT/ HYDRAULIC RADIUS
1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
5/ 0.00 6/ 0.00 7/ 0.00 8/ 0.00
9/ 0.00 10/ 0.00
    
```

```

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION
1/ 180.41/ 180.27 2/ 180.27/ 180.13 3/ 180.13/ 179.93
4/ 179.93/ 179.73 5/ 179.73/ 179.61 6/ 179.61/ 179.46
7/ 179.46/ 179.38 8/ 179.38/ 179.23 9/ 179.23/ 179.08
10/ 179.08/ 178.93
    
```

```

*****
* FINAL MODEL CONDITION *
* FINAL TIME = 16.67 HOURS *
*****
    
```

```

>>> ENDING DATE AND TIME OF EXTRAN RUN ARE:
JULIAN DATE: 1906195
YR/MC/DA: 1906/ 7/14
TIME OF DAY: 16.667 HRS
    
```

```

JUNCTION / DEPTH / ELEVATION ==> "*" JUNCTION IS SURCHARGED.
1/ 1.11 / 181.52 2/ 1.25 / 181.52 3/ 1.39 / 181.52
4/ 1.59 / 181.52 5/ 1.79 / 181.52 6/ 1.90 / 181.51
7/ 1.34 / 180.80 8/ 1.33 / 180.71 9/ 1.32 / 180.55
10/ 1.30 / 180.38 11/ 0.86 / 179.79
    
```

```

CONDUIT/ FLOW ==> "*" CONDUIT USES THE NORMAL FLOW OPTION.
    
```

1/	11.77	2/	11.69	3/	11.61	4/	11.54
5/	11.48	6/	11.48	7/	11.48	8/	11.48
9/	11.48	10/	11.48	90011/	11.48		

CONDUIT/ VELOCITY

1/	0.03	2/	0.02	3/	0.02	4/	0.02
5/	0.37	6/	3.62	7/	1.07	8/	1.08
9/	1.11	10/	1.55				

CONDUIT/ CROSS SECTIONAL AREA

1/	422.16	2/	473.58	3/	536.38	4/	610.78
5/	30.62	6/	3.17	7/	10.73	8/	10.60
9/	10.35	10/	7.42				

CONDUIT/ FINAL VOLUME

1/	29551.41	2/	33150.29	3/	53638.02	4/	61078.38
5/	1837.36	6/	237.90	7/	429.35	8/	795.01
9/	775.93	10/	556.28				

CONDUIT/ HYDRAULIC RADIUS

1/	1.15	2/	1.29	3/	1.45	4/	1.63
5/	0.99	6/	0.42	7/	0.76	8/	0.74
9/	0.73	10/	0.62				

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/	181.52/	181.52	2/	181.52/	181.52	3/	181.52/	181.52
4/	181.52/	181.52	5/	181.52/	181.51	6/	181.51/	180.80
7/	180.80/	180.71	8/	180.71/	180.55	9/	180.55/	180.38
10/	180.38/	179.79						

 # Surcharge Iteration Summary #
 #####

Maximum number of iterations in a time step..... 2
 Total number of iterations in the simulation... 8001
 Average number of iterations per time step..... 2.00
 Surcharge iterations during the simulation..... 1
 Maximum surcharge flow error during simulation.. 0.00E+00 cms
 Total number of time steps during simulation.. 4000

 * CONDUIT COURANT CONDITION SUMMARY *
 * TIME IN MINUTES DELT > COURANT TIME STEP *

 * SEE BELOW FOR EXPLANATION OF COURANT TIME STEP. *

CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)
1	0.25	2	0.50	3	0.00	4	0.00
5	0.00	6	991.25	7	996.00	8	0.00
9	0.00	10	0.00				

1

 * CONDUIT COURANT CONDITION SUMMARY *

 * COURANT = CONDUIT LENGTH *
 * TIME STEP = ----- *
 * VELOCITY + SORT(GRVT*AREA/WIDTH) *

 * AVERAGE COURANT CONDITION TIME STEP(SECONDS) *

CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)
1	23.95	2	22.19	3	29.36	4	27.23
5	18.18	6	11.29	7	11.02	8	22.31
9	20.58	10	20.94				

1

 * EXTRAN CONTINUITY BALANCE AT THE LAST TIME STEP *

 * JUNCTION INFLOW, OUTFLOW OR STREET FLOODING *

JUNCTION INFLOW, CU M

 1 7.0800E+05

JUNCTION OUTFLOW, CU M

11 5.8673E+05

```

*****
* INITIAL SYSTEM VOLUME = 7.4000E-03 CU M *
* TOTAL SYSTEM INFLOW VOLUME = 7.0800E+05 CU M *
* INFLOW + INITIAL VOLUME = 7.0800E+05 CU M *
*****
* TOTAL SYSTEM OUTFLOW = 5.8673E+05 CU M *
* VOLUME LEFT IN SYSTEM = 1.8209E+05 CU M *
* OUTFLOW + FINAL VOLUME = 7.6882E+05 CU M *
*****
* ERROR IN CONTINUITY, PERCENT = -8.59 *
*****

```

```

TEST WRITE OF ALTERNATIVE CONTINUITY ERROR CALCULATION
VOLUME LEFT IN SYSTEM = 1.1790E+05 CU. FT.
ERROR IN CONTINUITY PERCENT = 4.76

```

----- SUMMARY OF FULL FLOW CHANNEL WARNINGS -----

OPEN CHANNEL NUMBER	TIME STEP OF FIRST OCCURRENCE (HOURS)	TIME OF FIRST OCCURRENCE (HOURS)	TIME STEP OF LAST OCCURRENCE	TIME OF LAST OCCURRENCE (HOURS)
1	1	0.00	1	0.00

THE PROGRAM USES FULL DEPTH CHANNEL CHARACTERISTICS TO COMPUTE FLOW THROUGH THE TRAPEZOIDAL, IRREGULAR, OR PARABOLIC/POWER FUNCTION CONDUIT WHEN THE COMPUTED DEPTHS EXCEED MAXIMUM DEPTH. THIS WILL AFFECT THE MAXIMUM COMPUTED HEAD AND FLOWS IN THE MODEL. IT IS HIGHLY RECOMMENDED THAT THE MODELED CROSS SECTIONS BE EXTENDED TO ELIMINATE THESE FULL FLOW CHANNEL WARNINGS

1

***** JUNCTION SUMMARY STATISTICS *****

Detroit River International Crossing
Lennon Drain Existing Condition - 100-Year Flow

JUNCTION	UPPERMOST GROUND ELEVATION	MEAN PIPE CROWN ELEVATION	MAXIMUM JUNCTION AVERAGE ELEV.	TIME OF OCCURRENCE AT MAX	METERS OF SURCHARGE DEPTH IS BELOW GROUND	LENGTH OF SURCHARGE FLOODING	MAXIMUM JUNCTION AREA

NUMBER	(M)	(M)	(M)	% CHANGE	(M)	HR. MIN.	ELEVATION	(MIN)	(MIN)	(SQ-MET)
1	183.00	182.61	181.27	0.0685	182.52	0	0.00	0.48	0.0	2.607E+04
2	183.00	182.47	181.27	0.0591	181.71	0	0.00	1.29	0.0	2.607E+04
3	183.00	182.33	181.27	0.0487	181.62	0	0.00	1.38	0.0	3.141E+04
4	183.00	182.13	181.26	0.0315	181.52	16	40	1.48	0.0	3.721E+04
5	183.00	182.03	181.26	0.0361	181.52	16	40	1.48	0.0	1.962E+04
6	183.00	181.91	181.25	0.0486	181.51	16	39	1.49	0.0	1.033E+03
7	183.00	181.46	180.70	0.0307	180.80	16	40	2.20	0.0	3.744E+02
8	183.00	181.38	180.62	0.0245	180.71	16	40	2.29	0.0	8.012E+02
9	183.00	181.03	180.46	0.0227	180.55	16	40	2.45	0.0	1.043E+03
10	183.00	180.88	180.29	0.0193	180.38	16	40	2.62	0.0	9.927E+02
11	183.00	180.73	179.72	0.0133	179.79	16	40	3.21	0.0	8.074E+02

1

 * CONDUIT SUMMARY STATISTICS *

Detroit River International Crossing
 Lennon Drain Existing Condition - 100-Year Flow

CONDUIT NUMBER	DESIGN FLOW (CMS)	DESIGN VELOCITY (M/S)	CONDUIT VERTICAL DEPTH (M)	MAXIMUM COMPUTED FLOW (CMS)	TIME OF OCCURRENCE HR. MIN.	MAXIMUM COMPUTED VELOCITY (MPS)	TIME OF OCCURRENCE HR. MIN.	RATIO OF MAXIMUM DEPTH ABOVE LENGTH CONDUIT		
								DESIGN UPSTREAM FLOW (M)	MAX. TO INV. AT CONDUIT ENDS OF NORM SLOPE	
1	1.69E+03	2.10	2.200	1.79E+03	0 0	2.85	0 0	2.11	2.02	0.3 0.00200
2	1.69E+03	2.10	2.200	1.75E+03	0 0	5.45	0 1	2.02	1.39	0.3 0.00200
3	1.69E+03	2.10	2.200	5.39E+02	0 1	1.79	0 1	1.39	1.59	0.3 0.00200
4	1.69E+03	2.10	2.200	2.21E+02	0 2	1.16	0 2	1.59	1.79	0.5 0.00200
5	6.73E+01	1.46	2.300	1.17E+01	0 3	1.41	0 3	1.79	1.90	1.0 0.00200
6	5.26E+00	1.66	1.220	1.15E+01	16 39	3.62	16 40	1.90	1.34	1.8 0.00200
7	2.95E+01	1.35	2.000	1.15E+01	16 40	1.91	0 4	1.34	1.33	7.5 0.00200
8	2.29E+01	1.26	1.800	1.15E+01	16 40	1.21	0 5	1.33	1.32	2.5 0.00200
9	2.29E+01	1.26	1.800	1.15E+01	16 38	1.26	0 7	1.32	1.30	1.5 0.00200
10	2.29E+01	1.26	1.800	1.15E+01	16 40	1.55	16 38	1.30	0.86	0.0 0.00200
90011	UNDEF	UNDEF	UNDEF	1.15E+01	16 40			1.30		

1

 * SUBCRITICAL AND CRITICAL FLOW ASSUMPTIONS FROM *
 * SUBROUTINE HEAD. SEE FIGURE 5-4 IN THE EXTRAN *

MANUAL FOR FURTHER INFORMATION.

CONDUIT NUMBER	LENGTH OF DRY FLOW (MIN)	LENGTH OF SUBCRITICAL FLOW (MIN)	LENGTH OF UPSTR. OF CRITICAL FLOW (MIN)	LENGTH OF DOWNSTR. CRITICAL FLOW (MIN)	MEAN FLOW (CMS)	AVERAGE FLOW	TOTAL FLOW	MAXIMUM HYDRAULIC RADIUS (NET)	MAXIMUM CROSS SECT AREA (SQ.M)
1	0.00	1000.00	0.00	0.00	12.23	0.0669	7.3409E+05	1.6751	627.0582
2	0.25	999.75	0.00	0.00	12.06	0.0679	7.2343E+05	1.2850	473.5616
3	0.25	999.75	0.00	0.00	11.35	0.0253	6.8101E+05	1.4461	536.3661
4	0.50	999.50	0.00	0.00	10.39	0.0271	6.2365E+05	1.6343	610.7724
5	0.50	999.50	0.00	0.00	9.84	0.0374	5.9042E+05	0.9892	30.6229
6	1.00	999.00	0.00	0.00	9.82	0.1841	5.8922E+05	0.5952	3.1720
7	1.75	998.25	0.00	0.00	9.81	0.0357	5.8889E+05	0.7601	10.7336
8	2.25	997.75	0.00	0.00	9.80	0.0259	5.8828E+05	0.7439	10.5999
9	3.25	996.75	0.00	0.00	9.79	0.0190	5.8749E+05	0.7348	10.3456
10	3.75	996.25	0.00	0.00	9.78	0.0136	5.8673E+05	0.6199	7.4170
90011	UNDEFINED	UNDEFINED	UNDEFINED	UNDEFINED	9.78		5.8673E+05		

 * AVERAGE % CHANGE IN JUNCTION OR CONDUIT IS DEFINED AS: *
 * CONDUIT % CHANGE ==> 100.0 (Q(n+1) - Q(n)) / Qfull *
 * JUNCTION % CHANGE ==> 100.0 (Y(n+1) - Y(n)) / Yfull *

The Conduit with the largest average change... 6 had 0.184 percent
 The Junction with the largest average change... 1 had 0.068 percent

====> Extended Transport model simulation ended normally.

====> SWMM 4.4GU simulation ended normally.
 Always check output file for possible warning messages.

====> Your input file was named: PCTmpl.dat
 ====> Your output file was named: PCTmpl.out

 * SWMM 4.4GU Simulation Date and Time Summary *

 * Starting Date... November 17, 2006 *
 * Time... 9:15:35.600 *
 * Ending Date... November 17, 2006 *
 * Time... 9:15:49.50 *

```
* Elapsed Time... 0.224 minutes, *  
* Elapsed Time... 13.450 seconds, *  
*****
```

* U.S. Environmental Protection Agency *
* Storm Water Management Model (SWMM) *
* Version 4.4GU *
* *
* CDM/OSU Beta *
* Release Date - November 23, 1999 *
* Camp Dresser & McKee and Oregon State Univ. *
* Chuck Moore and Wayne Huber *
* Compiled using Digital Visual Fortran 6.0 *

Developed by

* Metcalf & Eddy, Inc. *
* University of Florida *
* Water Resources Engineers, Inc. *
* (Now Camp, Dresser and McKee, Inc.) *
* September 1970 *

Distributed and Maintained by

* U.S. Environmental Protection Agency *
* Center for Exposure Assessment Modeling (CEAM) *
* Athens Environmental Research Laboratory *
* 960 College Station Road *
* Athens, GA 30605-2720 *

* This is a new release of SWMM. If any *
* problems occur executing this model *
* system, contact Mr. Frank Stancil, *
* U.S. Environmental Protection Agency. *
* 706/355-8328 (voice) *
* e-mail: stancil@athens.ath.epa.gov *
* Or contact Wayne C. Huber at Oregon St. U. *
* 541/737-6150 or wayne.huber@orst.edu *
* Or Michael F. Schmidt at Camp Dresser & *
* McKee (904) 281-0170 SCHMIDTF@CDM.COM *

```

*****
* This is an implementation of EPA SWMM 4.4GU *
* "Nature is full of infinite causes which *
* have never occurred in experience" da Vinci *
*****

```

```

#####
# File names by SWMM Block #
# JIN -> Input to a Block #
# JOUT -> Output from a Block #
#####

```

```

JIN for Block # 1 File # 0 JIN.UF
JOUT for Block # 1 File # 9 PCTmp1.int

```

```

#####
# Scratch file names for this simulation. #
#####

```

```

NSCRAT # 1 File # 21 SCRT1.UF
NSCRAT # 2 File # 22 SCRT2.UF
NSCRAT # 3 File # 23 SCRT3.UF
NSCRAT # 4 File # 24 SCRT4.UF
NSCRAT # 5 File # 25 SCRT5.UF
NSCRAT # 6 File # 26 SCRT6.UF
NSCRAT # 7 File # 27 SCRT7.UF
NSCRAT # 8 File # 28 SCRT8.UF

```

```

*****
* Parameter Values on the Tapes Common Block *
*****

```

```

Number of Subcatchments in the Runoff Block (NW)..... 1000
Number of Channel/Pipes in the Runoff Block (NG)..... 1000
Number of Connections to Runoff Channels/Inlets (NCP). 6
Number of Water Quality Constituents (MQUAL)..... 20
Number of Runoff Land Uses per Subcatchment (NLU)..... 20
Number of Groundwater Subcatchments in Runoff (NGW)... 100
Number of Interface Locations for all Blocks (NIE).... 1000
Number of Elements in the Transport Block (NET)..... 500
Number of Storage Junctions in Transport (NTSE)..... 100
Number of Transport interface input locations (NTHI).. 500
Number of Transport interface output locations (NTHO). 500
Number of Transport input locations on R lines (NTHR). 80

```

Number of Transport printed output locations (NTOA)... 80
 Number of Tubular Flow Splitters in Transport (NTSP)... 50
 Number of Elements in the Extran Block (NEE)..... 4000
 Number of Pumps in Extran (NEP)..... 75
 Number of Orifices in Extran (NEO)..... 200
 Number of Tide Gates/Free Outfalls in Extran (NTG).... 200
 Number of Extran Weirs (NEW)..... 400
 Number of Extran Printout Locations (NPO)..... 150
 Number of Tide Elements in Extran (NTE)..... 50
 Number of Natural Channels (NNC)..... 1200
 Number of Storage Junctions in Extran (NVSE)..... 1000
 Number of Time History Data Points in Extran (NTVAL).. 500
 Number of Data Points for Variable Storage Elements
 in the Extran Block (NVST)..... 25
 Number of Input Hydrographs in Extran (NEH)..... 500
 Number of Allowable Channel Connections to
 Junctions in the Extran Block (NCHN)..... 15
 Number Rain Gages in Rain and Runoff (MAXRG)..... 200
 Number PRATE/VRATE Points for Extran Pump
 Input (MAXPRA)..... 10
 Number of Variable Orifices in Extran (NVORF)..... 50
 Number of Variable Orifice Data Points (NVOTIM)..... 50
 Number of Allowable Precip. Values/yr in Rain (LINRN). 5000
 Number of Storm Events for Rain Analysis (LSTORM).....20000
 Number of Plugs for Plug-flow in S/T (NPLUG)..... 3000
 Number Conduits for Extran Results to ASCII
 File (MXFLOW)..... 400

 * Entry made to the EXTENDED TRANSPORT MODEL (EXTRAN) *
 * developed 1973 by Camp, Dresser and McKee (CDM) with *
 * modifications 1977-1991 by the University of Florida. *
 *
 * Most recent update: March 1999 by CDM, Oregon *
 * State University, and XP Software, Inc. *
 *
 * "Smooth runs the water where the brook is deep." *
 * Shakespeare, Henry VI, II, III, 1 *

WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC. ANNANDALE, VIRGINIA *****

Detroit River International Crossing
Lennon Drain Alternative 1B,2B Syphon - 100-Year Flow

Control information for simulation

Integration cycles..... 12000

Length of integration step is..... 5.00 seconds

Simulation length..... 16.67 hours

Do not create equiv. pipes (NEQUAL). 0

Use metric units for I/O..... 1

Printing starts in cycle..... 1

Intermediate printout intervals of. 1 cycles

Intermediate printout intervals of. 0.08 minutes

Summary printout intervals of..... 1 cycles

Summary printout time interval of.. 0.08 minutes

Hot start file parameter (JREDO)... 0

Initial time (TZERO)..... 0.00 hours

This is time displacement from JIN interface file starting date/time when interface file is used.

This also describes starting hour in K3 line hydrograph input when K3 lines are used.

Initial date (IDATZ)..... 19060714 (yr/mo/day)

NOTE: Initial date from JIN interface file will be used, if accessed, unless IDATZ is negative.

Iteration variables: ITMAX..... 30

SURTOL..... 0.0500

Default surface area of junctions.. 1.22 square meters.

EXTRAN VERSION 3.3 SOLUTION. (ISOL = 0).

Sum of junction flow is zero during surcharge.

NORMAL FLOW OPTION WHEN THE WATER SURFACE SLOPE IS LESS THAN THE GROUND SURFACE SLOPE (KSUPER=0).....

NJSW INPUT HYDROGRAPH JUNCTIONS..... 0

INTERMEDIATE HEADER LINES ARE PRINTED AS IN ORIGINAL PROGRAM

IDS ARE WRITTEN AS IN ORIGINAL PROGRAM

CONDUIT LENGTHS ON C1 LINE MUST EQUAL IRREGULAR SECTION LENGTH ENTERED ON THE C3 OR X1 LINES (IWLEN = 0)

JELEV = 0 (DEFAULT). STANDARD INPUTS ARE DEPTHS NOT ELEVATIONS

JDOWN = 0 - Minimum of normal or critical depth will be used at free outfalls (I1).

Characteristic depth for M2 and S2 water surface profiles will be computed as in previous versions of EXTRAN (IM2 = 0).

SEDIMENT DEPTHS WILL NOT BE READ FROM C1 LINES

Intermediate continuity output will not be created

1-----
 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
 ***** ANALYSIS MODULE ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Lennon Drain Alternative 1B,2B Syphon - 100-Year Flow

1

 * Conduit Data *

INP NUM	CONDUIT NUMBER	LENGTH (M)	CONDUIT CLASS	AREA (SQ M)	MANNING COEF.	MAX WIDTH (M)	DEPTH (M)	JUNCTIONS AT THE ENDS	INVERT HEIGHT ABOVE JUNCTIONS	TRAPEZOID SIDE SLOPES
1	1	70.	TRAPEZOID	803.64	0.03500	350.00	2.20	1	2	5.20 8.70
2	2	70.	TRAPEZOID	803.64	0.03500	350.00	2.20	2	3	5.20 8.70
3	3	100.	TRAPEZOID	803.64	0.03500	350.00	2.20	3	4	5.20 8.70
4	4	100.	TRAPEZOID	803.64	0.03500	350.00	2.20	4	5	5.20 8.70
5	5	30.	RECTANGLE	3.12	0.01500	2.60	1.20	5	55	

6	55	35. RECTANGLE	4.50	0.01500	3.00	1.50	55	6	0.00	0.30
7	6	50. RECTANGLE	4.50	0.01500	3.00	1.50	6	66		
8	66	35. RECTANGLE	4.50	0.01500	3.00	1.50	66	7		
9	7	10. RECTANGLE	4.50	0.01500	3.00	1.50	7	8		
10	8	75. TRAPEZOID	18.18	0.03500	2.00	1.80	8	9	4.30	4.70
11	9	75. TRAPEZOID	18.18	0.03500	2.00	1.80	9	10	4.30	4.70
12	10	75. TRAPEZOID	18.18	0.03500	2.00	1.80	10	11	4.30	4.70

==> WARNING !!! (C*DELT/LEN) IN CONDUIT 7 IS 1.9 AT FULL DEPTH.

 * Conduit Volume *

Input full depth volume..... 2.7801E+05 cubic meters
 Conduit #... 6 has zero slope. 0.001 feet added to upstream invert.

==> Warning !! The upstream and downstream junctions for the following conduits have been reversed to correspond to the positive flow and decreasing slope EXTRAN convention. A negative flow in the output thus means the flow was from your original upstream junction to your original downstream junction. Any initial flow has been multiplied by -1.

1. Conduit #... 66 has been changed.

1-----
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 ***** ANALYSIS MODULE *****

Detroit River International Crossing
 Lennon Drain Alternative 1B,2B Syphon - 100-Year Flow

1

 * Junction Data *

INP NUM	JUNCTION NUMBER	GROUND ELEV.	CROWN ELEV.	INVERT ELEV.	QINST CMS	INITIAL DEPTH(M)	CONNECTING CONDUITS
1	1	183.00	182.61	180.41	11.80	0.00	1
2	2	183.00	182.47	180.27	0.00	0.00	1 2

3	183.00	182.33	180.13	0.00	0.00	2	3
4	183.00	182.13	179.93	0.00	0.00	3	4
5	183.00	181.93	179.73	0.00	0.00	4	5
6	183.00	181.17	179.67	0.00	0.00	5	55
7	183.00	170.90	169.10	0.00	0.00	55	6

6 lie above the Junction invert.

8	183.00	170.60	169.10	0.00	0.00	6	66
9	183.00	180.96	179.46	0.00	0.00	66	7
10	183.00	181.18	179.38	0.00	0.00	7	8
11	183.00	181.03	179.23	0.00	0.00	8	9
12	183.00	180.88	179.08	0.00	0.00	9	10
13	183.00	180.73	178.93	0.00	0.00	10	

====> Warning all conduits connecting to Junction

 * FREE OUTFALL DATA (DATA GROUP I1) *
 * BOUNDARY CONDITION ON DATA GROUP J1 *

OUTFALL AT JUNCTION... 11 HAS BOUNDARY CONDITION NUMBER... 1

1-----
 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC. *****
 ***** ANALYSIS MODULE ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Lennon Drain Alternative 1B,2B Syphon - 100-Year Flow

 * INTERNAL CONNECTIVITY INFORMATION *

CONDUIT	JUNCTION	JUNCTION
90013	11	0

1

 * BOUNDARY CONDITION INFORMATION *
 * DATA GROUPS J1-J4 *

BC NUMBER... 1 HAS NO CONTROL WATER SURFACE.
 TZERO = 1906195 0.000000E+00

 * INITIAL MODEL CONDITION *
 * INITIAL TIME = 0.00 HOURS *

JUNCTION / DEPTH / ELEVATION ==> "*" JUNCTION IS SURCHARGED.
 1/ 0.00 / 180.41 2/ 0.00 / 180.27 3/ 0.00 / 180.13
 4/ 0.00 / 179.93 5/ 0.00 / 179.73 55/ 0.00 / 179.67
 6/ 0.00 / 169.10 66/ 0.00 / 169.10 7/ 0.00 / 179.46
 8/ 0.00 / 179.38 9/ 0.00 / 179.23 10/ 0.00 / 179.08
 11/ 0.00 / 178.93

CONDUIT/ FLOW ==> "*" CONDUIT USES THE NORMAL FLOW OPTION.
 1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
 5/ 0.00 55/ 0.00 6/ 0.00 66/ 0.00
 7/ 0.00 8/ 0.00 9/ 0.00 10/ 0.00
 90013/ 0.00

CONDUIT/ VELOCITY
 1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
 5/ 0.00 55/ 0.00 6/ 0.00 66/ 0.00
 7/ 0.00 8/ 0.00 9/ 0.00 10/ 0.00

CONDUIT/ CROSS SECTIONAL AREA
 1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
 5/ 0.00 55/ 0.00 6/ 0.00 66/ 0.00
 7/ 0.00 8/ 0.00 9/ 0.00 10/ 0.00

CONDUIT/ HYDRAULIC RADIUS
 1/ 0.00 2/ 0.00 3/ 0.00 4/ 0.00
 5/ 0.00 55/ 0.00 6/ 0.00 66/ 0.00
 7/ 0.00 8/ 0.00 9/ 0.00 10/ 0.00

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION
 1/ 180.41/ 180.27 2/ 180.27/ 180.13 3/ 180.13/ 179.93
 4/ 179.93/ 179.73 5/ 179.73/ 179.67 55/ 179.67/ 169.40
 6/ 169.10/ 169.10 66/ 179.46/ 169.10 7/ 179.46/ 179.38
 8/ 179.38/ 179.23 9/ 179.23/ 179.08 10/ 179.08/ 178.93

* FINAL MODEL CONDITION *
 * FINAL TIME = 16.67 HOURS *

>>> ENDING DATE AND TIME OF EXTRAN RUN ARE:

JULIAN DATE: 1906195

YR/MO/DA: 1906/ 7/14

TIME OF DAY: 16.667 HRS

JUNCTION / DEPTH / ELEVATION ===> *** JUNCTION IS SURCHARGED.
 1/ 1.14 / 181.55 2/ 1.28 / 181.55 3/ 1.42 / 181.55
 4/ 1.62 / 181.55 5/ 1.82 / 181.55 55/ 1.60* / 181.27
 6/ 12.05* / 181.15 66/ 11.87* / 180.97 7/ 0.91 / 180.37
 8/ 1.32 / 180.70 9/ 1.31 / 180.54 10/ 1.28 / 180.36
 11/ 0.85 / 179.78

CONDUIT/ FLOW ===> ** CONDUIT USES THE NORMAL FLOW OPTION.

1/ 11.72 2/ 11.55 3/ 11.45 4/ 11.23
 5/ 11.16 55/ 11.16 6/ 11.16 66/ -11.16
 7/ 11.16* 8/ 11.16 9/ 11.16 10/ 11.16
 90013/ 11.16

CONDUIT/ VELOCITY
 1/ 0.03 2/ 0.02 3/ 0.02 4/ 0.02
 5/ 3.58 55/ 2.48 6/ 2.48 66/ -3.09
 7/ 3.34 8/ 1.07 9/ 1.10 10/ 1.54

CONDUIT/ CROSS SECTIONAL AREA

1/ 435.22 2/ 486.71 3/ 549.59 4/ 624.10
 5/ 3.12 55/ 4.50 6/ 4.50 66/ 3.62
 7/ 3.35 8/ 10.39 9/ 10.15 10/ 7.26

CONDUIT/ FINAL VOLUME

1/ 30465.41 2/ 34069.54 3/ 54958.80 4/ 62409.62
 5/ 93.60 55/ 157.50 6/ 225.00 66/ 126.59
 7/ 33.45 8/ 779.13 9/ 761.00 10/ 544.85

CONDUIT/ HYDRAULIC RADIUS

1/ 1.19 2/ 1.32 3/ 1.48 4/ 1.67
 5/ 0.41 55/ 0.50 6/ 0.50 66/ 0.67
 7/ 0.64 8/ 0.74 9/ 0.73 10/ 0.61

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/ 181.55/ 181.55 2/ 181.55/ 181.55 3/ 181.55/ 181.55
 4/ 181.55/ 181.55 5/ 181.55/ 181.27 55/ 181.27/ 181.15

6/ 181.15/ 180.97 66/ 180.37/ 180.97 7/ 180.37/ 180.70
 8/ 180.70/ 180.54 9/ 180.54/ 180.36 10/ 180.36/ 179.78

 # Surcharge Iteration Summary #
 #####

Maximum number of iterations in a time step..... 31
 Total number of iterations in the simulation.. 26677
 Average number of iterations per time step..... 2.22
 Surcharge iterations during the simulation..... 2677
 Maximum surcharge flow error during simulation.. 1.52E+00 cms
 Total number of time steps during simulation.. 12000

1

 * CONDUIT COURANT CONDITION SUMMARY *
 * TIME IN MINUTES DELT > COURANT TIME STEP *

 * SEE BELOW FOR EXPLANATION OF COURANT TIME STEP. *

CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)
1	0.00	2	0.00	3	0.00	4	0.00
5	832.67	55	990.67	6	990.00	66	989.92
7	989.58	8	0.00	9	0.00	10	0.00

1

 * CONDUIT COURANT CONDITION SUMMARY *

 * COURANT = CONDUIT LENGTH *
 * TIME STEP = ----- *
 * VELOCITY + SQRT(GRVT*AREA/WIDTH) *

 * AVERAGE COURANT CONDITION TIME STEP (SECONDS) *

CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)
1	24.99	2	23.01	3	30.32	4	28.57

5	4.82	55	2.82	6	4.72	66	2.69
7	1.69	8	20.78	9	21.25	10	20.95

1

 * EXTRAN CONTINUITY BALANCE AT THE LAST TIME STEP *

 * JUNCTION INFLOW, OUTFLOW OR STREET FLOODING *

JUNCTION INFLOW, CU M

1 7.0800E+05

JUNCTION OUTFLOW, CU M

11 5.5374E+05

 * INITIAL SYSTEM VOLUME = 7.2500E-03 CU M *
 * TOTAL SYSTEM INFLOW VOLUME = 7.0800E+05 CU M *
 * INFLOW + INITIAL VOLUME = 7.0800E+05 CU M *

 * TOTAL SYSTEM OUTFLOW = 5.5374E+05 CU M *
 * VOLUME LEFT IN SYSTEM = 1.8466E+05 CU M *
 * OUTFLOW + FINAL VOLUME = 7.3840E+05 CU M *

 * ERROR IN CONTINUITY, PERCENT = -4.29 *

TEST WRITE OF ALTERNATIVE CONTINUITY ERROR CALCULATION
 VOLUME LEFT IN SYSTEM = 1.5432E+05 CU. FT.
 ERROR IN CONTINUITY PERCENT = -0.08

----- SUMMARY OF FULL FLOW CHANNEL WARNINGS -----

OPEN CHANNEL NUMBER	TIME STEP OF FIRST OCCURRENCE	TIME OF FIRST OCCURRENCE (HOURS)	TIME STEP OF LAST OCCURRENCE	TIME OF LAST OCCURRENCE (HOURS)
1	1	0.00	1	0.00

THE PROGRAM USES FULL DEPTH CHANNEL CHARACTERISTICS TO COMPUTE FLOW THROUGH THE TRAPEZOIDAL, IRREGULAR, OR PARABOLIC/POWER FUNCTION CONDUIT WHEN THE COMPUTED DEPTHS EXCEED MAXIMUM DEPTH. THIS WILL AFFECT THE MAXIMUM COMPUTED HEAD AND FLOWS IN THE MODEL. IT IS HIGHLY RECOMMENDED THAT THE MODELED CROSS SECTIONS BE EXTENDED TO ELIMINATE THESE FULL FLOW CHANNEL WARNINGS

1

 * JUNCTION SUMMARY STATISTICS *

Detroit River International Crossing
 Lennon Drain Alternative 1B,2B Syphon - 100-Year Flow

JUNCTION NUMBER	GROUND ELEVATION (M)	UPPERMOST PIPE CROWN ELEVATION (M)	MEAN JUNCTION ELEVATION (M)	% CHANGE	AVERAGE ELEV. (M)	MAXIMUM JUNCTION ELEV. (M)	TIME OF OCCURENCE HR. MIN.	METERS SURCHARGE AT MAX ELEVATION	METERS MAX. DEPTH IS BELOW GROUND ELEVATION	LENGTH OF SURCHARGE (MIN)	LENGTH OF FLOODING (MIN)	MAXIMUM JUNCTION AREA (SQ.MET)
1	183.00	182.61	181.24	0.0231	182.52	182.52	0 0	0.00	0.48	0.0	0.0	2.607E+04
2	183.00	182.47	181.24	0.0114	181.55	181.55	16 40	0.00	1.45	0.0	0.0	2.575E+04
3	183.00	182.33	181.24	0.0093	181.55	181.55	16 40	0.00	1.45	0.0	0.0	3.145E+04
4	183.00	182.13	181.23	0.0122	181.55	181.55	16 40	0.00	1.45	0.0	0.0	3.726E+04
5	183.00	181.93	181.23	0.0176	181.55	181.55	16 40	0.00	1.45	0.0	0.0	1.877E+04
55	183.00	181.17	180.94	1.4442	181.28	181.28	15 1	0.11	1.72	341.2	0.0	1.668E+03
6	183.00	170.90	180.81	1.1602	181.17	181.17	15 1	10.27	1.83	990.3	0.0	3.954E+03
66	183.00	170.60	180.68	0.9829	181.02	181.02	15 0	10.42	1.98	990.7	0.0	5.581E+03
7	183.00	180.96	180.47	0.6701	180.61	180.61	4 40	0.00	2.39	0.0	0.0	3.256E+03
8	183.00	181.18	180.58	0.2041	180.71	180.71	15 0	0.00	2.29	0.0	0.0	5.390E+02
9	183.00	181.03	180.42	0.0629	180.54	180.54	15 1	0.00	2.46	0.0	0.0	1.037E+03
10	183.00	180.88	180.25	0.0223	180.37	180.37	15 1	0.00	2.63	0.0	0.0	9.870E+02
11	183.00	180.73	179.69	0.0197	179.79	179.79	15 1	0.00	3.21	0.0	0.0	8.023E+02

1

 * CONDUIT SUMMARY STATISTICS *

Detroit River International Crossing
 Lennon Drain Alternative 1B,2B Syphon - 100-Year Flow

CONDUIT	MAXIMUM	TIME	MAXIMUM	TIME	RATIO OF	MAXIMUM DEPTH ABOVE	LENGTH	CONDUIT
---------	---------	------	---------	------	----------	---------------------	--------	---------

CONDUIT NUMBER	DESIGN FLOW		VERTICAL DEPTH		COMPUTED FLOW		OF OCCURRENCE		COMPUTED VELOCITY		OF OCCURRENCE		MAX. TO INV. AT CONDUIT ENDS		OF NORM FLOW		
	(CMS)	(M/S)	(M)	(M)	(CMS)	(CMS)	HR.	MIN.	(MPS)	(MPS)	HR.	MIN.	DESIGN FLOW	UPSTREAM (M)	DOWNSTREAM (M)	(MIN)	(M/M)
1	1.69E+03	2.10	2.200	2.200	7.32E+02	0	0	0	2.04	0	0	0	0.43	2.11	1.28	28.7	0.00200
2	1.69E+03	2.10	2.200	2.200	2.50E+02	0	0	0	1.47	0	0	0	0.15	1.28	1.42	4.4	0.00200
3	1.69E+03	2.10	2.200	2.200	9.72E+01	0	2	0	0.88	0	2	0	0.06	1.42	1.62	2.8	0.00200
4	1.69E+03	2.10	2.200	2.200	7.55E+01	0	4	0	0.68	0	4	0	0.04	1.62	1.82	0.7	0.00200
5	5.14E+00	1.65	1.200	1.200	1.23E+01	4	28	0	3.94	4	28	0	2.39	1.82	1.61	17.0	0.00200
55	1.02E+02	22.75	1.500	1.500	1.13E+01	14	56	0	7.84	0	2	0	0.11	1.61	11.77	9.4	0.29343
6	8.48E-01	0.19	1.500	1.500	1.13E+01	14	56	0	2.50	14	56	0	13.28	12.07	11.93	0.6	0.00002
66	1.03E+02	22.85	1.500	1.500	-1.13E+01	14	56	0	-3.09	15	1	0	-0.11	1.15	11.93	0.0	0.29600
7	1.69E+01	3.76	1.500	1.500	1.17E+01	15	0	0	3.43	15	0	0	0.69	1.15	1.33	108.4	0.00800
8	2.29E+01	1.26	1.800	1.800	1.14E+01	15	0	0	1.09	0	12	0	0.50	1.33	1.31	17.2	0.00200
9	2.29E+01	1.26	1.800	1.800	1.13E+01	15	1	1	1.11	15	1	0	0.49	1.31	1.29	8.5	0.00200
10	2.29E+01	1.26	1.800	1.800	1.13E+01	15	1	1	1.54	15	1	0	0.49	1.29	0.86	0.0	0.00200
90013	UNDEF	UNDEF	UNDEF	UNDEF	1.13E+01	15	1	1									

1

 * SUBCRITICAL AND CRITICAL FLOW ASSUMPTIONS FROM *
 * SUBROUTINE HEAD. SEE FIGURE 5-4 IN THE EXTRAN *
 * MANUAL FOR FURTHER INFORMATION. *

CONDUIT NUMBER	LENGTH OF DRY		LENGTH OF SUBCRITICAL		LENGTH OF UPSTR. OF DOWNSTR. CRITICAL		MEAN FLOW (CMS)	AVERAGE % CHANGE	TOTAL FLOW		MAXIMUM	
	FLOW(MIN)	FLOW(MIN)	FLOW(MIN)	FLOW(MIN)	FLOW(MIN)	FLOW(CUBIC MET)			CROSS SECT AREA(SQ.M)	FLOW	HYDRAULIC RADIUS (MET)	FLOW
1	0.00	1000.00	0.00	0.00	0.00	0.00	12.03	0.0125	7.2191E+05	1.1857	435.2089	
2	0.08	999.92	0.00	0.00	0.00	0.00	11.53	0.0059	6.9179E+05	1.3189	486.6992	
3	0.08	999.92	0.00	0.00	0.00	0.00	10.80	0.0084	6.4818E+05	1.4797	549.5795	
4	0.25	999.75	0.00	0.00	0.00	0.00	9.82	0.0168	5.8950E+05	1.6677	624.0876	
5	0.83	999.17	0.00	0.00	0.00	0.00	9.27	3.2861	5.5639E+05	0.5899	3.1200	
55	1.67	993.92	0.00	0.00	4.42	0.00	9.27	0.0759	5.5624E+05	0.7104	4.5000	
6	1.83	998.17	0.00	0.00	0.00	0.00	9.26	9.0791	5.5584E+05	0.7045	4.5000	
66	9.92	990.08	0.00	0.00	0.00	0.00	-9.26	0.0750	-5.5581E+05	0.7033	3.9726	
7	10.00	990.00	0.00	0.00	0.00	0.00	9.26	0.8226	5.5565E+05	0.6718	3.6499	
8	10.08	989.92	0.00	0.00	0.00	0.00	9.26	0.2413	5.5531E+05	0.7406	10.5079	
9	10.17	989.83	0.00	0.00	0.00	0.00	9.24	0.0746	5.5448E+05	0.7306	10.2297	
10	10.75	989.25	0.00	0.00	0.00	0.00	9.23	0.0247	5.5373E+05	0.6160	7.3254	
90013	UNDEFINED	UNDEFINED	UNDEFINED	UNDEFINED	UNDEFINED	UNDEFINED	9.23		5.5373E+05			

* AVERAGE % CHANGE IN JUNCTION OR CONDUIT IS DEFINED AS: *
* CONDUIT % CHANGE ==> 100.0 (Q(n+1) - Q(n)) / Qfull *
* JUNCTION % CHANGE ==> 100.0 (Y(n+1) - Y(n)) / Yfull *

The Conduit with the largest average change... 6 had 9.079 percent
The Junction with the largest average change... 55 had 1.444 percent

==> Extended Transport model simulation ended normally.

==> SWMM 4.4GU simulation ended normally.
Always check output file for possible warning messages.

==> Your input file was named : PCTmpl.dat
==> Your output file was named: PCTmpl.out

* SWMM 4.4GU Simulation Date and Time Summary *

* Starting Date... November 17, 2006 *
* Time... 9:35:20.890 *
* Ending Date... November 17, 2006 *
* Time... 9:36: 1.700 *
* Elapsed Time... 0.680 minutes. *
* Elapsed Time... 40.809 seconds. *

Appendix A.2.4.3

Cahill Drain

```

*****
* U.S. Environmental Protection Agency
* Storm Water Management Model (SWMM)
* Version 4.4GU
*
* CDM/OSU Beta
* Release Date - November 23, 1999
* Camp Dresser & McKee and Oregon State Univ.
* Chuck Moore and Wayne Huber
* Compiled using Digital Visual Fortran 6.0
*****

```

Developed by

```

*****
* Metcalf & Eddy, Inc.
* University of Florida
* Water Resources Engineers, Inc.
* (Now Camp, Dresser and McKee, Inc.)
* September 1970
*****

```

Distributed and Maintained by

```

*****
* U.S. Environmental Protection Agency
* Center for Exposure Assessment Modeling (CEAM)
* Athens Environmental Research Laboratory
* 960 College Station Road
* Athens, GA 30605-2720
*****

```

```

*****
* This is a new release of SWMM. If any
* problems occur executing this model
* system, contact Mr. Frank Stancil,
* U.S. Environmental Protection Agency.
* 706/355-8328 (voice)
* e-mail: stancil@athens.ath.epa.gov
* Or contact Wayne C. Huber at Oregon St. U.
* 541/737-6150 or wayne.huber@orst.edu
* Or Michael F. Schmidt at Camp Dresser &
* McKee (904) 281-0170 SCHMIDTF@CDM.COM
*****

```

```

*****
* This is an implementation of EPA SWMM 4.4GU *
* "Nature is full of infinite causes which *
* have never occurred in experience" da Vinci *
*****

```

```

#####
# File names by SWMM Block #
# JIN -> Input to a Block #
# JOUT -> Output from a Block #
#####

```

```

JIN for Block # 1 File # 0 JIN.UF
JOUT for Block # 1 File # 9 PCTmpl.int

```

```

#####
# Scratch file names for this simulation. #
#####

```

```

NSCRAT # 1 File # 21 SCRT1.UF
NSCRAT # 2 File # 22 SCRT2.UF
NSCRAT # 3 File # 23 SCRT3.UF
NSCRAT # 4 File # 24 SCRT4.UF
NSCRAT # 5 File # 25 SCRT5.UF
NSCRAT # 6 File # 26 SCRT6.UF
NSCRAT # 7 File # 27 SCRT7.UF
NSCRAT # 8 File # 28 SCRT8.UF

```

```

*****
* Parameter Values on the Tapes Common Block *
*****

```

```

Number of Subcatchments in the Runoff Block (NW)..... 1000
Number of Channel/Pipes in the Runoff Block (NG)..... 1000
Number of Connections to Runoff Channels/Inlets (NCP). 6
Number of Water Quality Constituents (MQUAL)..... 20
Number of Runoff Land Uses per Subcatchment (NLU).... 20
Number of Groundwater Subcatchments in Runoff (NGW)... 100
Number of Interface Locations for all Blocks (NIE).... 1000
Number of Elements in the Transport Block (NET)..... 500
Number of Storage Junctions in Transport (NTSE)..... 100
Number of Transport interface input locations (NTHI).. 500
Number of Transport interface output locations (NTHO). 500
Number of Transport input locations on R Lines (NTHR). 80

```

Number of Transport printed output locations (NTOA)... 80
 Number of Tabular Flow Splitters in Transport (NTSP).. 50
 Number of Elements in the Extran Block (NEE)..... 4000
 Number of Pumps in Extran (NEP)..... 75
 Number of Orifices in Extran (NEO)..... 200
 Number of Tide Gates/Free Outfalls in Extran (NTG).... 200
 Number of Extran Weirs (NEW)..... 400
 Number of Extran Printout Locations (NPO)..... 150
 Number of Tide Elements in Extran (NTE)..... 50
 Number of Natural Channels (NNC)..... 1200
 Number of Storage Junctions in Extran (NVSE)..... 1000
 Number of Time History Data Points in Extran (NTVAL).. 500
 Number of Data Points for Variable Storage Elements
 in the Extran Block (NVST)..... 25
 Number of Input Hydrographs in Extran (NEH)..... 500
 Number of Allowable Channel Connections to
 Junctions in the Extran Block (NCHN)..... 15
 Number Rain Gages in Rain and Runoff (MAXRG)..... 200
 Number PRATE/VRATE Points for Extran Pump
 Input (MAXPRA)..... 10
 Number of Variable Orifices in Extran (NVORE)..... 50
 Number of Variable Orifice Data Points (NVOTIM)..... 50
 Number of Allowable Precip. Values/yr in Rain (LIMRN). 5000
 Number of Storm Events for Rain Analysis (LSTORM).....20000
 Number of Plugs for Plug-flow in S/T (NPLUG)..... 3000
 Number Conduits for Extran Results to ASCII
 File (MKFLOW)..... 400

 * Entry made to the EXTENDED TRANSPORT MODEL (EXTRAN) *
 * developed 1973 by Camp, Dresser and McKee (CDM) with *
 * modifications 1977-1991 by the University of Florida. *
 *
 * Most recent update: March 1999 by CDM, Oregon *
 * State University, and XP Software, Inc. *
 *
 * "Smooth runs the water where the brook is deep." *
 * Shakespear, Henry VI, II, III, 1 *

WASHINGTON, D.C.

ANALYSIS MODULE

CAMP DRESSER & MCKEE INC.
ANNANDALE, VIRGINIA

Detroit River International Crossing
Wolfe/Cahill Drain Existing Condition - 100-Year Flow

Control information for simulation

Integration cycles..... 1440

Length of integration step is..... 5.00 seconds

Simulation length..... 2.00 hours

Do not create equiv. pipes (NEQUAL). 0

Use metric units for I/O..... 1

Printing starts in cycle..... 1

Intermediate printout intervals of. 1 cycles

Intermediate printout intervals of. 0.08 minutes

Summary printout intervals of..... 1 cycles

Summary printout time interval of.. 0.08 minutes

Hot start file parameter (JREDO)... 0

Initial time (TZERO)..... 0.00 hours

This is time displacement from JIN interface file starting date/time when interface file is used.

This also describes starting hour in K3 line hydrograph input when K3 lines are used.

Initial date (IDATZ)..... 19060714 (yr/mo/day)

NOTE: Initial date from JIN interface file will be used, if accessed, unless IDATZ is negative.

Iteration variables: ITMAX..... 30

SURTOL..... 0.0500

Default surface area of junctions.. 1.22 square meters.

EXTRAH VERSION 3.3 SOLUTION. (ISOL = 0).

Sum of junction flow is zero during surcharge.

NORMAL FLOW OPTION WHEN THE WATER SURFACE SLOPE IS LESS THAN THE GROUND SURFACE SLOPE (KSUPER=0).....

NJSW INPUT HYDROGRAPH JUNCTIONS.... 0

INTERMEDIATE HEADER LINES ARE PRINTED AS IN ORIGINAL PROGRAM

IDS ARE WRITTEN AS IN ORIGINAL PROGRAM

CONDUIT LENGTHS ON C1 LINE MUST EQUAL IRREGULAR SECTION LENGTH ENTERED ON THE C3 OR X1 LINES (IWLEN = 0)

JELEV = 0 (DEFAULT). STANDARD INPUTS ARE DEPTHS NOT ELEVATIONS

JDOWN = 0 - Minimum of normal or critical depth will be used at free outfalls (I1).

Characteristic depth for M2 and S2 water surface profiles will be computed as in previous versions of EXTRAN (IM2 = 0).

SEDIMENT DEPTHS WILL NOT BE READ FROM C1 LINES

Intermediate continuity output will not be created

```

1-----
ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
***** ANALYSIS MODULE ***** ANNANDALE, VIRGINIA
    
```

Detroit River International Crossing
 Wolfe/Cahill Drain Existing Condition - 100-Year Flow

```

1 *****
* Conduit Data *
*****
    
```

INP NUM	CONDUIT NUMBER	LENGTH (M)	CONDUIT CLASS	AREA (SQ M)	MANNING MAX COEF.	WIDTH (M)	DEPTH (M)	JUNCTIONS AT THE ENDS	INVERT HEIGHT ABOVE JUNCTIONS	TRAPEZOID SIDE SLOPES
1	1	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	1	2.00	2.00
2	2	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	2	2.00	2.00
3	3	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	3	2.00	2.00
4	4	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	4	2.00	2.00
5	5	40.	RECTANGLE	6.75	0.01500	4.50	1.50	5	2.00	2.00
	6							6		

6	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	6	2.00	2.00
7	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	7	2.00	2.00
8	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	8	2.00	2.00
9	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	9	2.00	2.00
10	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	10	2.00	2.00
11	100.	TRAPEZOID	12.27	0.03500	0.91	2.26	11	2.00	2.00

 * Conduit Volume *

Input full depth volume..... 1.1315E+04 cubic meters

1-----
 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
 ***** ANALYSIS MODULE ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Wolfe/Cahill Drain Existing Condition - 100-Year Flow

1 *****
 * Junction Data *

INP NUM	JUNCTION NUMBER	GROUND ELEV.	CROWN ELEV.	INVERT ELEV.	QINST CMS	INITIAL DEPTH(M)	CONNECTING CONDUITS
1	1	183.00	181.74	179.48	12.10	0.00	1
2	2	183.00	181.50	179.24	0.00	0.00	1 2
3	3	183.00	181.26	179.00	0.00	0.00	2 3
4	4	183.00	181.02	178.76	0.00	0.00	3 4
5	5	183.00	180.78	178.52	0.00	0.00	4 5
6	6	183.00	180.68	178.42	0.00	0.00	5 6
7	7	183.00	180.44	178.18	0.00	0.00	6 7
8	8	183.00	180.20	177.94	0.00	0.00	7 8
9	9	183.00	179.96	177.70	0.00	0.00	8 9
10	10	183.00	179.72	177.46	0.00	0.00	9 10
11	11	183.00	179.48	177.22	0.00	0.00	10

* FREE OUTFALL DATA (DATA GROUP I1) *
 * BOUNDARY CONDITION ON DATA GROUP J1 *

OUTFALL AT JUNCTION... 11 HAS BOUNDARY CONDITION NUMBER... 1

1-----

ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Wolfe/Cahill Drain Existing Condition - 100-Year Flow

 * INTERNAL CONNECTIVITY INFORMATION *

CONDUIT JUNCTION JUNCTION

 90011 11 0

 * BOUNDARY CONDITION INFORMATION *
 * DATA GROUPS J1-J4 *

BC NUMBER.. 1 HAS NO CONTROL WATER SURFACE.
 TZERO = 1906195 0.0000000E+00

 * INITIAL MODEL CONDITION *
 * INITIAL TIME = 0.00 HOURS *

JUNCTION /	DEPTH /	ELEVATION	====>	*****	JUNCTION IS	SURCHARGED.
1/	0.00 /	179.48	2/	0.00 /	179.24	3/ 0.00 / 179.00
4/	0.00 /	178.76	5/	0.00 /	178.52	6/ 0.00 / 178.42
7/	0.00 /	178.18	8/	0.00 /	177.94	9/ 0.00 / 177.70
10/	0.00 /	177.46	11/	0.00 /	177.22	

CONDUIT/ FLOW ==> "*" CONDUIT USES THE NORMAL FLOW OPTION.

1/	0.00	2/	0.00	3/	0.00	4/	0.00
5/	0.00	6/	0.00	7/	0.00	8/	0.00
9/	0.00	10/	0.00	90011/	0.00		

CONDUIT/ VELOCITY

1/	0.00	2/	0.00	3/	0.00	4/	0.00
5/	0.00	6/	0.00	7/	0.00	8/	0.00
9/	0.00	10/	0.00				

CONDUIT/ CROSS SECTIONAL AREA

1/	0.00	2/	0.00	3/	0.00	4/	0.00
5/	0.00	6/	0.00	7/	0.00	8/	0.00
9/	0.00	10/	0.00				

CONDUIT/ HYDRAULIC RADIUS

1/	0.00	2/	0.00	3/	0.00	4/	0.00
5/	0.00	6/	0.00	7/	0.00	8/	0.00
9/	0.00	10/	0.00				

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/	179.48/	179.24	2/	179.24/	179.00	3/	179.00/	178.76
4/	178.76/	178.52	5/	178.52/	178.42	6/	178.42/	178.18
7/	178.18/	177.94	8/	177.94/	177.70	9/	177.70/	177.46
10/	177.46/	177.22						

 * FINAL MODEL CONDITION *
 * FINAL TIME = 2.00 HOURS *

>>> ENDING DATE AND TIME OF EXTRAN RUN ARE:

JULIAN DATE: 1906195
 YR/MO/DA: 1906/ 7/14
 TIME OF DAY: 2.000 HRS

JUNCTION / DEPTH / ELEVATION ==> "*" JUNCTION IS SURCHARGED.

1/	1.90 /	181.38	2/	1.90 /	181.14	3/	1.89 /	180.89
4/	1.88 /	180.64	5/	1.86 /	180.38	6/	1.90 /	180.32
7/	1.90 /	180.08	8/	1.89 /	179.83	9/	1.88 /	179.58
10/	1.87 /	179.33	11/	1.29 /	178.51			

CONDUIT/ FLOW ==> "*" CONDUIT USES THE NORMAL FLOW OPTION.

1/	12.10	2/	12.10	3/	12.10	4/	12.10
----	-------	----	-------	----	-------	----	-------

5/	12.10	6/	12.10	7/	12.10	8/	12.10
9/	12.10	10/	12.10	90011/	12.10		

CONDUIT/ VELOCITY

1/	1.36	2/	1.36	3/	1.37	4/	1.39
5/	1.79	6/	1.36	7/	1.36	8/	1.37
9/	1.38	10/	1.89				

CONDUIT/ CROSS SECTIONAL AREA

1/	8.92	2/	8.89	3/	8.84	4/	8.70
5/	6.75	6/	8.92	7/	8.90	8/	8.85
9/	8.75	10/	6.41				

CONDUIT/ FINAL VOLUME

1/	891.86	2/	889.28	3/	883.55	4/	870.33
5/	270.00	6/	892.25	7/	890.12	8/	885.41
9/	874.71	10/	641.08				

CONDUIT/ HYDRAULIC RADIUS

1/	0.95	2/	0.95	3/	0.95	4/	0.94
5/	0.56	6/	0.95	7/	0.95	8/	0.95
9/	0.94	10/	0.80				

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/	181.38/ 181.14	2/	181.14/ 180.89	3/	180.89/ 180.64
4/	180.64/ 180.38	5/	180.38/ 180.32	6/	180.32/ 180.08
7/	180.08/ 179.83	8/	179.83/ 179.58	9/	179.58/ 179.33
10/	179.33/ 178.51				

 # Surcharge Iteration Summary #
 #####

Maximum number of iterations in a time step..... 6
 Total number of iterations in the simulation.. 2890
 Average number of iterations per time step..... 2.01
 Surcharge iterations during the simulation..... 10
 Maximum surcharge flow error during simulation.. 3.97E-02 cms
 Total number of time steps during simulation.. 1440

* CONDUIT COURANT CONDITION SUMMARY *
 * TIME IN MINUTES DELT > COURANT TIME STEP *

 * SEE BELOW FOR EXPLANATION OF COURANT TIME STEP. *

CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)
1	0.00	2	0.00	3	0.00	4	0.00
5	0.00	6	0.00	7	0.00	8	0.00
9	0.00	10	0.00				

1

 * CONDUIT COURANT CONDITION SUMMARY *

 * COURANT = CONDUIT LENGTH *
 * TIME STEP = VELOCITY + SQRT(GRVT*AREA/WIDTH) *

 * AVERAGE COURANT CONDITION TIME STEP(SECONDS) *

CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)
1	21.85	2	21.96	3	23.41	4	30.44
5	12.05	6	29.42	7	42.38	8	37.77
9	33.44	10	56.77				

1

 * EXTRAN CONTINUITY BALANCE AT THE LAST TIME STEP *

 * JUNCTION INFLOW, OUTFLOW OR STREET FLOODING *

JUNCTION	INFLOW, CU M
1	8.7120E+04

JUNCTION	OUTFLOW, CU M
1	3.6105E+01

11 7.9703E+04

```

*****
* INITIAL SYSTEM VOLUME = 9.4000E-03 CU M *
* TOTAL SYSTEM INFLOW VOLUME = 8.7120E+04 CU M *
* INFLOW + INITIAL VOLUME = 8.7120E+04 CU M *
*****
* TOTAL SYSTEM OUTFLOW = 7.9739E+04 CU M *
* VOLUME LEFT IN SYSTEM = 8.0054E+03 CU M *
* OUTFLOW + FINAL VOLUME = 8.7744E+04 CU M *
*****
* ERROR IN CONTINUITY, PERCENT = -0.72 *
*****

```

```

TEST WRITE OF ALTERNATIVE CONTINUITY ERROR CALCULATION
VOLUME LEFT IN SYSTEM = 7.3918E+03 CU. FT.
ERROR IN CONTINUITY PERCENT = -0.12

```

----- SUMMARY OF FULL FLOW CHANNEL WARNINGS -----

OPEN CHANNEL NUMBER	TIME STEP OF FIRST OCCURRENCE	TIME OF FIRST OCCURRENCE (HOURS)	TIME STEP OF LAST OCCURRENCE	TIME OF LAST OCCURRENCE (HOURS)
1	1	0.00	4	0.01

THE PROGRAM USES FULL DEPTH CHANNEL CHARACTERISTICS TO COMPUTE FLOW THROUGH THE TRAPEZOIDAL, IRREGULAR, OR PARABOLIC/POWER FUNCTION CONDUIT WHEN THE COMPUTED DEPTHS EXCEED MAXIMUM DEPTH. THIS WILL AFFECT THE MAXIMUM COMPUTED HEAD AND FLOWS IN THE MODEL. IT IS HIGHLY RECOMMENDED THAT THE MODELED CROSS SECTIONS BE EXTENDED TO ELIMINATE THESE FULL FLOW CHANNEL WARNINGS

1

* JUNCTION SUMMARY STATISTICS *

Detroit River International Crossing
Wolfe/Cahill Drain Existing Condition - 100-Year Flow

UPPERMOST GROUND ELEVATION	MEAN PIPE CROWN ELEVATION	MAXIMUM JUNCTION AVERAGE ELEV.	TIME OF OCCURRENCE AT MAX	METERS SURCHARGE BELOW GROUND	DEPTH IS	METERS OF SURCHARGE AT MAX	LENGTH OF FLOODING	LENGTH OF SURCHARGE	MAXIMUM JUNCTION AREA
----------------------------	---------------------------	--------------------------------	---------------------------	-------------------------------	----------	----------------------------	--------------------	---------------------	-----------------------

NUMBER	(M)	(M)	(M)	% CHANGE	(M)	HR. MIN.	ELEVATION	ELEVATION	(MIN)	(MIN)	(SQ.MET)
1	183.00	181.74	181.38	0.1772	183.00	0	0	1.26	0.00	0.3	2.438E+03
2	183.00	181.50	181.12	0.0655	181.14	0	54	0.00	1.86	0.0	1.701E+03
3	183.00	181.26	180.86	0.0638	180.89	1	22	0.00	2.11	0.0	8.485E+02
4	183.00	181.02	180.60	0.0664	180.64	1	55	0.00	2.36	0.0	8.444E+02
5	183.00	180.78	180.31	0.0705	180.38	1	4	0.00	2.62	0.0	5.098E+02
6	183.00	180.68	180.25	0.0716	180.32	1	4	0.00	2.68	0.0	5.162E+02
7	183.00	180.44	179.99	0.0614	180.08	1	4	0.00	2.92	0.0	8.506E+02
8	183.00	180.20	179.73	0.0610	179.83	1	5	0.00	3.17	0.0	8.491E+02
9	183.00	179.96	179.46	0.0604	179.59	1	23	0.00	3.41	0.0	8.457E+02
10	183.00	179.72	179.19	0.0598	179.33	1	9	0.00	3.67	0.0	8.110E+02
11	183.00	179.48	178.41	0.0414	178.51	1	40	0.00	4.49	0.0	6.651E+02

1

 * CONDUIT SUMMARY STATISTICS *

Detroit River International Crossing
 Wolfe/Cahill Drain Existing Condition - 100-Year Flow

CONDUIT NUMBER	DESIGN FLOW (CMS)	DESIGN VELOCITY (M/S)	CONDUIT VERTICAL DEPTH (M)	MAXIMUM COMPUTED FLOW (CMS)	TIME OF OCCURRENCE HR. MIN.	MAXIMUM COMPUTED VELOCITY (MPS)	TIME OF OCCURRENCE HR. MIN.	RATIO OF MAXIMUM DEPTH ABOVE INV. AT CONDUIT ENDS OF NORH SLOPE		LENGTH CONDUIT (M)		
								DESIGN UPSTREAM FLOW (M)	DOWNSTREAM FLOW (M/M)			
1	1.85E+01	1.50	2.260	1.64E+01	0	0	0	0.89	3.52	1.90	0.2	0.00240
2	1.85E+01	1.50	2.260	1.29E+01	0	2	0	0.70	1.90	1.89	0.1	0.00240
3	1.85E+01	1.50	2.260	1.22E+01	0	3	0	0.66	1.89	1.88	0.8	0.00240
4	1.85E+01	1.50	2.260	1.31E+01	0	4	0	0.71	1.88	1.86	0.0	0.00240
5	1.53E+01	2.27	1.500	1.33E+01	0	4	0	0.87	1.86	1.90	0.4	0.00250
6	1.85E+01	1.50	2.260	1.21E+01	1	28	0	0.66	1.90	1.90	0.0	0.00240
7	1.85E+01	1.50	2.260	1.21E+01	1	47	0	0.66	1.90	1.89	0.0	0.00240
8	1.85E+01	1.50	2.260	1.21E+01	1	28	0	0.66	1.89	1.89	0.0	0.00240
9	1.85E+01	1.50	2.260	1.21E+01	1	23	0	0.66	1.89	1.87	0.0	0.00240
10	1.85E+01	1.50	2.260	1.21E+01	1	40	1	0.66	1.87	1.87	0.0	0.00240
90011	UNDEF	UNDEF	UNDEF	1.21E+01	1	40	1	0.66	1.87	1.29	0.0	0.00240

1

 * SUBCRITICAL AND CRITICAL FLOW ASSUMPTIONS FROM *
 * SUBROUTINE HEAD. SEE FIGURE 5-4 IN THE EXTRAN *

MANUAL FOR FURTHER INFORMATION.

CONDUIT NUMBER	LENGTH OF DRY FLOW (MIN)	LENGTH OF SUBCRITICAL FLOW (MIN)	LENGTH OF UPSTR. OF CRITICAL FLOW (MIN)	LENGTH OF DOWNSTR. CRITICAL FLOW (MIN)	MEAN FLOW (CMS)	AVERAGE % CHANGE	TOTAL FLOW CUBIC MET	MAXIMUM HYDRAULIC RADIUS (MET)	MAXIMUM CROSS SECT AREA (SQ.M)
1	0.00	120.00	0.00	0.00	12.11	0.1154	8.7187E+04	0.9497	8.9187
2	0.08	119.92	0.00	0.00	11.98	0.0708	8.6267E+04	0.9483	8.8929
3	0.08	119.92	0.00	0.00	11.86	0.0753	8.5367E+04	0.9453	8.8356
4	0.33	119.67	0.00	0.00	11.74	0.0842	8.4494E+04	0.9382	8.7034
5	0.92	119.08	0.00	0.00	11.64	0.1979	8.3829E+04	0.8361	6.7500
6	1.58	118.42	0.00	0.00	11.56	0.0648	8.3255E+04	0.9499	8.9225
7	2.33	117.67	0.00	0.00	11.44	0.0546	8.2361E+04	0.9488	8.9013
8	3.17	116.83	0.00	0.00	11.32	0.0502	8.1470E+04	0.9463	8.8543
9	3.92	116.08	0.00	0.00	11.19	0.0459	8.0586E+04	0.9405	8.7473
10	4.92	115.08	0.00	0.00	11.07	0.0459	7.9702E+04	0.8050	6.4109
90011	UNDEFINED	UNDEFINED	UNDEFINED	UNDEFINED	11.07		7.9702E+04		

 * AVERAGE % CHANGE IN JUNCTION OR CONDUIT IS DEFINED AS: *
 * CONDUIT % CHANGE ==> 100.0 (Q(n+1) - Q(n)) / Qfull *
 * JUNCTION % CHANGE ==> 100.0 (Y(n+1) - Y(n)) / Yfull *

The Conduit with the largest average change... 5 had 0.198 percent
 The Junction with the largest average change... 1 had 0.177 percent

==> Extended Transport model simulation ended normally.

==> SWMM 4.4GU simulation ended normally.
 Always check output file for possible warning messages.

==> Your input file was named : PCTmpl.dat
 ==> Your output file was named: PCTmpl.out

 * SWMM 4.4GU Simulation Date and Time Summary *

 * Starting Date... November 16, 2006 *
 * Time... 16:31:49.300 *
 * Ending Date... November 16, 2006 *
 * Time... 16:31:53.630 *

```
* Elapsed Time... 0.072 minutes.*  
* Elapsed Time... 4.332 seconds.*  
*****
```



```

*****
* U.S. Environmental Protection Agency
* Storm Water Management Model (SWMM)
* Version 4.4GU
*
* CDM/OSU Beta
* Release Date - November 23, 1991
* Camp Dresser & McKee and Oregon State Univ.
* Chuck Moore and Wayne Huber
* Compiled using Digital Visual Fortran 6.0
*****

```

Developed by

```

*****
* Metcalf & Eddy, Inc.
* University of Florida
* Water Resources Engineers, Inc.
* (How Camp, Dresser and McKee, Inc.)
* September 1970
*****

```

Distributed and Maintained by

```

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* U.S. Environmental Protection Agency
* Center for Exposure Assessment Modeling (CEAM)
* Athens Environmental Research Laboratory
* 960 College Station Road
* Athens, GA 30605-2720
*****

```

```

*****
* This is a new release of SWMM. If any
* problems occur executing this model
* system, contact Mr. Frank Stancil
* U.S. Environmental Protection Agency.
* 706/355-8328 (voice)
* e-mail: stancil@athens.ath.epa.gov
* Or contact Wayne C. Huber at Oregon State Univ.
* 541/737-6150 or wayne.huber@orst.ath.
* Or Michael F. Schmidt at Camp Dresser &
* McKee (904) 281-0170 SCHMIDT@CFL.COM
*****

```

```

*****
* This is an implementation of EPA SWMM 4.4CU *
* "Nature is full of infinite causes which *
* have never occurred in experience" da Vinci *
*****

```

```

#####
# File names by SWMM Block #
# JIN -> Input to a Block #
# JOUT -> Output from a Block #
#####

```

```

JIN for Block # 1 File # 0 JIN.UF
JOUT for Block # 1 File # 9 PCTmpl.int

```

```

#####
# Scratch file names for this simulation. #
#####

```

```

NSCRAT # 1 File # 21 SCRT1.UF
NSCRAT # 2 File # 22 SCRT2.UF
NSCRAT # 3 File # 23 SCRT3.UF
NSCRAT # 4 File # 24 SCRT4.UF
NSCRAT # 5 File # 25 SCRT5.UF
NSCRAT # 6 File # 26 SCRT6.UF
NSCRAT # 7 File # 27 SCRT7.UF
NSCRAT # 8 File # 28 SCRT8.UF

```

```

*****
* Parameter Values on the Tapes Common Block *
*****

```

```

Number of Subcatchments in the Runoff Block (NR)..... 1000
Number of Channel/Pipes in the Runoff Block (NG)..... 1000
Number of Connections to Runoff Channels/Inlets (NCP). 6
Number of Water Quality Constituents (MQUAL)..... 20
Number of Runoff Land Uses per Subcatchment (NLJ)..... 20
Number of Groundwater Subcatchments in Runoff (IGW)... 100
Number of Interface Locations for all Blocks (NIE).... 1000
Number of Elements in the Transport Block (NET)..... 500
Number of Storage Junctions in Transport (NTSE)..... 100
Number of Transport interface input locations (NTHI).. 500
Number of Transport interface output locations (NTHO). 500
Number of Transport input locations on R lines (NTHR). 80

```


WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
***** AMRANDALE, VIRGINIA

ANALYSIS MODULE

Detroit River International Crossing
Wolfe/Cahill Drain Alternative 1B,2B Syphon - 100-Year Flow

Control information for simulation

Integration cycles..... 1440
Length of integration step is..... 5.00 seconds
Simulation length..... 2.00 hour

Do not create equiv. pipes (NEQUAL)..... 0

Use metric units for I/O..... 1

Printing starts in cycle..... 1

Intermediate printout intervals of..... 1 cycle
Intermediate printout intervals of..... 0.08 minutes

Summary printout intervals of..... 1 cycle
Summary printout time interval of..... 0.08 minutes

Hot start file parameter (JREDO)..... 0

Initial time (TZERO)..... 0.00 hour

This is time displacement from JIN interface file: starting date/time when interface file is used.

This also describes starting hour in K3 line hydrograph input when K3 lines are used.

Initial date (IDATZ)..... 19060714 (yr/mo/day)

NOTE: Initial date from JIN interface file will be used, if accessed, unless IDATZ is negative.

Iteration variables: JTMAX..... 30

SURTOL..... 0.0500

Default surface area of junctions.. 1.22 square meters.

EXTRAW VERSION 3.3 SOLUTION. (ISOL = 0).

Sum of junction flow is zero during surcharge.

NORMAL FLOW OPTION WHEN THE WATER SURFACE SLOPE IS LESS THAN THE GROUND SURFACE SLOPE (KSUPER=0).....

NJSW INPUT HYDROGRAPH JUNCTIONS..... 0

INTERMEDIATE HEADER LINES ARE PRINTED AS IN ORIGINAL PROGRAM

IDS ARE WRITTEN AS IN ORIGINAL PROGRAM

CONDUIT LENGTHS ON C1 LINE MUST EQUAL IRREGULAR SECTION LENGTH ENTERED ON THE C3 OR X1 LINES (IWLEN = 0)

JELEV = 0 (DEFAULT). STANDARD INPUTS ARE DEPTHS NOT ELEVATIONS

JDOWN = 0 - Minimum of normal or critical depth will be used at free outfalls (11).

Characteristic depth for M2 and S2 water surface profiles will be computed as in previous versions of EXTRAN (IM2 = 0).

SEDIMENT DEPTHS WILL NOT BE READ FROM C1 LINES

Intermediate continuity output will not be created

 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** ANNANDALE, VIRGINIA

Detroit River International Crossing
 Wolfe/Cahill Drain Alternative 1B,2B Syphon . 100-Year Flow

1 *****
 + Conduit Data *

INP NUM	CONDUIT NUMBER	LENGTH (M)	CONDUIT CLASS	AREA (SQ M)	MAINTENANCE COEF.	MAX WIDTH (M)	DEPTH (M)	JUNCTIONS AT THE ENDS	INVERT HEIGHT ABOVE JUNCTIONS	TRAPEZOID SIDE SLOPES
1	1	100.	TRAPEZOID	12.27	0.13500	0.91	2.26	1	2.00	2.00
2	2	100.	TRAPEZOID	12.27	0.13500	0.91	2.26	2	2.00	2.00
3	3	100.	TRAPEZOID	12.27	0.13500	0.91	2.26	3	2.00	2.00
4	4	70.	TRAPEZOID	12.27	0.13500	0.91	2.26	4	2.00	2.00
5	5	30.	TRAPEZOID	11.25	0.11500	4.50	1.50	5	2.00	2.00

6	55	15. RECTANGLE	6.75	0.01500	4.50	1.50	55	66
7	66	50. RECTANGLE	6.75	0.01500	4.50	1.50	66	6
8	6	15. RECTANGLE	6.75	0.01500	4.50	1.50	6	77
9	77	30. TRAPEZOID	11.25	0.01500	4.50	1.50	77	7
10	7	70. TRAPEZOID	12.27	0.01500	0.91	2.26	7	8
11	8	100. TRAPEZOID	12.27	0.03500	0.91	2.26	8	9
12	9	100. TRAPEZOID	12.27	0.03500	0.91	2.26	9	10
13	10	100. TRAPEZOID	12.27	0.03500	0.91	2.26	10	11

====> WARNING !!! (C*DELT/LEN) IN CONDUIT 55 IS 1.3 AT FULL DEPTH.
 ====> WARNING !!! (C*DELT/LEN) IN CONDUIT 6 IS 1.3 AT FULL DEPTH.

 * Conduit Volume *

Input full depth volume..... 1.0296E+01 cubic meters
 Conduit #... 66 has zero slope. 0.001 feet added to upstream invert.

====> Warning !! The upstream and downstream junctions for the following conduits
 have been reversed to correspond to the positive flow and decreasing
 slope EXTRAN convention. A negative flow in the output thus means
 the flow was from your original upstream junction to your original
 downstream junction. Any initial flow has been multiplied by -1.

1. Conduit #... 6 has been changed.

1-----
 ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
 WASHINGTON, D.C. ***** ANALYSIS MODULE ***** CAMP DRESSER & MCKEE INC.
 ANNANDALE, VIRGINIA

Detroit River International Crossing
 Wolfe/Cahill Drain Alternative 1B,2B Syphon - 100-Year Flow

1

 * Junction Data *

INP JUNCTION GROUND CROWN INVERT INVERT INITIAL CONNECTING CONDUITS
 NUM NUMBER ELEV. ELEV. ELEV. CHS DEPTH(M)

1	183.00	181.70	179.44	2.10	0.00	1
2	183.00	181.46	179.20	0.00	0.00	1
3	183.00	181.22	178.96	0.00	0.00	2
4	183.00	180.98	178.72	0.00	0.00	3
5	183.00	180.82	178.56	0.00	0.00	4
6	183.00	179.98	178.48	0.00	0.00	5
7	183.00	173.94	172.44	0.00	0.00	55
8	183.00	173.94	172.44	0.00	0.00	66
9	183.00	179.96	178.46	0.00	0.00	6
10	183.00	180.64	178.38	0.00	0.00	77
11	183.00	180.48	178.22	0.00	0.00	7
12	183.00	180.24	177.98	0.00	0.00	8
13	183.00	180.00	177.74	0.00	0.00	9
14	183.00	179.76	177.50	0.00	0.00	10

```

*****
+ FREE OUTFALL DATA (DATA GROUP I1) *
+ BOUNDARY CONDITION ON DATA GROUP J1 *
*****
    
```

OUTFALL AT JUNCTION... 11 HAS BOUNDARY CONDITION NUMBER... 1

```

1-----
ENVIRONMENTAL PROTECTION AGENCY ***** EXTENDED TRANSPORT PROGRAM ***** WATER RESOURCES DIVISION
WASHINGTON, D.C. ***** CAMP DRESSER & MCKEE INC.
***** ANNANDALE, VIRGINIA
    
```

Detroit River International Crossing
 Wolfe/Cahill Drain Alternative 1B,2B Syphon 100-Year Flow

```

*****
+ INTERNAL CONNECTIVITY INFORMATION *
*****
    
```

```

CONDUIT JUNCTION JUNCTION
-----
90014 11 0
    
```

```

*****
+ BOUNDARY CONDITION INFORMATION *
+ DATA GROUPS J1-J4 *
*****
    
```

BC NUMBER.. 1 HAS NO CONTROL WATER SURFACE.
 TZERO = 1906195 0.0000000E+00

 * INITIAL MODEL CONDITION *
 * INITIAL TIME = 0.00 HOURS *

JUNCTION / DEPTH / ELEVATION	==> "==" JUNCTION IS SURCHARGED.		
1/ 0.00 / 179.44	2/ 0.00 / 179.20	3/ 0.00 / 178.96	
4/ 0.00 / 178.72	5/ 0.00 / 178.56	55/ 0.00 / 178.48	
66/ 0.00 / 172.44	6/ 0.00 / 172.44	77/ 0.00 / 178.46	
7/ 0.00 / 178.38	8/ 0.00 / 178.22	9/ 0.00 / 177.98	
10/ 0.00 / 177.74	11/ 0.00 / 177.50		

CONDUIT/ FLOW	==> "==" CONDUIT USES TH; NORMAL FLOW OPTION.		
1/ 0.00	2/ 0.00	3/ 0.00	4/ 0.00
5/ 0.00	55/ 0.00	66/ 0.00	6/ 0.00
77/ 0.00	7/ 0.00	8/ 0.00	9/ 0.00
10/ 0.00	90014/ 0.00		

CONDUIT/ VELOCITY			
1/ 0.00	2/ 0.00	3/ 0.00	4/ 0.00
5/ 0.00	55/ 0.00	66/ 0.00	6/ 0.00
77/ 0.00	7/ 0.00	8/ 0.00	9/ 0.00
10/ 0.00			

CONDUIT/ CROSS SECTIONAL AREA			
1/ 0.00	2/ 0.00	3/ 0.00	4/ 0.00
5/ 0.00	55/ 0.00	66/ 0.00	6/ 0.00
77/ 0.00	7/ 0.00	8/ 0.00	9/ 0.00
10/ 0.00			

CONDUIT/ HYDRAULIC RADIUS			
1/ 0.00	2/ 0.00	3/ 0.00	4/ 0.00
5/ 0.00	55/ 0.00	66/ 0.00	6/ 0.00
77/ 0.00	7/ 0.00	8/ 0.00	9/ 0.00
10/ 0.00			

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION			
1/ 179.44/ 179.20	2/ 179.2 / 178.96	3/ 178.96/ 178.72	
4/ 178.72/ 178.56	5/ 178.5 / 178.48	55/ 178.48/ 172.44	

66/	172.44/	172.44	6/	178.44/	172.44	77/	178.46/	178.38
7/	178.38/	178.22	8/	178.22/	177.98	9/	177.98/	177.74
10/	177.74/	177.50						

 * FINAL MODEL CONDITION *
 * FINAL TIME = 2.00 HOURS *

>>> ENDING DATE AND TIME OF EXTRAN RUN ARE:

JULIAN DATE: 1906195

YR/MO/DA: 1906/ 7/14

TIME OF DAY: 2.000 HRS

JUNCTION / DEPTH / ELEVATION ==> "==" JUNCT ON IS SURCHARGED.

1/	1.89 /	181.33	2/	1.88 /	181.08	3/	1.86 /	180.82
4/	1.81 /	180.53	5/	1.70 /	180.26	55/	1.77*/	180.25
66/	7.79*/	180.23	6/	7.71*/	180.15	77/	1.66*/	180.12
7/	1.74 /	180.12	8/	1.89 /	180.11	9/	1.89 /	179.87
10/	1.87 /	179.61	11/	1.29 /	178.79			

CONDUIT/ FLOW ==> "==" CONDUIT USES TH : NORMAL FLOW OPTION.

1/	12.10	2/	12.10	3/	12.10	4/	12.10
5/	12.10	55/	12.10	66/	12.10	6/	-12.10
77/	12.10	7/	12.10	8/	12.10	9/	12.10
10/	12.10	90014/	12.10				

CONDUIT/ VELOCITY

1/	1.37	2/	1.39	3/	1.44	4/	1.56
5/	1.08	55/	1.79	66/	1.79	6/	-1.79
77/	1.08	7/	1.47	8/	1.37	9/	1.38
10/	1.89						

CONDUIT/ CROSS SECTIONAL AREA

1/	8.84	2/	8.72	3/	8.43	4/	7.75
5/	11.25	55/	6.75	66/	6.75	6/	6.75
77/	11.25	7/	8.24	8/	8.85	9/	8.75
10/	6.41						

CONDUIT/ FINAL VOLUME

1/	884.47	2/	872.49	3/	842.70	4/	542.76
5/	337.50	55/	101.25	66/	337.50	6/	101.25
77/	337.50	7/	576.73	8/	885.41	9/	874.71
10/	641.08						

CONDUIT/	HYDRAULIC RADIUS	2/	0.94	3/	0.92	4/	0.89
1/	0.95						
5/	1.00	55/	0.56	66/	0.56	6/	0.56
77/	1.00	7/	0.91	8/	0.95	9/	0.94
10/	0.80						

CONDUIT/ UPSTREAM/ DOWNSTREAM ELEVATION

1/	181.33/	181.08	2/	181.03/	180.82	3/	180.82/	180.53
4/	180.53/	180.26	5/	180.23/	180.25	55/	180.25/	180.23
66/	180.23/	180.15	6/	180.11/	180.15	77/	180.12/	180.12
7/	180.12/	180.11	8/	180.11/	179.87	9/	179.87/	179.61
10/	179.61/	178.79						

 # Surcharge Iteration Summary #
 #####

Maximum number of iterations in a time step..... 16
 Total number of iterations in the simulation.. 3063
 Average number of iterations per time step..... 2.13
 Surcharge iterations during the simulation..... 183
 Maximum surcharge flow error during simulation.. 5.47E-01 cms
 Total number of time steps during simulation.. 1440

1 *****
 * CONDUIT COURANT CONDITION SUMMARY *
 * TIME IN MINUTES DELT > COURANT TIME STEP *

 * SEE BELOW FOR EXPLANATION OF COURANT TIME STEP *

CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)	CONDUIT #	TIME(MN)
1	0.00	2	0.00	3	0.00	4	0.00
5	0.25	55	117.42	66	114.08	6	115.50
77	0.08	7	0.00	8	0.00	9	0.00
10	0.00						

1 *****

```

* CONDUIT COURANT CONDITION SUMMARY *
*****
* COURANT = CONDUIT LENGTH *
* TIME STEP = ----- *
* VELOCITY + SQRT(GRAV*AREA/WIDTH) *
*****
* AVERAGE COURANT CONDITION TIME STEP (SECONDS) *
*****

```

CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)	CONDUIT #	TIME(SEC)
1	21.83	2	21.91	3	23.28	4	20.53
5	9.42	55	1.90	66	5.51	6	1.57
77	6.90	7	14.89	8	22.23	9	24.30
10	34.41						

```

1
*****
* EXTRAN CONTINUITY BALANCE AT THE LAST TIME STEP *
*****

```

```

*****
* JUNCTION INFLOW, OUTFLOW OR STREET FLOODING *
*****

```

JUNCTION	INFLOW, CU M
1	8.7120E+04

JUNCTION	OUTFLOW, CU M
1	3.5265E+01
11	7.9960E+04

```

*****
* INITIAL SYSTEM VOLUME = 8.8000E-03 CU M *
* TOTAL SYSTEM INFLOW VOLUME = 8.7120E+04 CU M *
* INFLOW + INITIAL VOLUME = 8.7120E+04 CU M *
*****
* TOTAL SYSTEM OUTFLOW = 7.9995E+04 CU M *
* VOLUME LEFT IN SYSTEM = 7.3537E+03 CU M *
* OUTFLOW + FINAL VOLUME = 8.7348E+04 CU M *
*****
* ERROR IN CONTINUITY, PERCENT = -0.26 *
*****

```

TEST WRITE OF ALTERNATIVE CONTINUITY ERROR CALCULATION
 VOLUME LEFT IN SYSTEM = 7.7720E+03 CU. FT.
 ERROR IN CONTINUITY PERCENT = -7.42

SUMMARY OF FULL FLOW CHANNEL WARNINGS

OPEN CHANNEL NUMBER	TIME STEP OF FIRST OCCURRENCE (HOURS)	TIME OF FIRST OCCURRENCE (HOURS)	TIME STEP OF LAST OCCURRENCE	TIME OF LAST OCCURRENCE (HOURS)
1	1	0.00	4	0.01
5	68	0.09	1440	2.00
77	97	0.13	1440	2.00

THE PROGRAM USES FULL DEPTH CHANNEL CHARACTERISTICS TO COMPUTE FLOW THROUGH THE TRAPEZOIDAL, IRREGULAR, OR PARABOLIC/POWER FUNCTION CONDUIT WHEN THE COMPUTED DEPTHS EXCEED MAXIMUM DEPTH. THIS WILL AFFECT THE MAXIMUM COMPUTED HEAD AND FLOWS IN THE MODEL. IT IS HIGHLY RECOMMENDED THAT THE MODELED CROSS SECTIONS BE EXTENDED TO ELIMINATE THESE FULL FLOW CHANNEL WARNINGS

1

JUNCTION SUMMARY STATISTICS

Detroit River International Crossing
 Wolfe/Cahill Drain Alternative 1B,2B Sphphon - 100-Year Flow

JUNCTION NUMBER	GROUND ELEVATION (M)	UPPERMOST PIPE CROWN ELEVATION (M)	MEAN JUNCTION ELEVATION (M)	MAXIMUM JUNCTION AVERAGE ELEV. (M)	TIME OF OCCURRENCE HR. MIN.	METERS OF SURCHARGE AT MAX ELEVATION	METERS MAX. DEPTH IS BELOW GROUND ELEVATION	LENGTH OF SURCHARGE (MIN)	LENGTH OF FLOODING (MIN)	MAXIMUM JUNCTION AREA (SQ.MET)
1	183.00	181.70	181.33	0.179	0	1.30	0.00	0.3	0.2	2.438E+03
2	183.00	181.46	181.07	0.065	0	0.00	1.92	0.0	0.0	1.704E+03
3	183.00	181.22	180.80	0.065	1	0.00	2.18	0.0	0.0	8.361E+02
4	183.00	180.98	180.49	0.073	0	0.00	2.44	0.0	0.0	6.932E+02
5	183.00	180.82	180.18	0.156	0	0.00	2.74	0.0	0.0	1.821E+03
55	183.00	179.98	180.16	0.390	0	0.59	2.43	111.8	0.0	1.913E+03
66	183.00	173.94	179.94	1.466	0	6.87	2.18	115.8	0.0	2.341E+03
6	183.00	173.94	179.86	0.808	1	6.21	2.85	115.9	0.0	2.881E+03
77	183.00	179.96	180.04	0.320	0	0.16	2.88	110.6	0.0	1.591E+03

CONDUIT NUMBER	DESIGN FLOW (CMS)	DESIGN VELOCITY (M/S)	CONDUIT VERTICAL DEPTH (M)	CONDUIT MAXIMUM FLOW COMPUTED (CMS)	TIME OF OCCURRENCE (HR. MIN.)	MAXIMUM COMPUTED VELOCITY (MPS)	TIME OF OCCURRENCE (HR. MIN.)	RATIO OF MAX. TO DESIGN FLOW	MAXIMUM UPSTREAM INV. AT CONDUIT ENDS (M)	DEPTH ABOVE DOWNSTREAM (M)	LENGTH OF CONDUIT (MIN)	NORM. SLOPE (M/M)
7	183.00	180.64	180.03	0.130	180.12	1	56	0.00	2.88	0.0	0.0	1.192E+03
8	183.00	180.48	180.01	0.067	180.11	1	36	0.00	2.89	0.0	0.0	7.163E+02
9	183.00	180.24	179.75	0.063	179.87	1	50	0.00	3.13	0.0	0.0	8.457E+02
10	183.00	180.00	179.47	0.059	179.61	1	49	0.00	3.39	0.0	0.0	8.110E+02
11	183.00	179.76	178.69	0.041	178.79	1	21	0.00	4.21	0.0	0.0	6.651E+02

1

 * CONDUIT SUMMARY STATISTICS *

Detroit River International Crossing
 Wolfe/Cahill Drain Alternative 1B,2B Syphon - 100-Year Flow

CONDUIT NUMBER	DESIGN FLOW (CMS)	DESIGN VELOCITY (M/S)	CONDUIT VERTICAL DEPTH (M)	CONDUIT MAXIMUM FLOW COMPUTED (CMS)	TIME OF OCCURRENCE (HR. MIN.)	MAXIMUM COMPUTED VELOCITY (MPS)	TIME OF OCCURRENCE (HR. MIN.)	RATIO OF MAX. TO DESIGN FLOW	MAXIMUM UPSTREAM INV. AT CONDUIT ENDS (M)	DEPTH ABOVE DOWNSTREAM (M)	LENGTH OF CONDUIT (MIN)	NORM. SLOPE (M/M)
1	1.85E+01	1.50	2.260	1.64E+01	0	0	0	0.89	3.56	1.88	0.2	0.00240
2	1.85E+01	1.50	2.260	1.28E+01	0	2	1	0.69	1.88	1.86	0.3	0.00240
3	1.85E+01	1.50	2.260	1.25E+01	0	6	2	0.68	1.86	1.84	0.3	0.00240
4	1.80E+01	1.47	2.260	1.63E+01	0	5	4	0.90	1.84	1.70	0.0	0.00229
5	3.88E+01	3.45	1.500	2.80E+01	0	4	4	0.72	1.70	2.09	0.3	0.00267
55	1.95E+02	28.83	1.500	2.65E+01	0	4	4	0.14	2.09	8.37	1.8	0.40267
66	1.38E+00	0.20	1.500	2.65E+01	0	4	4	19.23	8.37	7.71	0.1	0.00002
6	1.94E+02	28.78	1.500	-2.63E+01	0	4	4	-0.14	1.66	7.71	0.0	0.40133
77	3.88E+01	3.45	1.500	2.87E+01	0	4	4	0.74	1.66	1.74	0.2	0.00267
7	4.20E+01	3.42	2.260	1.45E+01	0	6	6	0.34	1.74	1.89	0.8	0.00229
8	1.85E+01	1.50	2.260	1.21E+01	1	16	16	0.66	1.89	1.89	0.1	0.00240
9	1.85E+01	1.50	2.260	1.21E+01	1	12	12	0.66	1.89	1.87	0.0	0.00240
10	1.85E+01	1.50	2.260	1.21E+01	1	21	21	0.66	1.87	1.29	0.0	0.00240
90014	UNDEF	UNDEF	UNDEF	1.21E+01	1	21	21	0.66	1.87	1.29	0.0	0.00240

1

 * SUBCRITICAL AND CRITICAL FLOW ASSUMPTIONS FROM *
 * SUBROUTINE HEAD. SEE FIGURE 5-4 IN THE EXPLAN *
 * MANUAL FOR FURTHER INFORMATION *

CONDUIT NUMBER	DESIGN FLOW (CMS)	DESIGN VELOCITY (M/S)	CONDUIT VERTICAL DEPTH (M)	CONDUIT MAXIMUM FLOW COMPUTED (CMS)	TIME OF OCCURRENCE (HR. MIN.)	MAXIMUM COMPUTED VELOCITY (MPS)	TIME OF OCCURRENCE (HR. MIN.)	RATIO OF MAX. TO DESIGN FLOW	MAXIMUM UPSTREAM INV. AT CONDUIT ENDS (M)	DEPTH ABOVE DOWNSTREAM (M)	LENGTH OF CONDUIT (MIN)	NORM. SLOPE (M/M)
7	183.00	180.64	180.03	0.130	180.12	1	56	0.00	2.88	0.0	0.0	1.192E+03
8	183.00	180.48	180.01	0.067	180.11	1	36	0.00	2.89	0.0	0.0	7.163E+02
9	183.00	180.24	179.75	0.063	179.87	1	50	0.00	3.13	0.0	0.0	8.457E+02
10	183.00	180.00	179.47	0.059	179.61	1	49	0.00	3.39	0.0	0.0	8.110E+02
11	183.00	179.76	178.69	0.041	178.79	1	21	0.00	4.21	0.0	0.0	6.651E+02

CONDUIT NUMBER	DRY FLOW (MIN)	SUBCRITICAL FLOW (MIN)	CRITICAL FLOW (MIN)	CRITICAL FLOW (MIN)	FLOW (CMS)	AVERAGE % CHANGE	FLOW CUBIC MET RADIUS (MET)	HYDRAULIC RADIUS (MET)	CROSS SECT AREA (SQ.M)
1	0.00	120.00	0.00	0.00	12.11	0.1153	8.7190E+04	0.9458	8.8448
2	0.08	119.92	0.00	0.00	11.98	0.0695	8.6280E+04	0.9393	8.7250
3	0.08	119.92	0.00	0.00	11.86	0.0956	8.5401E+04	0.9231	8.4271
4	0.33	119.67	0.00	0.00	11.76	0.1699	8.4692E+04	0.8855	7.7539
5	0.83	119.17	0.00	0.00	11.69	0.3079	8.4199E+04	1.0037	11.2500
55	1.58	118.42	0.00	0.00	11.63	0.0295	8.3708E+04	0.8363	6.7500
66	2.08	117.92	0.00	0.00	11.61	7.0886	8.3579E+04	0.8108	6.7500
6	4.33	115.67	0.00	0.00	-11.55	0.0321	-8.3148E+04	0.8421	6.7500
77	4.33	115.67	0.00	0.00	11.52	0.2510	8.2934E+04	1.0037	11.2500
7	4.42	115.58	0.00	0.00	11.45	0.0637	8.2458E+04	0.9128	8.2392
8	4.50	115.50	0.00	0.00	11.35	0.0761	8.1731E+04	0.9463	8.8543
9	4.67	115.33	0.00	0.00	11.23	0.0474	8.0845E+04	0.9405	8.7473
10	5.00	115.00	0.00	0.00	11.11	0.0459	7.9959E+04	0.8050	6.4109
90014	UNDEFINED	UNDEFINED	UNDEFINED	UNDEFINED	11.11		7.9959E+04		

```

*****
* AVERAGE % CHANGE IN JUNCTION OR CONDUIT IS DEFINED AS:
* CONDUIT % CHANGE ==> 100.0 ( Q(n+1) - Q(n) ) / Qfull
* JUNCTION % CHANGE ==> 100.0 ( Y(n+1) - Y(n) ) / Yfull
*****

```

```

The Conduit with the largest average change... 66 had 7.089 percent
The Junction with the largest average change... 66 had 1.466 percent

```

```

==> Extended Transport model simulation ended normally.
==> SWMM 4.4GU simulation ended normally.
Always check output file for possible warning messages.

```

```

==> Your input file was named : PCTmpl.dat
==> Your output file was named: PCTmpl.out

```

```

*****
* SWMM 4.4GU Simulation Date and Time Summary
*****
* Starting Date... November 17, 2006
* Time... 9:34:42. 0
* Ending Date... November 17, 2006
* Time... 9:34:46.830
* Elapsed Time... 0.081 minutes.
* Elapsed Time... 4.831 seconds.

```

Appendix A.3

Alternative 2A

Cahill Drain Crossing
Lennon Drain Crossing
Basin Drain Crossing
Titcombe Drain Crossing

Culvert Calculator Report

Basin Drain -All Alternatives - Reg Check

Solve For: Headwater Elevation

Culvert Summary

Allowable HW Elevation	181.40 m	Headwater Depth/Height	1.20
Computed Headwater Elevat	180.32 m	Discharge	8.1000 m ³ /s
Inlet Control HW Elev.	180.32 m	Tailwater Elevation	179.70 m
Outlet Control HW Elev.	180.32 m	Control Type	Inlet Control

Grades

Upstream Invert	178.50 m	Downstream Invert	178.20 m
Length	58.00 m	Constructed Slope	0.005172 m/m

Hydraulic Profile

Profile	CompositeS1S2	Depth, Downstream	1.05 m
Slope Type	Steep	Normal Depth	1.05 m
Flow Regime	N/A	Critical Depth	1.14 m
Velocity Downstream	3.62 m/s	Critical Slope	0.004178 m/m

Section

Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.13 m
Section Size	2130 x 1520 mm	Rise	1.52 m
Number Sections	1		

Outlet Control Properties

Outlet Control HW Elev.	180.32 m	Upstream Velocity Head	0.57 m
Ke	0.20	Entrance Loss	0.11 m

Inlet Control Properties

Inlet Control HW Elev.	180.32 m	Flow Control	Transition
Inlet Type	90° headwall w 45° bevels	Area Full	3.3 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Culvert Calculator Report

Lennon Drain - Alt2A-100yr-Existing

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	182.00 m	Headwater Depth/Height	1.54
Computed Headwater Elevat	181.38 m	Discharge	8.3000 m ³ /s
Inlet Control HW Elev.	181.21 m	Tailwater Elevation	180.70 m
Outlet Control HW Elev.	181.38 m	Control Type	Outlet Control

Grades			
Upstream Invert	179.50 m	Downstream Invert	179.20 m
Length	67.00 m	Constructed Slope	0.004478 m/m

Hydraulic Profile			
Profile	PressureProfile	Depth, Downstream	1.50 m
Slope Type	N/A	Normal Depth	0.93 m
Flow Regime	N/A	Critical Depth	1.02 m
Velocity Downstream	2.63 m/s	Critical Slope	0.003567 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.59 m
Section Size	1219 mm x 2591 mm	Rise	1.22 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	181.38 m	Upstream Velocity Head	0.35 m
Ke	0.20	Entrance Loss	0.07 m

Inlet Control Properties			
Inlet Control HW Elev.	181.21 m	Flow Control	Submerged
Inlet Type	90° headwall w 45° bevels	Area Full	3.2 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Culvert Calculator Report Cahill Alt 2A-Future

Comments: Unknown Flow

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	184.40 m	Headwater Depth/Height	1.97
Computed Headwater Elevat	182.77 m	Discharge	27.6000 m ³ /s
Inlet Control HW Elev.	182.77 m	Tailwater Elevation	181.14 m
Outlet Control HW Elev.	182.61 m	Control Type	Inlet Control
Grades			
Upstream Invert	179.82 m	Downstream Invert	179.64 m
Length	74.00 m	Constructed Slope	0.002432 m/m
Hydraulic Profile			
Profile	PressureProfile	Depth, Downstream	1.50 m
Slope Type	N/A	Normal Depth	N/A m
Flow Regime	N/A	Critical Depth	1.50 m
Velocity Downstream	4.09 m/s	Critical Slope	0.006085 m/m
Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	4.50 m
Section Size	4500 x 1500 mm	Rise	1.50 m
Number Sections	1		
Outlet Control Properties			
Outlet Control HW Elev.	182.61 m	Upstream Velocity Head	0.85 m
Ke	0.20	Entrance Loss	0.17 m
Inlet Control Properties			
Inlet Control HW Elev.	182.77 m	Flow Control	N/A
Inlet Type	90° headwall w 45° bevels	Area Full	6.8 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Appendix A.4

Alternative 2B

Appendix A.4.1

Titcombe Drain Crossing

Titcombe Drain

Worksheet for Circular Channel

Project Description	
Worksheet	Titcome_prelimin
Flow Element	Circular Channel
Method	Manning's Formu
Solve For	Full Flow Diametr

Input Data	
Mannings Coeffici	0.013
Channel Slope	005000 m/m
Discharge	3.2000 m ³ /s

Results	
Depth	1.27 m
Diameter	1,269.0 mm
Flow Area	1.3 m ²
Wetted Perimet	4.30 m
Top Width	0.00 m
Critical Depth	0.97 m
Percent Full	100.0 %
Critical Slope	005735 m/m
Velocity	2.53 m/s
Velocity Head	0.33 m
Specific Energy	1.60 m
Froude Number	0.00
Maximum Disch	3.4423 m ³ /s
Discharge Full	3.2000 m ³ /s
Slope Full	005000 m/m
Flow Type	N/A

Notes: Discharge of 3.2 m³/s was taken from taking 50% of the 100 year flow of subcatchment 140 of turkey creek watershed.
 Drainage area D/S of Titcombe is approximately 274 ha, 55% of the entire subcatch #140. Drainage area therefore U/S Titcombe crossing is app.
 50% of the entire subcatch.
 From 1989 Maclaren report:
 Catch # 140
 DA = 496 Ha.
 100 yr existing = 6.4 m³/s
 100 yr efuture = 16.7 m³/s

Appendix A.4.2

Basin Drain Crossing

Culvert Calculator Report

Basin Drain -All Alternatives - Reg Check

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	181.40 m	Headwater Depth/Height	1.20
Computed Headwater Elevat	180.32 m	Discharge	8.1000 m ³ /s
Inlet Control HW Elev.	180.32 m	Tailwater Elevation	179.70 m
Outlet Control HW Elev.	180.32 m	Control Type	Inlet Control

Grades			
Upstream Invert	178.50 m	Downstream Invert	178.20 m
Length	58.00 m	Constructed Slope	0.005172 m/m

Hydraulic Profile			
Profile	CompositeS1S2	Depth, Downstream	1.05 m
Slope Type	Steep	Normal Depth	1.05 m
Flow Regime	N/A	Critical Depth	1.14 m
Velocity Downstream	3.62 m/s	Critical Slope	0.004178 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.13 m
Section Size	2130 x 1520 mm	Rise	1.52 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	180.32 m	Upstream Velocity Head	0.57 m
Ke	0.20	Entrance Loss	0.11 m

Inlet Control Properties			
Inlet Control HW Elev.	180.32 m	Flow Control	Transition
Inlet Type	90° headwall w 45° bevels	Area Full	3.3 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Appendix A.5

Alternative 2B – Revised

Appendix A.5.1

Titcombe Drain Crossing

Titcombe Drain

Worksheet for Circular Channel

Project Description	
Worksheet	Titcome_prelimin
Flow Element	Circular Channel
Method	Manning's Formu
Soive For	Full Flow Diametr

Input Data	
Mannings Coeffici	0.013
Channel Slope	005000 m/m
Discharge	3.2000 m ³ /s

Results	
Depth	1.27 m
Diameter	1,269.0 mm
Flow Area	1.3 m ²
Wetted Perimet	4.30 m
Top Width	0.00 m
Critical Depth	0.97 m
Percent Full	100.0 %
Critical Slope	005735 m/m
Velocity	2.53 m/s
Velocity Head	0.33 m
Specific Energy	1.60 m
Froude Number	0.00
Maximum Discr	3.4423 m ³ /s
Discharge Full	3.2000 m ³ /s
Slope Full	005000 m/m
Flow Type	N/A

Notes: Discharge of 3.2 m³/s was taken from taking 50% of the 100 year flow of subcatchment 140 of turkey creek watershed.
 Drainage area D/S of Titcombe is approximately 274 ha, 55% of the entire subcatch #140. Drainage area therefore U/S Titcombe crossing is app.
 50% of the entire subcatch.
 From 1989 Maclaren report:
 Catch # 140
 DA = 496 Ha.
 100 yr existing = 6.4 m³/s
 100 yr efuture = 16.7 m³/s

Appendix A.5.2

Basin Drain Crossing

Culvert Calculator Report

Basin Drain -All Alternatives - Reg Check

Solve For: Headwater Elevation

Culvert Summary			
Allowable HW Elevation	181.40 m	Headwater Depth/Height	1.20
Computed Headwater Elevat	180.32 m	Discharge	8.1000 m ³ /s
Inlet Control HW Elev.	180.32 m	Tailwater Elevation	179.70 m
Outlet Control HW Elev.	180.32 m	Control Type	Inlet Control

Grades			
Upstream Invert	178.50 m	Downstream Invert	178.20 m
Length	58.00 m	Constructed Slope	0.005172 m/m

Hydraulic Profile			
Profile	CompositeS1S2	Depth, Downstream	1.05 m
Slope Type	Steep	Normal Depth	1.05 m
Flow Regime	N/A	Critical Depth	1.14 m
Velocity Downstream	3.62 m/s	Critical Slope	0.004178 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.13 m
Section Size	2130 x 1520 mm	Rise	1.52 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	180.32 m	Upstream Velocity Head	0.57 m
Ke	0.20	Entrance Loss	0.11 m

Inlet Control Properties			
Inlet Control HW Elev.	180.32 m	Flow Control	Transition
Inlet Type	90° headwall w 45° bevels	Area Full	3.3 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Appendix A.5.3

Turkey Creek Hydraulic Analysis



LEGEND

- WATERCOURSE
- CROSS SECTION
- GROUND CONTOUR
- HEC RAS SECTION ID

APPENDIX A
 NOVEMBER 2008
 ALTERNATIVE 2B PRELIMINARY PROFILE - BELOW GRADE
 HEC RAS CROSS SECTION LOCATIONS
 SCALE: 1:50
 Ontario MINDOT
 URS

GRAND MARAIS DR

TURNKEY CREEK

183.00

178.50

181.50

182.00

183.00

178.50

181.50

182.00

9.1

9.2

9.3

9.4

9.5

9.6

9.7

9.8

9.9

10.0

14+000

14+500

R=1

Appendix A.5.3.1

Existing Condition

dric.rep

HEC-RAS Version 3.1.3 May 2005
U.S. Army Corp of Engineers
Hydrologic Engineering Center
609 Second Street
Davis, California

```

X   X  XXXXXX   XXXX   XXXX   XX   XXXX
X   X  X       X   X   X   X   X   X
X   X  X       X       X   X   X   X   X
XXXXXXXX XXXX   X       XXX XXXX XXXXXXX XXXX
X   X  X       X       X   X   X   X   X
X   X  X       X   X   X   X   X   X   X
X   X  XXXXXX   XXXX   X   X   X   X   XXXXX

```

PROJECT DATA

Project Title: Turkey River
Project File : dric.prj
Run Date and Time: 15/11/2006 2:56:15 PM

Project in SI units

PLAN DATA

Plan Title: Plan 37
Plan File : o:\DRIC\19_waterResources\hec\dric.p37

Geometry Title: existing
Geometry File : o:\DRIC\19_waterResources\hec\dric.g01

Flow Title : Flow 01
Flow File : o:\DRIC\19_waterResources\hec\dric.f01

Plan Summary Information:

Number of:	Cross Sections = 10	Multiple Openings = 0
	Culverts = 0	Inline Structures = 0
	Bridges = 1	Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.003
Critical depth calculation tolerance = 0.003
Maximum number of iterations = 20
Maximum difference tolerance = 0.1
Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary
Conveyance Calculation Method: At breaks in n values only
Friction Slope Method: Average Conveyance
Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: Flow 01
Flow File : o:\DRIC\19_waterResources\hec\dric.f01

Flow Data (m3/s)

```

*****
* River      Reach      RS      *      100 yr      Regional *
* Turkey Creek Main      10      *      39.5      62.6 *
*****

```

Boundary Conditions

```

*****
* River      Reach      Profile      *      Upstream      Downstream
*
*****
* Turkey Creek Main      100 yr      *      Normal S = 0.000375
*

```

GEOMETRY DATA

Geometry Title: existing
 Geometry File : o:\DRIC\19_waterResources\hec\dric.g01

CROSS SECTION

RIVER: Turkey Creek
 REACH: Main RS: 10

INPUT

Description:

Station Elevation Data		num= 30		Sta Elev		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	183	76.736	183	99.766	182.5	101.278	182	102.644	181.5		
104.01	181	105.363	180.5	106.485	180	107.607	179.5	108.729	179		
109.851	178.5	110.973	178	112.095	177.5	113.217	177	116.947	176.56		
117.547	176.36	119.547	176.36	120.147	176.56	123.877	177	124.941	177.5		
126.005	178	127.069	178.5	128.133	179	129.197	179.5	130.261	180		
131.624	180.5	132.798	181	134.684	181.5	136.554	182	264.86	182.5		

Manning's n Values		num= 3		Sta n Val		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.03	116.947	.017	120.147	.03				

Bank	Sta: Left	Right	Lengths: Left	Channel	Right	Coeff Contr.	Expan.
	116.947	120.147	207.817	210.778	216.406	.1	.3

CROSS SECTION OUTPUT Profile #100 yr

* E.G. Elev (m)	* 178.68	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.27	* Wt. n-Val.	* 0.030	* 0.017	* 0.030
* W.S. Elev (m)	* 178.40	* Reach Len. (m)	* 207.82	* 210.78	* 216.41
* Crit W.S. (m)	*	* Flow Area (m2)	* 8.25	* 6.41	* 8.13
* E.G. Slope (m/m)	* 0.001101	* Area (m2)	* 8.25	* 6.41	* 8.13
* Q Total (m3/s)	* 39.50	* Flow (m3/s)	* 9.99	* 19.62	* 9.89
* Top Width (m)	* 16.78	* Top width (m)	* 6.87	* 3.20	* 6.71
* Vel Total (m/s)	* 1.73	* Avg. Vel. (m/s)	* 1.21	* 3.06	* 1.22
* Max Chl Dpth (m)	* 2.04	* Hydr. Depth (m)	* 1.20	* 2.00	* 1.21
* Conv. Total (m3/s)	* 1190.6	* Conv. (m3/s)	* 301.0	* 591.3	* 298.2
* Length wtd. (m)	* 211.47	* Wetted Per. (m)	* 7.20	* 3.26	* 7.05
* Min Ch El (m)	* 176.36	* Shear (N/m2)	* 12.37	* 21.19	* 12.45
* Alpha	* 1.80	* Stream Power (N/m s)	* 14.98	* 64.86	* 15.15
* Frctn Loss (m)	* 0.16	* Cum Volume (1000 m3)	* 3.89	* 5.02	* 4.35
* C & E Loss (m)	* 0.03	* Cum SA (1000 m2)	* 3.03	* 2.34	* 3.46

CROSS SECTION OUTPUT Profile #Regional

* E.G. Elev (m)	* 179.26	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.36	* Wt. n-Val.	* 0.030	* 0.017	* 0.030
* W.S. Elev (m)	* 178.90	* Reach Len. (m)	* 207.82	* 210.78	* 216.41
* Crit W.S. (m)	*	* Flow Area (m2)	* 11.93	* 8.00	* 11.72
* E.G. Slope (m/m)	* 0.001143	* Area (m2)	* 11.93	* 8.00	* 11.72
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 16.96	* 28.90	* 16.74
* Top Width (m)	* 18.95	* Top width (m)	* 7.99	* 3.20	* 7.77
* Vel Total (m/s)	* 1.98	* Avg. Vel. (m/s)	* 1.42	* 3.61	* 1.43
* Max Chl Dpth (m)	* 2.54	* Hydr. Depth (m)	* 1.49	* 2.50	* 1.51
* Conv. Total (m3/s)	* 1851.8	* Conv. (m3/s)	* 501.8	* 854.8	* 495.2
* Length wtd. (m)	* 211.51	* Wetted Per. (m)	* 8.42	* 3.26	* 8.22
* Min Ch El (m)	* 176.36	* Shear (N/m2)	* 15.89	* 27.45	* 15.99
* Alpha	* 1.82	* Stream Power (N/m s)	* 22.59	* 99.18	* 22.83
* Frctn Loss (m)	* 0.19	* Cum Volume (1000 m3)	* 5.48	* 6.16	* 6.15
* C & E Loss (m)	* 0.03	* Cum SA (1000 m2)	* 3.54	* 2.34	* 3.96

CROSS SECTION

RIVER: Turkey Creek

REACH: Main RS: 9.9

INPUT

Description:

Station Elevation Data		num= 32		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	120.028	182.5	136.616	182	137.952	181.5	139.286	181
140.62	180.5	141.772	180	142.859	179.5	143.941	179	145.023	178.5
146.105	178	147.187	177.5	148.269	177	149.351	176.5	152.673	176.06
153.273	175.86	155.273	175.86	155.873	176.06	159.195	176.5	160.399	177
161.603	177.5	162.805	178	164.002	178.5	165.197	179	166.39	179.5
167.992	180	169.998	180.5	172.048	181	174.284	181.5	180.916	182
191.253	182.5	277.679	183						

Manning's n Values		num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.03	152.673	.017	155.873	.03		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	152.673	155.873		104.462	100.685	96.329	.1 .3

CROSS SECTION OUTPUT Profile #100 yr

* E.G. Elev (m)	* 178.48	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.18	* wt. n-val.	* 0.030	* 0.017	* 0.030
* W.S. Elev (m)	* 178.30	* Reach Len. (m)	* 104.46	* 100.68	* 96.33
* Crit W.S. (m)	*	* Flow Area (m2)	* 10.23	* 7.69	* 10.63
* E.G. Slope (m/m)	* 0.000579	* Area (m2)	* 10.23	* 7.69	* 10.63
* Q Total (m3/s)	* 39.50	* Flow (m3/s)	* 9.96	* 19.28	* 10.26
* Top width (m)	* 18.08	* Top width (m)	* 7.22	* 3.20	* 7.66
* Vel Total (m/s)	* 1.38	* Avg. Vel. (m/s)	* 0.97	* 2.51	* 0.97
* Max chl Dpth (m)	* 2.44	* Hydr. Depth (m)	* 1.42	* 2.40	* 1.39
* Conv. Total (m3/s)	* 1642.2	* Conv. (m3/s)	* 414.2	* 801.6	* 426.5
* Length wtd. (m)	* 100.42	* Wetted Per. (m)	* 7.65	* 3.26	* 8.04
* Min ch El (m)	* 175.86	* Shear (N/m2)	* 7.59	* 13.37	* 7.50
* Alpha	* 1.85	* Stream Power (N/m s)	* 7.39	* 33.51	* 7.24
* Frctn Loss (m)	* 0.06	* Cum Volume (1000 m3)	* 1.97	* 3.53	* 2.32
* C & E Loss (m)	* 0.00	* Cum SA (1000 m2)	* 1.57	* 1.67	* 1.91

CROSS SECTION OUTPUT Profile #Regional

* E.G. Elev (m)	* 179.04	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.26	* wt. n-val.	* 0.030	* 0.017	* 0.030
* W.S. Elev (m)	* 178.78	* Reach Len. (m)	* 104.46	* 100.68	* 96.33
* Crit W.S. (m)	*	* Flow Area (m2)	* 13.93	* 9.22	* 14.55
* E.G. Slope (m/m)	* 0.000698	* Area (m2)	* 13.93	* 9.22	* 14.55
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 16.67	* 28.63	* 17.30
* Top width (m)	* 20.25	* Top width (m)	* 8.26	* 3.20	* 8.80
* Vel Total (m/s)	* 1.66	* Avg. Vel. (m/s)	* 1.20	* 3.11	* 1.19
* Max chl Dpth (m)	* 2.92	* Hydr. Depth (m)	* 1.69	* 2.88	* 1.65
* Conv. Total (m3/s)	* 2369.7	* Conv. (m3/s)	* 631.1	* 1083.9	* 654.7
* Length wtd. (m)	* 100.36	* Wetted Per. (m)	* 8.79	* 3.26	* 9.28
* Min ch El (m)	* 175.86	* Shear (N/m2)	* 10.85	* 19.33	* 10.73
* Alpha	* 1.88	* Stream Power (N/m s)	* 12.99	* 60.02	* 12.75
* Frctn Loss (m)	* 0.08	* Cum Volume (1000 m3)	* 2.80	* 4.34	* 3.31
* C & E Loss (m)	* 0.00	* Cum SA (1000 m2)	* 1.85	* 1.67	* 2.17

CROSS SECTION

RIVER: Turkey Creek

REACH: Main RS: 9.8

INPUT

Description: US of Huron Church Road

Station Elevation Data		num= 30		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	37.489	182	45.643	181.5	47.136	181	48.636	180.5
50.024	180	51.1	179.5	52.174	179	53.246	178.5	54.316	178
55.384	177.5	56.451	177	57.519	176.5	60.347	176.06	60.947	175.86
62.947	175.86	63.547	176.06	66.375	176.5	67.488	177	71.05	177.5
72.19	178	73.33	178.5	74.472	179	75.609	179.5	76.733	180
77.849	180.5	94.858	181	118.672	181.5	142.126	182	143.791	182.5

Manning's n Values		num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val

Sta n Val Sta n Val Sta n Val

0 .03 60.347 .017 63.547 .03

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
60.347 63.547 24.356 24.248 24.356 .1 .3

CROSS SECTION OUTPUT Profile #100 yr

Table with 7 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top Width, Vel Total, Max Chl Dpth, Conv. Total, Length wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

CROSS SECTION OUTPUT Profile #Regional

Table with 7 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top Width, Vel Total, Max Chl Dpth, Conv. Total, Length wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

CROSS SECTION

RIVER: Turkey Creek
REACH: Main RS: 9.7

INPUT

Description: DS of Huron Church Road

Table with 10 columns: Station, Elevation, Station, Elevation, Station, Elevation, Station, Elevation, Station, Elevation. Rows show station data points.

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val

0 .02 148.469 .017 157.961 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
148.469 157.961 35.414 35.414 35.414 .3 .5

CROSS SECTION OUTPUT Profile #100 yr

Table with 7 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top Width, Vel Total.

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dric.rep
* Max Ch1 Dpth (m) * 2.38 * Hydr. Depth (m) * 0.62 * 1.97 * 0.99 *
* Conv. Total (m3/s) * 1985.3 * Conv. (m3/s) * 59.8 * 1683.1 * 242.4 *
* Length wtd. (m) * 0.62 * Wetted Per. (m) * 3.06 * 9.86 * 5.50 *
* Min Ch El (m) * 175.86 * Shear (N/m2) * 2.21 * 7.36 * 3.60 *
* Alpha * 1.19 * Stream Power (N/m s) * 1.51 * 13.19 * 3.40 *
* Frctn Loss (m) * 0.00 * Cum Volume (1000 m3) * 0.86 * 2.45 * 1.08 *
* C & E Loss (m) * 0.00 * Cum SA (1000 m2) * 0.74 * 1.19 * 0.93 *
*****

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```

CROSS SECTION OUTPUT Profile #Regional
*****
* E.G. Elev (m) * 178.92 * Element * Left OB * Channel * Right OB *
* Vel Head (m) * 0.21 * wt. n-Val. * 0.020 * 0.017 * 0.020 *
* W.S. Elev (m) * 178.72 * Reach Len. (m) * 0.62 * 0.62 * 0.62 *
* Crit W.S. (m) * 177.87 * Flow Area (m2) * 3.32 * 23.25 * 7.74 *
* E.G. Slope (m/m) * 0.000433 * Area (m2) * 3.32 * 23.25 * 7.74 *
* Q Total (m3/s) * 62.60 * Flow (m3/s) * 2.97 * 50.43 * 9.20 *
* Top width (m) * 19.12 * Top width (m) * 3.79 * 9.49 * 5.83 *
* Vel Total (m/s) * 1.82 * Avg. Vel. (m/s) * 0.89 * 2.17 * 1.19 *
* Max Ch1 Dpth (m) * 2.86 * Hydr. Depth (m) * 0.88 * 2.45 * 1.33 *
* Conv. Total (m3/s) * 3008.5 * Conv. (m3/s) * 142.8 * 2423.6 * 442.1 *
* Length wtd. (m) * 0.62 * Wetted Per. (m) * 4.16 * 9.86 * 6.34 *
* Min Ch El (m) * 175.86 * Shear (N/m2) * 3.39 * 10.02 * 5.18 *
* Alpha * 1.21 * Stream Power (N/m s) * 3.03 * 21.72 * 6.16 *
* Frctn Loss (m) * 0.00 * Cum Volume (1000 m3) * 1.27 * 3.04 * 1.58 *
* C & E Loss (m) * 0.00 * Cum SA (1000 m2) * 0.89 * 1.19 * 1.06 *
*****

```

BRIDGE

RIVER: Turkey Creek
 REACH: Main RS: 9.65

INPUT
 Description:
 Distance from Upstream XS = .621
 Deck/Roadway width = 34.79
 Weir Coefficient = 1.44
 Upstream Deck/Roadway Coordinates

num=	9				
Sta Hi Cord	Lo Cord	Sta Hi Cord	Lo Cord	Sta Hi Cord	Lo Cord
0	181.74	90.83	182.081	118.067	182.057
140.62	182.199	181.5	182.207	181.5	182.179
168.067	182.156	190.032	181.964	218.067	181.884

Upstream Bridge Cross Section Data

Station Elevation Data num= 22

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182	135.597	182	140.617	181.5	141.147	181	141.663	180.5
142.193	180	142.948	179.5	144.066	179	145.159	178.5	146.135	178
147.372	177.5	148.469	177	149.758	176.5	151.708	176.06	152.308	175.86
154.308	175.86	154.908	176.06	156.858	176.5	157.961	177	161.475	177.1
167.781	181.5	235.174	182						

Manning's n Values num= 3

Sta	n val	Sta	n val	Sta	n val
0	.02	148.469	.017	157.961	.02

Bank Sta: Left Right Coeff Contr. Expan.
 148.469 157.961 .3 .5

Downstream Deck/Roadway Coordinates num= 11

Sta Hi Cord	Lo Cord	Sta Hi Cord	Lo Cord	Sta Hi Cord	Lo Cord
0	181.74	90.83	182.081	118.067	182.057
133.85	182.157	181.5	182.199	181.5	182.207
165.46	182.179	181.5	182.156	181.5	182.11
190.032	181.964	218.067	181.884		

Downstream Bridge Cross Section Data

Station Elevation Data num= 18

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182	125.33	182	133.85	181.5	139.252	179.5	140.572	177
141.87	176.5	145.27	176.06	145.87	175.86	147.87	175.86	148.47	176.06

151.87 176.5 152.973 177 156.487 177.1 159.275 179.5 161.948 180.5
 166.14 181 170.386 181.5 199.519 182

dric.rep

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val

 0 .02 140.572 .017 152.973 .02

Bank Sta: Left Right Coeff Contr. Expan.
 140.572 152.973 .3 .5

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
 Downstream Embankment side slope = 0 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .95
 Elevation at which weir flow begins =
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data

Energy

Selected Low Flow Methods = Highest Energy Answer

High Flow Method

Pressure and weir flow

Submerged Inlet Cd =
 Submerged Inlet + Outlet cd = .8
 Max Low Cord =

Additional Bridge Parameters

Add Friction component to Momentum
 Do not add weight component to Momentum
 Class B flow critical depth computations use critical depth
 inside the bridge at the upstream end
 Criteria to check for pressure flow = Upstream energy grade line

BRIDGE OUTPUT Profile #100 yr

 * E.G. US. (m) * 178.38 * Element *Inside BR US *Inside BR DS *
 * W.S. US. (m) * 178.24 * E.G. Elev (m) * 178.38 * 178.35 *
 * Q Total (m3/s) * 39.50 * W.S. Elev (m) * 178.24 * 178.25 *
 * Q Bridge (m3/s) * 39.50 * Crit W.S. (m) * 177.50 * 177.31 *
 * Q Weir (m3/s) * * Max Chl Dpth (m) * 2.38 * 2.39 *
 * Weir Sta Lft (m) * * Vel Total (m/s) * 1.55 * 1.32 *
 * Weir Sta Rgt (m) * * Flow Area (m2) * 25.52 * 29.91 *
 * Weir Submerg * * Froude # Chl * 0.41 * 0.33 *
 * Weir Max Depth (m) * * Specif Force (m3) * 29.14 * 33.37 *
 * Min El Weir Flow (m) * 181.95 * Hydr Depth (m) * 1.46 * 1.67 *
 * Min El Prs (m) * 181.50 * W.P. Total (m) * 18.42 * 19.41 *
 * Delta EG (m) * 0.03 * Conv. Total (m3/s) * 1984.8 * 2482.2 *
 * Delta WS (m) * -0.01 * Top width (m) * 17.43 * 17.91 *
 * BR Open Area (m2) * 99.65 * Frctn Loss (m) * 0.01 * 0.00 *
 * BR Open Vel (m/s) * 1.55 * C & E Loss (m) * 0.02 * 0.00 *
 * Coef of Q * * Shear Total (N/m2) * 5.38 * 3.83 *
 * Br Sel Method *Energy only * Power Total (N/m s) * 8.33 * 5.05 *

BRIDGE OUTPUT Profile #Regional

 * E.G. US. (m) * 178.92 * Element *Inside BR US *Inside BR DS *
 * W.S. US. (m) * 178.72 * E.G. Elev (m) * 178.92 * 178.88 *
 * Q Total (m3/s) * 62.60 * W.S. Elev (m) * 178.72 * 178.73 *
 * Q Bridge (m3/s) * 62.60 * Crit W.S. (m) * 177.87 * 177.64 *
 * Q Weir (m3/s) * * Max Chl Dpth (m) * 2.86 * 2.87 *
 * Weir Sta Lft (m) * * Vel Total (m/s) * 1.82 * 1.61 *
 * Weir Sta Rgt (m) * * Flow Area (m2) * 34.31 * 38.79 *
 * Weir Submerg * * Froude # Chl * 0.44 * 0.37 *
 * Weir Max Depth (m) * * Specif Force (m3) * 49.38 * 55.26 *
 * Min El Weir Flow (m) * 181.95 * Hydr Depth (m) * 1.79 * 2.07 *
 * Min El Prs (m) * 181.50 * W.P. Total (m) * 20.36 * 20.71 *
 * Delta EG (m) * 0.04 * Conv. Total (m3/s) * 3007.8 * 3671.7 *
 * Delta WS (m) * -0.02 * Top width (m) * 19.11 * 18.73 *
 * BR Open Area (m2) * 99.65 * Frctn Loss (m) * 0.01 * 0.00 *
 * BR Open Vel (m/s) * 1.82 * C & E Loss (m) * 0.03 * 0.00 *
 * Coef of Q * * Shear Total (N/m2) * 7.16 * 5.34 *
 * Br Sel Method *Energy only * Power Total (N/m s) * 13.06 * 8.62 *

CROSS SECTION

RIVER: Turkey Creek
 REACH: Main RS: 9.6

INPUT

Description:

Station Elevation Data		num= 18							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182	125.33	182	133.85	181.5	139.252	179.5	140.572	177
141.87	176.5	145.27	176.06	145.87	175.86	147.87	175.86	148.47	176.06
151.87	176.5	152.973	177	156.487	177.1	159.275	179.5	161.948	180.5
166.14	181	170.386	181.5	199.519	182				

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	140.572	.017	152.973	.02

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	140.572	152.973		3.943	3	4.798	.3
							.5

CROSS SECTION OUTPUT Profile #100 yr

* E.G. Elev (m)	* 178.35	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.10	* Wt. n-Val.	* 0.020	* 0.017	* 0.020
* W.S. Elev (m)	* 178.25	* Reach Len. (m)	* 3.94	* 3.00	* 4.80
* Crit W.S. (m)	*	* Flow Area (m2)	* 0.41	* 24.52	* 4.98
* E.G. Slope (m/m)	* 0.000253	* Area (m2)	* 0.41	* 24.52	* 4.98
* Q Total (m3/s)	* 39.50	* Flow (m3/s)	* 0.14	* 35.54	* 3.81
* Top Width (m)	* 17.91	* Top width (m)	* 0.66	* 12.40	* 4.85
* Vel Total (m/s)	* 1.32	* Avg. Vel. (m/s)	* 0.35	* 1.45	* 0.77
* Max Chl Dpth (m)	* 2.39	* Hydr. Depth (m)	* 0.62	* 1.98	* 1.03
* Conv. Total (m3/s)	* 2482.2	* Conv. (m3/s)	* 9.1	* 2233.4	* 239.7
* Length wtd. (m)	* 3.18	* Wetted Per. (m)	* 1.41	* 12.72	* 5.28
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 0.72	* 4.79	* 2.34
* Alpha	* 1.12	* Stream Power (N/m s)	* 0.25	* 6.94	* 1.80
* Frctn Loss (m)	* 0.00	* Cum volume (1000 m3)	* 0.83	* 1.69	* 0.91
* C & E Loss (m)	* 0.01	* Cum SA (1000 m2)	* 0.68	* 0.81	* 0.75

CROSS SECTION OUTPUT Profile #Regional

* E.G. Elev (m)	* 178.88	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.15	* Wt. n-Val.	* 0.020	* 0.017	* 0.020
* W.S. Elev (m)	* 178.73	* Reach Len. (m)	* 3.94	* 3.00	* 4.80
* Crit W.S. (m)	*	* Flow Area (m2)	* 0.79	* 30.53	* 7.47
* E.G. Slope (m/m)	* 0.000291	* Area (m2)	* 0.79	* 30.53	* 7.47
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 0.37	* 54.88	* 7.35
* Top Width (m)	* 18.73	* Top width (m)	* 0.92	* 12.40	* 5.41
* Vel Total (m/s)	* 1.61	* Avg. Vel. (m/s)	* 0.47	* 1.80	* 0.98
* Max Chl Dpth (m)	* 2.87	* Hydr. Depth (m)	* 0.87	* 2.46	* 1.38
* Conv. Total (m3/s)	* 3671.7	* Conv. (m3/s)	* 21.7	* 3218.8	* 431.2
* Length wtd. (m)	* 3.22	* Wetted Per. (m)	* 1.96	* 12.72	* 6.02
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 1.15	* 6.84	* 3.54
* Alpha	* 1.13	* Stream Power (N/m s)	* 0.54	* 12.29	* 3.48
* Frctn Loss (m)	* 0.00	* Cum volume (1000 m3)	* 1.20	* 2.09	* 1.31
* C & E Loss (m)	* 0.01	* Cum SA (1000 m2)	* 0.81	* 0.81	* 0.86

CROSS SECTION

RIVER: Turkey Creek
 REACH: Main RS: 9.5

INPUT

Description:

Station Elevation Data		num= 14							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	58.964	182	67.647	181.5	80.298	176.5	83.844	176.06
84.444	175.86	86.444	175.86	87.044	176.06	90.59	176.5	92.841	177.5
98.2	180	99.757	180.5	106.622	181.5	138.493	182		

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val

0 .02 80.298 .017 90.59 .02 dric.rep

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
80.298 90.59 39.545 39.545 39.545 .1 .3

CROSS SECTION OUTPUT Profile #100 yr

Table with 10 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top width, Vel Total, Max Chl Dpth, Conv. Total, Length wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION OUTPUT Profile #Regional

Table with 10 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top width, Vel Total, Max Chl Dpth, Conv. Total, Length wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION

RIVER: Turkey Creek
REACH: Main RS: 9.4

INPUT

Table with 10 columns: Station, Elevation, Data, num=, Sta, Elev, Sta, Elev, Sta, Elev. Rows show station data points.

Table with 6 columns: Manning's n values, num=, Sta, n Val, Sta, n Val. Shows Manning's n values for different stations.

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
50.694 68.2 3.308 3.678 4.068 .1 .3

CROSS SECTION OUTPUT Profile #100 yr

Table with 10 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total.

dric.rep

* Top Width (m)	* 29.59	* Top Width (m)	* 7.31	* 17.51	* 4.77
* Vel Total (m/s)	* 0.84	* Avg. Vel. (m/s)	* 0.46	* 0.96	* 0.45
* Max Chl Dpth (m)	* 2.41	* Hydr. Depth (m)	* 0.88	* 2.06	* 0.88
* Conv. Total (m3/s)	* 3900.9	* Conv. (m3/s)	* 292.7	* 3421.6	* 186.6
* Length wtd. (m)	* 3.67	* wetted Per. (m)	* 7.52	* 17.60	* 5.09
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 0.86	* 2.06	* 0.83
* Alpha	* 1.17	* Stream Power (N/m s)	* 0.40	* 1.98	* 0.37
* Frctn Loss (m)	* 0.00	* Cum Volume (1000 m3)	* 0.62	* 0.49	* 0.74
* C & E Loss (m)	* 0.00	* Cum SA (1000 m2)	* 0.44	* 0.22	* 0.56

CROSS SECTION OUTPUT Profile #Regional

* E.G. Elev (m)	* 178.84	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.06	* Wt. n-Val.	* 0.020	* 0.017	* 0.020
* W.S. Elev (m)	* 178.77	* Reach Len. (m)	* 3.31	* 3.68	* 4.07
* Crit W.S. (m)	*	* Flow Area (m2)	* 10.68	* 44.88	* 6.97
* E.G. Slope (m/m)	* 0.000114	* Area (m2)	* 10.68	* 44.88	* 6.97
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 6.10	* 52.62	* 3.89
* Top Width (m)	* 33.03	* Top Width (m)	* 9.39	* 17.51	* 6.13
* Vel Total (m/s)	* 1.00	* Avg. Vel. (m/s)	* 0.57	* 1.17	* 0.56
* Max Chl Dpth (m)	* 2.91	* Hydr. Depth (m)	* 1.14	* 2.56	* 1.14
* Conv. Total (m3/s)	* 5862.1	* Conv. (m3/s)	* 570.8	* 4927.4	* 363.8
* Length wtd. (m)	* 3.67	* wetted Per. (m)	* 9.67	* 17.60	* 6.54
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 1.24	* 2.85	* 1.19
* Alpha	* 1.20	* Stream Power (N/m s)	* 0.71	* 3.34	* 0.66
* Frctn Loss (m)	* 0.00	* Cum Volume (1000 m3)	* 0.85	* 0.60	* 1.03
* C & E Loss (m)	* 0.00	* Cum SA (1000 m2)	* 0.50	* 0.22	* 0.62

CROSS SECTION

RIVER: Turkey Creek
REACH: Main RS: 9.3

INPUT

Description:
Station Elevation Data num= 14

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	22.465	182	35.015	181.5	36.837	181	51.171	176.5
58.923	176.06	59.523	175.86	61.523	175.86	62.123	176.06	69.875	176.5
81.981	180.5	86.631	181	89.072	182	225.65	182.5		

Manning's n values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.02	51.171	.017	69.875	.02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
51.171 69.875 3.308 5 4.166 .1 .3

CROSS SECTION OUTPUT Profile #100 yr

* E.G. Elev (m)	* 178.31	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.04	* Wt. n-Val.	* 0.020	* 0.017	* 0.020
* W.S. Elev (m)	* 178.27	* Reach Len. (m)	* 3.31	* 5.00	* 4.17
* Crit W.S. (m)	*	* Flow Area (m2)	* 5.00	* 38.48	* 4.75
* E.G. Slope (m/m)	* 0.000094	* Area (m2)	* 5.00	* 38.48	* 4.75
* Q Total (m3/s)	* 39.50	* Flow (m3/s)	* 2.16	* 35.29	* 2.05
* Top Width (m)	* 29.71	* Top Width (m)	* 5.64	* 18.70	* 5.36
* Vel Total (m/s)	* 0.82	* Avg. Vel. (m/s)	* 0.43	* 0.92	* 0.43
* Max Chl Dpth (m)	* 2.41	* Hydr. Depth (m)	* 0.89	* 2.06	* 0.89
* Conv. Total (m3/s)	* 4084.4	* Conv. (m3/s)	* 223.5	* 3649.2	* 211.6
* Length wtd. (m)	* 4.49	* wetted Per. (m)	* 5.92	* 18.79	* 5.65
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 0.78	* 1.88	* 0.77
* Alpha	* 1.15	* Stream Power (N/m s)	* 0.34	* 1.72	* 0.33
* Frctn Loss (m)	* 0.00	* Cum Volume (1000 m3)	* 0.60	* 0.35	* 0.72
* C & E Loss (m)	* 0.00	* Cum SA (1000 m2)	* 0.42	* 0.16	* 0.54

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION OUTPUT Profile #Regional

* E.G. Elev (m)	* 178.83	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.06	* Wt. n-Val.	* 0.020	* 0.017	* 0.020
* W.S. Elev (m)	* 178.78	* Reach Len. (m)	* 3.31	* 5.00	* 4.17

		dric.rep	
* Crit W.S. (m)	*	* Flow Area (m2)	* 8.25 * 47.91 * 7.84 *
* E.G. Slope (m/m)	* 0.000105	* Area (m2)	* 8.25 * 47.91 * 7.84 *
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 4.47 * 53.90 * 4.23 *
* Top Width (m)	* 32.84	* Top Width (m)	* 7.25 * 18.70 * 6.89 *
* Vel Total (m/s)	* 0.98	* Avg. Vel. (m/s)	* 0.54 * 1.13 * 0.54 *
* Max Chl Dpth (m)	* 2.92	* Hydr. Depth (m)	* 1.14 * 2.56 * 1.14 *
* Conv. Total (m3/s)	* 6107.2	* Conv. (m3/s)	* 435.8 * 5258.7 * 412.7 *
* Length Wtd. (m)	* 4.45	* Wetted Per. (m)	* 7.60 * 18.79 * 7.25 *
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 1.12 * 2.63 * 1.11 *
* Alpha	* 1.18	* Stream Power (N/m s)	* 0.61 * 2.96 * 0.60 *
* Frctn Loss (m)	* 0.00	* Cum Volume (1000 m3)	* 0.82 * 0.43 * 1.00 *
* C & E Loss (m)	* 0.00	* Cum SA (1000 m2)	* 0.47 * 0.16 * 0.59 *

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION

RIVER: Turkey Creek
 REACH: Main RS: 9.2

INPUT

Description:

Station Elevation Data		num= 28	
Sta	Elev	Sta	Elev
0	182	12.725	181.5
19.579	179.5	21.223	179
27.224	177	28.735	176.5
40.175	176.06	48.415	176.5
54.188	178.5	55.665	179
65.963	181	67.032	181.5

Manning's n Values		num= 3	
Sta	n Val	Sta	n Val
0	.03	36.975	.017
40.175			.03

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	36.975	40.175		30.4	31.81	33.814	.1 .3

CROSS SECTION OUTPUT Profile #100 yr

		Element	Left OB	Channel	Right OB
* E.G. Elev (m)	* 178.31	* Element	* 0.030	* 0.017	* 0.030
* Vel Head (m)	* 0.05	* Wt. n-Val.	* 30.40	* 31.81	* 33.81
* W.S. Elev (m)	* 178.26	* Reach Len. (m)	* 20.91	* 7.55	* 20.71
* Crit W.S. (m)	*	* Flow Area (m2)	* 20.91	* 7.55	* 20.71
* E.G. Slope (m/m)	* 0.000229	* Area (m2)	* 13.92	* 11.75	* 13.83
* Q Total (m3/s)	* 39.50	* Flow (m3/s)	* 13.49	* 3.20	* 13.30
* Top Width (m)	* 29.99	* Top width (m)	* 0.67	* 1.56	* 0.67
* Vel Total (m/s)	* 0.80	* Avg. Vel. (m/s)	* 1.55	* 2.36	* 1.56
* Max Chl Dpth (m)	* 2.40	* Hydr. Depth (m)	* 919.8	* 776.3	* 913.5
* Conv. Total (m3/s)	* 2609.6	* Conv. (m3/s)	* 13.79	* 3.26	* 13.61
* Length Wtd. (m)	* 32.03	* Wetted Per. (m)	* 3.41	* 5.19	* 3.42
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 2.27	* 8.09	* 2.28
* Alpha	* 1.60	* Stream Power (N/m s)	* 0.55	* 0.24	* 0.67
* Frctn Loss (m)	* 0.01	* Cum Volume (1000 m3)	* 0.38	* 0.10	* 0.50
* C & E Loss (m)	* 0.00	* Cum SA (1000 m2)	*	*	*

CROSS SECTION OUTPUT Profile #Regional

		Element	Left OB	Channel	Right OB
* E.G. Elev (m)	* 178.83	* Element	* 0.030	* 0.017	* 0.030
* Vel Head (m)	* 0.08	* Wt. n-Val.	* 30.40	* 31.81	* 33.81
* W.S. Elev (m)	* 178.76	* Reach Len. (m)	* 28.02	* 9.15	* 27.71
* Crit W.S. (m)	*	* Flow Area (m2)	* 28.02	* 9.15	* 27.71
* E.G. Slope (m/m)	* 0.000265	* Area (m2)	* 22.69	* 17.41	* 22.50
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 15.00	* 3.20	* 14.77
* Top Width (m)	* 32.97	* Top width (m)	* 0.81	* 1.90	* 0.81
* Vel Total (m/s)	* 0.96	* Avg. Vel. (m/s)	* 1.87	* 2.86	* 1.88
* Max Chl Dpth (m)	* 2.90	* Hydr. Depth (m)	* 1392.8	* 1069.0	* 1381.3
* Conv. Total (m3/s)	* 3843.1	* Conv. (m3/s)	* 15.38	* 3.26	* 15.16
* Length Wtd. (m)	* 32.07	* Wetted Per. (m)	* 4.74	* 7.29	* 4.76
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 3.84	* 13.88	* 3.86
* Alpha	* 1.59	* Stream Power (N/m s)	* 0.76	* 0.29	* 0.93
* Frctn Loss (m)	* 0.01	* Cum Volume (1000 m3)	* 0.44	* 0.10	* 0.55
* C & E Loss (m)	* 0.00	* Cum SA (1000 m2)	*	*	*

CROSS SECTION

RIVER: Turkey Creek
 REACH: Main RS: 9.1

INPUT

Description:

Station Elevation Data num= 29

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	181.5	84.335	181.5	89.24	181	91.04	180.5	92.826	180
94.617	179.5	96.484	179	98.345	178.5	100.173	178	101.97	177.5
103.725	177	106.858	176.5	111.217	176.06	111.817	175.86	113.817	175.86
114.417	176.06	118.776	176.5	122.227	177	125.249	177.5	130.295	178
131.458	178.5	132.634	179	133.814	179.5	134.997	180	136.182	180.5
142.388	181	143.457	181.5	144.535	182	251.616	182		

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.03	111.217	.017	114.417	.03

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

111.217	114.417	0	0	0	.1	.3
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CROSS SECTION OUTPUT Profile #100 yr

* E.G. Elev (m)	* 178.30	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.09	* wt. n-Val.	* 0.030	* 0.017	* 0.030
* W.S. Elev (m)	* 178.21	* Reach Len. (m)	*	*	*
* Crit W.S. (m)	* 177.38	* Flow Area (m2)	* 15.54	* 7.39	* 18.67
* E.G. Slope (m/m)	* 0.000375	* Area (m2)	* 15.54	* 7.39	* 18.67
* Q Total (m3/s)	* 39.50	* Flow (m3/s)	* 11.90	* 14.52	* 13.08
* Top width (m)	* 31.36	* Top Width (m)	* 11.80	* 3.20	* 16.36
* Vel Total (m/s)	* 0.95	* Avg. Vel. (m/s)	* 0.77	* 1.96	* 0.70
* Max Chl Dpth (m)	* 2.35	* Hydr. Depth (m)	* 1.32	* 2.31	* 1.14
* Conv. Total (m3/s)	* 2038.5	* Conv. (m3/s)	* 614.4	* 749.2	* 675.0
* Length Wtd. (m)	*	* Wetted Per. (m)	* 12.03	* 3.26	* 16.52
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 4.76	* 8.33	* 4.16
* Alpha	* 1.95	* Stream Power (N/m s)	* 3.64	* 16.37	* 2.91
* Frctn Loss (m)	*	* Cum volume (1000 m3)	*	*	*
* C & E Loss (m)	*	* Cum SA (1000 m2)	*	*	*

CROSS SECTION OUTPUT Profile #Regional

* E.G. Elev (m)	* 178.82	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.11	* wt. n-Val.	* 0.030	* 0.017	* 0.030
* W.S. Elev (m)	* 178.71	* Reach Len. (m)	*	*	*
* Crit W.S. (m)	* 177.71	* Flow Area (m2)	* 21.94	* 9.00	* 27.19
* E.G. Slope (m/m)	* 0.000375	* Area (m2)	* 21.94	* 9.00	* 27.19
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 19.16	* 20.16	* 23.29
* Top width (m)	* 34.39	* Top Width (m)	* 13.65	* 3.20	* 17.53
* Vel Total (m/s)	* 1.08	* Avg. Vel. (m/s)	* 0.87	* 2.24	* 0.86
* Max Chl Dpth (m)	* 2.85	* Hydr. Depth (m)	* 1.61	* 2.81	* 1.55
* Conv. Total (m3/s)	* 3231.8	* Conv. (m3/s)	* 989.1	* 1040.6	* 1202.2
* Length Wtd. (m)	*	* Wetted Per. (m)	* 13.95	* 3.26	* 17.80
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 5.79	* 10.14	* 5.62
* Alpha	* 1.83	* Stream Power (N/m s)	* 5.05	* 22.71	* 4.81
* Frctn Loss (m)	*	* Cum volume (1000 m3)	*	*	*
* C & E Loss (m)	*	* Cum SA (1000 m2)	*	*	*

SUMMARY OF MANNING'S N VALUES

River: Turkey Creek

* Reach	* River Sta.	* n1	* n2	* n3
*Main	* 10	* .03*	* .017*	* .03*
*Main	* 9.9	* .03*	* .017*	* .03*
*Main	* 9.8	* .03*	* .017*	* .03*
*Main	* 9.7	* .02*	* .017*	* .02*
*Main	* 9.65	* Bridge	*	*
*Main	* 9.6	* .02*	* .017*	* .02*
*Main	* 9.5	* .02*	* .017*	* .02*

```

*Main      * 9.4      *      .02*      dric.rep      .017*      .02*
*Main      * 9.3      *      .02*      .017*      .02*
*Main      * 9.2      *      .03*      .017*      .03*
*Main      * 9.1      *      .03*      .017*      .03*
*****

```

SUMMARY OF REACH LENGTHS

River: Turkey Creek

```

*****
* Reach      * River Sta. * Left * Channel * Right *
*****
*Main      * 10      * 207.817* 210.778* 216.406*
*Main      * 9.9      * 104.462* 100.685* 96.329*
*Main      * 9.8      * 24.356* 24.248* 24.356*
*Main      * 9.7      * 35.414* 35.414* 35.414*
*Main      * 9.65     * Bridge * * *
*Main      * 9.6      * 3.943* 3* 4.798*
*Main      * 9.5      * 39.545* 39.545* 39.545*
*Main      * 9.4      * 3.308* 3.678* 4.068*
*Main      * 9.3      * 3.308* 5* 4.166*
*Main      * 9.2      * 30.4* 31.81* 33.814*
*Main      * 9.1      * 0* 0* 0*
*****

```

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

River: Turkey Creek

```

*****
* Reach      * River Sta. * Contr. * Expan. *
*****
*Main      * 10      * .1* .3*
*Main      * 9.9      * .1* .3*
*Main      * 9.8      * .1* .3*
*Main      * 9.7      * .3* .5*
*Main      * 9.65     * Bridge * *
*Main      * 9.6      * .3* .5*
*Main      * 9.5      * .1* .3*
*Main      * 9.4      * .1* .3*
*Main      * 9.3      * .1* .3*
*Main      * 9.2      * .1* .3*
*Main      * 9.1      * .1* .3*
*****

```

Appendix A.5.3.2

Proposed Condition

dric.rep

HEC-RAS Version 3.1.3 May 2005
U.S. Army Corp of Engineers
Hydrologic Engineering Center
609 Second Street
Davis, California

X X XXXXXX XXXX XXXX XX XXXX
X X X X X X X X
X X X X X X X
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PROJECT DATA

Project Title: Turkey River
Project File : dric.prj
Run Date and Time: 15/11/2006 2:55:20 PM

Project in SI units

PLAN DATA

Plan Title: Plan 36
Plan File : o:\DRIC\19_waterResources\hec\dric.p36

Geometry Title: proposed
Geometry File : o:\DRIC\19_waterResources\hec\dric.g02

Flow Title : Flow 01
Flow File : o:\DRIC\19_waterResources\hec\dric.f01

Plan Summary Information:

Number of: Cross Sections = 10 Multiple Openings = 0
Culverts = 0 Inline Structures = 0
Bridges = 2 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.003
Critical depth calculation tolerance = 0.003
Maximum number of iterations = 20
Maximum difference tolerance = 0.1
Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary
Conveyance Calculation Method: At breaks in n values only
Friction Slope Method: Average Conveyance
Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: Flow 01
Flow File : o:\DRIC\19_waterResources\hec\dric.f01

Flow Data (m3/s)

* River Reach RS * 100 yr Regional *
* Turkey Creek Main 10 * 39.5 62.6 *

Boundary Conditions

* River Reach Profile * Upstream Downstream
* Turkey Creek Main 100 yr * Normal S = 0.000375
*

GEOMETRY DATA

Geometry Title: proposed
 Geometry File : o:\DRIC\19_waterResources\hec\dric.g02

CROSS SECTION

RIVER: Turkey Creek
 REACH: Main RS: 10

INPUT

Description:

Station Elevation Data		num= 30									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	183	76.736	183	99.766	182.5	101.278	182	102.644	181.5		
104.01	181	105.363	180.5	106.485	180	107.607	179.5	108.729	179		
109.851	178.5	110.973	178	112.095	177.5	113.217	177	116.947	176.56		
117.547	176.36	119.547	176.36	120.147	176.56	123.877	177	124.941	177.5		
126.005	178	127.069	178.5	128.133	179	129.197	179.5	130.261	180		
131.624	180.5	132.798	181	134.684	181.5	136.554	182	264.86	182.5		

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.03	116.947	.017	120.147	.03

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	116.947	120.147		207.817	210.778	216.406		.1	.3

CROSS SECTION OUTPUT Profile #100 yr

* E.G. Elev (m)	* 178.60	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.34	* wt. n-val.	* 0.030	* 0.017	* 0.030
* w.s. Elev (m)	* 178.26	* Reach Len. (m)	* 207.82	* 210.78	* 216.41
* Crit W.S. (m)	*	* Flow Area (m2)	* 7.29	* 5.95	* 7.20
* E.G. Slope (m/m)	*0.001477	* Area (m2)	* 7.29	* 5.95	* 7.20
* Q Total (m3/s)	* 39.50	* Flow (m3/s)	* 9.74	* 20.10	* 9.66
* Top width (m)	* 16.16	* Top width (m)	* 6.55	* 3.20	* 6.41
* Vel Total (m/s)	* 1.93	* Avg. Vel. (m/s)	* 1.34	* 3.38	* 1.34
* Max chl Dpth (m)	* 1.90	* Hydr. Depth (m)	* 1.11	* 1.86	* 1.12
* Conv. Total (m3/s)	* 1027.7	* Conv. (m3/s)	* 253.4	* 522.9	* 251.4
* Length Wtd. (m)	* 211.45	* Wetted Per. (m)	* 6.85	* 3.26	* 6.71
* Min Ch El (m)	* 176.36	* Shear (N/m2)	* 15.43	* 26.42	* 15.53
* Alpha	* 1.79	* Stream Power (N/m s)	* 20.61	* 89.18	* 20.85
* Frctn Loss (m)	* 0.23	* Cum Volume (1000 m3)	* 2.69	* 6.60	* 2.87
* C & E Loss (m)	* 0.03	* Cum SA (1000 m2)	* 2.28	* 2.51	* 2.58

CROSS SECTION OUTPUT Profile #Regional

* E.G. Elev (m)	* 179.19	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.43	* wt. n-val.	* 0.030	* 0.017	* 0.030
* w.s. Elev (m)	* 178.76	* Reach Len. (m)	* 207.82	* 210.78	* 216.41
* Crit W.S. (m)	*	* Flow Area (m2)	* 10.85	* 7.56	* 10.67
* E.G. Slope (m/m)	*0.001435	* Area (m2)	* 10.85	* 7.56	* 10.67
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 16.68	* 29.45	* 16.47
* Top width (m)	* 18.35	* Top width (m)	* 7.68	* 3.20	* 7.47
* Vel Total (m/s)	* 2.15	* Avg. Vel. (m/s)	* 1.54	* 3.90	* 1.54
* Max chl Dpth (m)	* 2.40	* Hydr. Depth (m)	* 1.41	* 2.36	* 1.43
* Conv. Total (m3/s)	* 1652.8	* Conv. (m3/s)	* 440.3	* 777.6	* 434.9
* Length Wtd. (m)	* 211.50	* Wetted Per. (m)	* 8.08	* 3.26	* 7.89
* Min Ch El (m)	* 176.36	* Shear (N/m2)	* 18.90	* 32.56	* 19.02
* Alpha	* 1.81	* Stream Power (N/m s)	* 29.05	* 126.90	* 29.37
* Frctn Loss (m)	* 0.24	* Cum Volume (1000 m3)	* 3.92	* 7.83	* 4.25
* C & E Loss (m)	* 0.03	* Cum SA (1000 m2)	* 2.69	* 2.51	* 3.01

CROSS SECTION

RIVER: Turkey Creek

REACH: Main RS: 9.9

INPUT

Description:

Station Elevation Data		num= 32		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	120.028	182.5	136.616	182	137.952	181.5	139.286	181
140.62	180.5	141.772	180	142.859	179.5	143.941	179	145.023	178.5
146.105	178	147.187	177.5	148.269	177	149.351	176.5	152.673	176.06
153.273	175.86	155.273	175.86	155.873	176.06	159.195	176.5	160.399	177
161.603	177.5	162.805	178	164.002	178.5	165.197	179	166.39	179.5
167.992	180	169.998	180.5	172.048	181	174.284	181.5	180.916	182
191.253	182.5	277.679	183						

Manning's n Values		num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
0	.03	152.673	.017	155.873	.03		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	152.673	155.873		104.462	100.685	96.329	.1 .3

CROSS SECTION OUTPUT Profile #100 yr

* E.G. Elev (m)	* 178.34	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.23	* wt. n-Val.	* 0.030	* 0.017	* 0.030
* W.S. Elev (m)	* 178.11	* Reach Len. (m)	* 104.46	* 100.68	* 96.33
* Crit W.S. (m)	*	* Flow Area (m2)	* 8.86	* 7.07	* 9.18
* E.G. Slope (m/m)	* 0.000814	* Area (m2)	* 8.86	* 7.07	* 9.18
* Q Total (m3/s)	* 39.50	* Flow (m3/s)	* 9.70	* 19.85	* 9.95
* Top Width (m)	* 17.19	* Top Width (m)	* 6.80	* 3.20	* 7.19
* Vel Total (m/s)	* 1.57	* Avg. Vel. (m/s)	* 1.09	* 2.81	* 1.08
* Max Chl Dpth (m)	* 2.25	* Hydr. Depth (m)	* 1.30	* 2.21	* 1.28
* Conv. Total (m3/s)	* 1384.8	* Conv. (m3/s)	* 339.9	* 696.1	* 348.8
* Length wtd. (m)	* 100.47	* Wetted Per. (m)	* 7.18	* 3.26	* 7.54
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 9.85	* 17.28	* 9.71
* Alpha	* 1.84	* Stream Power (N/m s)	* 10.77	* 48.52	* 10.53
* Frctn Loss (m)	* 0.11	* Cum Volume (1000 m3)	* 1.01	* 5.23	* 1.10
* C & E Loss (m)	* 0.01	* Cum SA (1000 m2)	* 0.90	* 1.84	* 1.11

CROSS SECTION OUTPUT Profile #Regional

* E.G. Elev (m)	* 178.92	* Element	* Left OB	* Channel	* Right OB
* Vel Head (m)	* 0.32	* wt. n-Val.	* 0.030	* 0.017	* 0.030
* W.S. Elev (m)	* 178.59	* Reach Len. (m)	* 104.46	* 100.68	* 96.33
* Crit W.S. (m)	*	* Flow Area (m2)	* 12.43	* 8.63	* 12.96
* E.G. Slope (m/m)	* 0.000916	* Area (m2)	* 12.43	* 8.63	* 12.96
* Q Total (m3/s)	* 62.60	* Flow (m3/s)	* 16.35	* 29.34	* 16.91
* Top Width (m)	* 19.40	* Top Width (m)	* 7.85	* 3.20	* 8.35
* Vel Total (m/s)	* 1.84	* Avg. Vel. (m/s)	* 1.32	* 3.40	* 1.31
* Max Chl Dpth (m)	* 2.73	* Hydr. Depth (m)	* 1.58	* 2.70	* 1.55
* Conv. Total (m3/s)	* 2068.9	* Conv. (m3/s)	* 540.2	* 969.7	* 558.9
* Length wtd. (m)	* 100.39	* Wetted Per. (m)	* 8.34	* 3.26	* 8.80
* Min Ch El (m)	* 175.86	* Shear (N/m2)	* 13.37	* 23.72	* 13.22
* Alpha	* 1.87	* Stream Power (N/m s)	* 17.60	* 80.69	* 17.26
* Frctn Loss (m)	* 0.11	* Cum Volume (1000 m3)	* 1.50	* 6.12	* 1.69
* C & E Loss (m)	* 0.01	* Cum SA (1000 m2)	* 1.08	* 1.84	* 1.30

CROSS SECTION

RIVER: Turkey Creek

REACH: Main RS: 9.8

INPUT

Description: US of Huron Church Road

Station Elevation Data		num= 30		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	37.489	182	45.643	181.5	47.136	181	48.636	180.5
50.024	180	51.1	179.5	52.174	179	53.246	178.5	54.316	178
55.384	177.5	56.451	177	57.519	176.5	60.347	176.06	60.947	175.86
62.947	175.86	63.547	176.06	66.375	176.5	67.488	177	71.05	177.5
72.19	178	73.33	178.5	74.472	179	75.609	179.5	76.733	180
77.849	180.5	94.858	181	118.672	181.5	142.126	182	143.791	182.5

Manning's n Values		num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val

Sta n Val Sta n Val Sta n Val

0 .03 60.347 .017 63.547 .03

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
60.347 63.547 24.356 24.248 24.356 .1 .3

CROSS SECTION OUTPUT Profile #100 yr

Table with 7 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top Width, Vel Total, Max Chl Dpth, Conv. Total, Length Wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

Warning: The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections.

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION OUTPUT Profile #Regional

Table with 7 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top Width, Vel Total, Max Chl Dpth, Conv. Total, Length Wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

Warning: The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections.

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION

RIVER: Turkey Creek
REACH: Main RS: 9.7

INPUT

Description: DS of Huron Church Road

Table with 10 columns: Station, Elevation, Station, Elevation, Station, Elevation, Station, Elevation, Station, Elevation. Rows show station and elevation data for three points.

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .02 146.159 .017 159.395 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
146.159 159.395 35.414 35.414 35.414 .3 .5

CROSS SECTION OUTPUT Profile #100 yr

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```

* E.G. Elev (m) * 178.14 * Element * Left OB * Channel * Right OB *
* Vel Head (m) * 0.14 * Wt. n-Val. * * 0.017 * *
* W.S. Elev (m) * 178.00 * Reach Len. (m) * 0.62 * 0.62 * 0.62 *
* Crit W.S. (m) * 177.15 * Flow Area (m2) * * 23.95 * *
* E.G. Slope (m/m) * 0.000472 * Area (m2) * * 23.95 * *
* Q Total (m3/s) * 39.50 * Flow (m3/s) * * 39.50 * *
* Top Width (m) * 13.24 * Top Width (m) * * 13.24 * *
* Vel Total (m/s) * 1.65 * Avg. Vel. (m/s) * * 1.65 * *
* Max Chl Dpth (m) * 2.14 * Hydr. Depth (m) * * 1.81 * *
* Conv. Total (m3/s) * 1818.7 * Conv. (m3/s) * * 1818.7 * *
* Length Wtd. (m) * 0.62 * Wetted Per. (m) * * 16.33 * *
* Min Ch El (m) * 175.86 * Shear (N/m2) * * 6.78 * *
* Alpha * 1.00 * Stream Power (N/m s) * * 11.19 * *
* Frctn Loss (m) * 0.00 * Cum Volume (1000 m3) * 0.13 * 4.19 * 0.17 *
* C & E Loss (m) * 0.00 * Cum SA (1000 m2) * 0.17 * 1.32 * 0.26 *
*****

```

CROSS SECTION OUTPUT Profile #Regional

```

*****
* E.G. Elev (m) * 178.71 * Element * Left OB * Channel * Right OB *
* Vel Head (m) * 0.21 * Wt. n-Val. * * 0.017 * *
* W.S. Elev (m) * 178.50 * Reach Len. (m) * 0.62 * 0.62 * 0.62 *
* Crit W.S. (m) * 177.50 * Flow Area (m2) * * 30.60 * *
* E.G. Slope (m/m) * 0.000567 * Area (m2) * * 30.60 * *
* Q Total (m3/s) * 62.60 * Flow (m3/s) * * 62.60 * *
* Top Width (m) * 13.24 * Top Width (m) * * 13.24 * *
* Vel Total (m/s) * 2.05 * Avg. Vel. (m/s) * * 2.05 * *
* Max Chl Dpth (m) * 2.64 * Hydr. Depth (m) * * 2.31 * *
* Conv. Total (m3/s) * 2629.0 * Conv. (m3/s) * * 2629.0 * *
* Length Wtd. (m) * 0.62 * Wetted Per. (m) * * 17.34 * *
* Min Ch El (m) * 175.86 * Shear (N/m2) * * 9.81 * *
* Alpha * 1.00 * Stream Power (N/m s) * * 20.07 * *
* Frctn Loss (m) * 0.00 * Cum Volume (1000 m3) * 0.23 * 4.82 * 0.31 *
* C & E Loss (m) * 0.00 * Cum SA (1000 m2) * 0.23 * 1.32 * 0.32 *
*****

```

BRIDGE

RIVER: Turkey Creek
REACH: Main RS: 9.65

INPUT

Description:
Distance from Upstream XS = .621
Deck/Roadway Width = 34.79
Weir Coefficient = 1.44
Upstream Deck/Roadway Coordinates

```

num= 9
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
*****
0 181.74 90.83 182.081 118.067 182.057
140.62 182.199 181.5 152.78 182.207 181.5 166.46 182.179 181.5
168.067 182.156 190.032 181.964 218.067 181.884

```

Upstream Bridge Cross Section Data

```

Station Elevation Data num= 13
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
*****
0 182 135.597 182 140.617 181.5 146.159 178.5 146.159 176.5
151.177 176.06 151.777 175.86 153.777 175.86 154.377 176.06 159.395 176.5
159.395 178.5 165.459 181.5 235.174 182

```

Manning's n Values

```

num= 3
Sta n Val Sta n Val Sta n Val
*****
0 .02 146.159 .017 159.395 .02

```

Bank Sta: Left Right Coeff Contr. Expan.
146.159 159.395 .3 .5

Downstream Deck/Roadway Coordinates

```

num= 11
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
*****
0 181.74 90.83 182.081 118.067 182.057
133.85 182.157 181.5 137.28 182.199 181.5 152.78 182.207 181.5
165.46 182.179 181.5 168.067 182.156 181.5 175.93 182.11 181.5
190.032 181.964 218.067 181.884

```

Downstream Bridge Cross Section Data

Station Elevation Data		num= 15							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182	125.33	182	133.85	181.5	140.248	179.5	140.248	176.5
145.27	176.06	145.87	175.86	147.87	175.86	148.47	176.06	153.492	176.5
153.492	179.5	161.948	180.5	166.14	181	170.386	181.5	199.519	182

Manning's n Values num= 3

Sta	n	Sta	n	Sta	n
0	.02	140.248	.017	153.492	.02

Bank Sta: Left Right Coeff Contr. Expan.

140.248	153.492	.3	.5
---------	---------	----	----

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
 Downstream Embankment side slope = 0 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .95
 Elevation at which weir flow begins =
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data

Energy
Selected Low Flow Methods = Highest Energy Answer

High Flow Method

Pressure and weir flow
 Submerged Inlet Cd =
 Submerged Inlet + outlet Cd = .8
 Max Low Cord =

Additional Bridge Parameters

Add Friction component to Momentum
 Do not add weight component to Momentum
 Class B flow critical depth computations use critical depth
 inside the bridge at the upstream end
 Criteria to check for pressure flow = Upstream energy grade line

BRIDGE OUTPUT Profile #100 yr

* E.G. US. (m)	*	178.14	* Element	* Inside BR US	* Inside BR DS
* W.S. US. (m)	*	178.00	* E.G. Elev (m)	* 178.14	* 178.12
* Q Total (m3/s)	*	39.50	* W.S. Elev (m)	* 178.00	* 177.98
* Q Bridge (m3/s)	*	39.50	* Crit W.S. (m)	* 177.16	* 177.16
* Q Weir (m3/s)	*		* Max Chl Dpth (m)	* 2.14	* 2.12
* Weir Sta Lft (m)	*		* Vel Total (m/s)	* 1.65	* 1.67
* Weir Sta Rgt (m)	*		* Flow Area (m2)	* 23.95	* 23.69
* Weir Submerg	*		* Froude # Chl	* 0.39	* 0.40
* Weir Max Depth (m)	*		* Specif Force (m3)	* 28.57	* 28.17
* Min El Weir Flow (m)	*	181.95	* Hydr Depth (m)	* 1.81	* 1.79
* Min El Prs (m)	*	181.50	* W.P. Total (m)	* 16.33	* 16.30
* Delta Egs (m)	*	0.02	* Conv. Total (m3/s)	* 1818.1	* 1788.3
* Delta WS (m)	*	0.02	* Top Width (m)	* 13.24	* 13.24
* BR Open Area (m2)	*	87.72	* Frctn Loss (m)	* 0.02	* 0.00
* BR Open Vel (m/s)	*	1.67	* C & E Loss (m)	* 0.00	* 0.00
* Coef of Q	*		* Shear Total (N/m2)	* 6.79	* 6.95
* Br Sel Method	*	*Energy only	* Power Total (N/m s)	* 11.19	* 11.59

BRIDGE OUTPUT Profile #Regional

* E.G. US. (m)	*	178.71	* Element	* Inside BR US	* Inside BR DS
* W.S. US. (m)	*	178.50	* E.G. Elev (m)	* 178.71	* 178.69
* Q Total (m3/s)	*	62.60	* W.S. Elev (m)	* 178.50	* 178.47
* Q Bridge (m3/s)	*	62.60	* Crit W.S. (m)	* 177.50	* 177.50
* Q Weir (m3/s)	*		* Max Chl Dpth (m)	* 2.64	* 2.61
* Weir Sta Lft (m)	*		* Vel Total (m/s)	* 2.05	* 2.07
* Weir Sta Rgt (m)	*		* Flow Area (m2)	* 30.60	* 30.28
* Weir Submerg	*		* Froude # Chl	* 0.43	* 0.44
* Weir Max Depth (m)	*		* Specif Force (m3)	* 48.68	* 48.06
* Min El Weir Flow (m)	*	181.95	* Hydr Depth (m)	* 2.31	* 2.29
* Min El Prs (m)	*	181.50	* W.P. Total (m)	* 17.34	* 17.30
* Delta Egs (m)	*	0.02	* Conv. Total (m3/s)	* 2628.3	* 2587.0
* Delta WS (m)	*	0.03	* Top Width (m)	* 13.24	* 13.24
* BR Open Area (m2)	*	87.72	* Frctn Loss (m)	* 0.02	* 0.00

```

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* BR Open vel (m/s)      *      2.07 * C & E Loss (m)      *      0.00 *      0.00 *
* Coef of Q              *              * Shear Total (N/m2) *      9.82 *      10.05 *
* Br Sel Method          *Energy only * Power Total (N/m s) *      20.09 *      20.78 *
*****

```

CROSS SECTION

RIVER: Turkey Creek
 REACH: Main RS: 9.6

INPUT

Description:

```

Station Elevation Data num= 15
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
*****
0 182 125.33 182 133.85 181.5 140.248 179.5 140.248 176.5
145.27 176.06 145.87 175.86 147.87 175.86 148.47 176.06 153.492 176.5
153.492 179.5 161.948 180.5 166.14 181 170.386 181.5 199.519 182

```

```

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
*****
0 .02 140.248 .017 153.492 .02

```

```

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
140.248 153.492 8.508 3 8.362 .3 .5

```

CROSS SECTION OUTPUT Profile #100 yr

```

*****
* E.G. Elev (m) * 178.12 * Element * Left OB * Channel * Right OB *
* Vel Head (m) * 0.14 * Wt. n-Val. * * 0.017 * *
* W.S. Elev (m) * 177.98 * Reach Len. (m) * 8.51 * 3.00 * 8.36 *
* Crit W.S. (m) * * * Flow Area (m2) * * 23.69 * *
* E.G. Slope (m/m) * 0.000488 * Area (m2) * * 23.69 * *
* Q Total (m3/s) * 39.50 * Flow (m3/s) * * 39.50 * *
* Top width (m) * 13.24 * Top width (m) * * 13.24 * *
* Vel Total (m/s) * 1.67 * Avg. Vel. (m/s) * * 1.67 * *
* Max Chl Dpth (m) * 2.12 * Hydr. Depth (m) * * 1.79 * *
* Conv. Total (m3/s) * 1788.3 * Conv. (m3/s) * * 1788.3 * *
* Length wtd. (m) * 3.00 * Wetted Per. (m) * * 16.30 * *
* Min Ch El (m) * 175.86 * Shear (N/m2) * * 6.95 * *
* Alpha * 1.00 * Stream Power (N/m s) * * 11.59 * *
* Frctn Loss (m) * 0.00 * Cum Volume (1000 m3) * 0.13 * 3.34 * 0.17 *
* C & E Loss (m) * 0.06 * Cum SA (1000 m2) * 0.17 * 0.85 * 0.26 *
*****

```

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION OUTPUT Profile #Regional

```

*****
* E.G. Elev (m) * 178.69 * Element * Left OB * Channel * Right OB *
* Vel Head (m) * 0.22 * Wt. n-Val. * * 0.017 * *
* W.S. Elev (m) * 178.47 * Reach Len. (m) * 8.51 * 3.00 * 8.36 *
* Crit W.S. (m) * * * Flow Area (m2) * * 30.28 * *
* E.G. Slope (m/m) * 0.000586 * Area (m2) * * 30.28 * *
* Q Total (m3/s) * 62.60 * Flow (m3/s) * * 62.60 * *
* Top width (m) * 13.24 * Top width (m) * * 13.24 * *
* Vel Total (m/s) * 2.07 * Avg. Vel. (m/s) * * 2.07 * *
* Max Chl Dpth (m) * 2.61 * Hydr. Depth (m) * * 2.29 * *
* Conv. Total (m3/s) * 2587.0 * Conv. (m3/s) * * 2587.0 * *
* Length wtd. (m) * 3.00 * Wetted Per. (m) * * 17.30 * *
* Min Ch El (m) * 175.86 * Shear (N/m2) * * 10.05 * *
* Alpha * 1.00 * Stream Power (N/m s) * * 20.78 * *
* Frctn Loss (m) * 0.00 * Cum Volume (1000 m3) * 0.23 * 3.74 * 0.31 *
* C & E Loss (m) * 0.09 * Cum SA (1000 m2) * 0.23 * 0.85 * 0.32 *
*****

```

Warning: The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections.

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION

RIVER: Turkey Creek
 REACH: Main RS: 9.5

INPUT

Description:

Station Elevation Data num= 14

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	58.964	182	66.492	180	66.492	176.5	80.892	176.06		
81.492	175.86	83.492	175.86	84.092	176.06	98.492	176.5	98.492	180		
99.757	180.5	103.154	181	106.622	181.5	138.493	182				

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.02	66.492	.017	98.492	.02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

66.492	98.492	39.545	39.545	39.545	.3	.5
--------	--------	--------	--------	--------	----	----

CROSS SECTION OUTPUT Profile #100 yr

	*	178.06	*	Element	*	Left OB	*	Channel	*	Right OB	*
* E.G. Elev (m)	*	0.02	*	Wt. n-Val.	*		*	0.017	*		*
* Vel Head (m)	*	178.03	*	Reach Len. (m)	*	0.14	*	0.14	*	0.14	*
* W.S. Elev (m)	*	176.78	*	Flow Area (m2)	*		*	57.38	*		*
* Crit W.S. (m)	*	*0.000071	*	Area (m2)	*		*	57.38	*		*
* E.G. Slope (m/m)	*	39.50	*	Flow (m3/s)	*		*	39.50	*		*
* Q Total (m3/s)	*	32.00	*	Top Width (m)	*		*	32.00	*		*
* Top width (m)	*	0.69	*	Avg. vel. (m/s)	*		*	0.69	*		*
* Vel Total (m/s)	*	2.17	*	Hydr. Depth (m)	*		*	1.79	*		*
* Max Chl Dpth (m)	*	4679.2	*	Conv. (m3/s)	*		*	4679.2	*		*
* Conv. Total (m3/s)	*	0.14	*	Wetted Per. (m)	*		*	35.15	*		*
* Length wtd. (m)	*	175.86	*	Shear (N/m2)	*		*	1.14	*		*
* Min Ch El (m)	*	1.00	*	Stream Power (N/m s)	*		*	0.79	*		*
* Alpha	*	0.00	*	Cum Volume (1000 m3)	*	0.13	*	3.22	*	0.17	*
* Frctn Loss (m)	*	0.00	*	Cum SA (1000 m2)	*	0.17	*	0.78	*	0.26	*
* C & E Loss (m)	*		*		*		*		*		*

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION OUTPUT Profile #Regional

	*	178.60	*	Element	*	Left OB	*	Channel	*	Right OB	*
* E.G. Elev (m)	*	0.04	*	Wt. n-Val.	*		*	0.017	*		*
* Vel Head (m)	*	178.56	*	Reach Len. (m)	*	0.14	*	0.14	*	0.14	*
* W.S. Elev (m)	*	176.97	*	Flow Area (m2)	*		*	74.31	*		*
* Crit W.S. (m)	*	*0.000079	*	Area (m2)	*		*	74.31	*		*
* E.G. Slope (m/m)	*	62.60	*	Flow (m3/s)	*		*	62.60	*		*
* Q Total (m3/s)	*	32.00	*	Top Width (m)	*		*	32.00	*		*
* Top width (m)	*	0.84	*	Avg. vel. (m/s)	*		*	0.84	*		*
* Vel Total (m/s)	*	2.70	*	Hydr. Depth (m)	*		*	2.32	*		*
* Max Chl Dpth (m)	*	7059.7	*	Conv. (m3/s)	*		*	7059.7	*		*
* Conv. Total (m3/s)	*	0.14	*	Wetted Per. (m)	*		*	36.21	*		*
* Length wtd. (m)	*	175.86	*	Shear (N/m2)	*		*	1.58	*		*
* Min Ch El (m)	*	1.00	*	Stream Power (N/m s)	*		*	1.33	*		*
* Alpha	*	0.00	*	Cum Volume (1000 m3)	*	0.23	*	3.59	*	0.31	*
* Frctn Loss (m)	*	0.01	*	Cum SA (1000 m2)	*	0.23	*	0.78	*	0.32	*
* C & E Loss (m)	*		*		*		*		*		*

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

BRIDGE

RIVER: Turkey Creek
REACH: Main RS: 9.45

INPUT

Description:

Distance from Upstream XS = .143
Deck/Roadway width = 36.402
Weir coefficient = 1.44
Upstream Deck/Roadway Coordinates

num= 17

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
0	180.1				32.49	180				66.99	179.06			176.06
66.99	179.06	177.86			76.99	179.03	177.86			76.99	179.03	177.86		176.06
77.49	179.03	176.06			77.49	179.03	177.86			82.49	179.03	177.86		177.86
87.49	179.03	177.86			87.49	179.03	176.06			87.99	179.03	176.06		176.06

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87.99	179.03	177.86	97.99	179.03	177.86	97.99	179.03	176.06
132.49	178.3		182.49	178				

Upstream Bridge Cross Section Data

Station Elevation Data num= 14

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	58.964	182	66.492	180	66.492	176.5	80.892	176.06
81.492	175.86	83.492	175.86	84.092	176.06	98.492	176.5	98.492	180
99.757	180.5	103.154	181	106.622	181.5	138.493	182		

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.02	66.492	.017	98.492	.02

Bank Sta: Left Right Coeff Contr. Expan.
 66.492 98.492 .3 .5

Downstream Deck/Roadway Coordinates

num= 17

Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
0	180				9.55	180				44.05	179.06	176.06		
44.05	179.06	177.86			54.05	179.03	177.86			54.05	179.03	176.06		
54.55	179.03	176.06			54.55	179.03	177.86			59.55	179.03	177.86		
64.55	179.03	177.86			64.55	179.03	176.06			65.05	179.03	176.06		
65.05	179.03	177.86			75.05	179.03	177.86			75.05	179.03	176.06		
109.55	178.3				159.55	178								

Downstream Bridge Cross Section Data

Station Elevation Data num= 15

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	182.5	22.451	182	30.035	181.5	43.553	179.5	43.553	176.5
57.953	176.06	58.553	175.86	60.553	175.86	61.153	176.06	75.553	176.5
75.553	179.5	84.157	181	86.382	181.5	87.423	182	124.022	182.5

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.02	43.553	.017	75.553	.02

Bank Sta: Left Right Coeff Contr. Expan.
 43.553 75.553 .3 .5

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
 Downstream Embankment side slope = 0 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .95
 Elevation at which weir flow begins =
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data

Energy
 Selected Low Flow Methods = Highest Energy Answer

High Flow Method
 Energy only

Additional Bridge Parameters

Add Friction component to Momentum
 Do not add weight component to Momentum
 Class B flow critical depth computations use critical depth
 inside the bridge at the upstream end
 Criteria to check for pressure flow = Upstream energy grade line

BRIDGE OUTPUT Profile #100 yr

* E.G. US. (m)	*	178.06	* Element	* Inside BR US	* Inside BR DS	*
* W.S. US. (m)	*	178.03	* E.G. Elev (m)	*	178.06	*
* Q Total (m3/s)	*	39.50	* W.S. Elev (m)	*	178.02	*
* Q Bridge (m3/s)	*	39.50	* Crit W.S. (m)	*	176.80	*
* Q Weir (m3/s)	*		* Max chl dpth (m)	*	2.16	*
* Weir Sta Lft (m)	*		* Vel Total (m/s)	*	0.81	*
* Weir Sta Rgt (m)	*		* Flow Area (m2)	*	48.73	*
* Weir Submerg	*		* Froude # chl	*	0.19	*

```

* Weir Max Depth (m) * * * dric.rep * * * 51.18 * * * 50.63 *
* Min El Weir Flow (m) * * * 179.02 * Hydr Depth (m) * * * * * *
* Min El Prs (m) * * * 177.86 * W.P. Total (m) * * * 69.58 * * * 69.58 *
* Delta EG (m) * * * 0.02 * Conv. Total (m3/s) * * * 2260.3 * * * 2260.2 *
* Delta WS (m) * * * 0.02 * Top width (m) * * * * * *
* BR Open Area (m2) * * * 48.73 * Frctn Loss (m) * * * 0.01 * * * 0.00 *
* BR Open Vel (m/s) * * * 0.81 * C & E Loss (m) * * * 0.00 * * * 0.00 *
* Coef of Q * * * * * Shear Total (N/m2) * * * 2.10 * * * 2.10 *
* Br Sel Method *Energy only * Power Total (N/m s) * * * 1.70 * * * 1.70 *
*****

```

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

```

BRIDGE OUTPUT Profile #Regional
*****
* E.G. US. (m) * * * 178.60 * Element *Inside BR US *Inside BR DS *
* W.S. US. (m) * * * 178.56 * E.G. Elev (m) * * * 178.59 * * * 178.56 *
* Q Total (m3/s) * * * 62.60 * W.S. Elev (m) * * * 178.50 * * * 178.47 *
* Q Bridge (m3/s) * * * 62.60 * Crit W.S. (m) * * * 177.00 * * * 177.00 *
* Q Weir (m3/s) * * * * * Max chl Dpth (m) * * * 2.64 * * * 2.61 *
* Weir Sta Lft (m) * * * * * Vel Total (m/s) * * * 1.28 * * * 1.28 *
* Weir Sta Rgt (m) * * * * * Flow Area (m2) * * * 48.73 * * * 48.73 *
* Weir Submerg * * * * * Froude # Chl * * * 0.27 * * * 0.27 *
* Weir Max Depth (m) * * * * * Specif Force (m3) * * * 79.45 * * * 78.09 *
* Min El Weir Flow (m) * * * 179.02 * Hydr Depth (m) * * * * * *
* Min El Prs (m) * * * 177.86 * W.P. Total (m) * * * 69.58 * * * 69.58 *
* Delta EG (m) * * * 0.07 * Conv. Total (m3/s) * * * 2260.3 * * * 2260.2 *
* Delta WS (m) * * * 0.07 * Top width (m) * * * * * *
* BR Open Area (m2) * * * 48.73 * Frctn Loss (m) * * * 0.03 * * * 0.00 *
* BR Open Vel (m/s) * * * 1.28 * C & E Loss (m) * * * 0.00 * * * 0.02 *
* Coef of Q * * * * * Shear Total (N/m2) * * * 5.27 * * * 5.27 *
* Br Sel Method *Energy only * Power Total (N/m s) * * * 6.77 * * * 6.77 *
*****

```

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION

RIVER: Turkey Creek
REACH: Main RS: 9.4

INPUT

Description:

```

Station Elevation Data num= 15
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
*****
0 182.5 22.451 182 30.035 181.5 43.553 179.5 43.553 176.5
57.953 176.06 58.553 175.86 60.553 175.86 61.153 176.06 75.553 176.5
75.553 179.5 84.157 181 86.382 181.5 87.423 182 124.022 182.5

```

```

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
*****
0 .02 43.553 .017 75.553 .02

```

```

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
43.553 75.553 5.639 5 6.353 .3 .5

```

```

CROSS SECTION OUTPUT Profile #100 yr
*****
* E.G. Elev (m) * * * 178.04 * Element * Left OB * Channel * Right OB *
* Vel Head (m) * * * 0.02 * Wt. n-val. * * * * * 0.017 * * *
* W.S. Elev (m) * * * 178.02 * Reach Len. (m) * * * 5.64 * * * 5.00 * * * 6.35 *
* Crit W.S. (m) * * * * * Flow Area (m2) * * * * * 56.76 * * *
* E.G. Slope (m/m) * * * 0.000074 * Area (m2) * * * * * 56.76 * * *
* Q Total (m3/s) * * * 39.50 * Flow (m3/s) * * * * * 39.50 * * *
* Top Width (m) * * * 32.00 * Top width (m) * * * * * 32.00 * * *
* Vel Total (m/s) * * * 0.70 * Avg. vel. (m/s) * * * * * 0.70 * * *
* Max chl Dpth (m) * * * 2.16 * Hydr. Depth (m) * * * * * 1.77 * * *
* Conv. Total (m3/s) * * * 4599.1 * Conv. (m3/s) * * * * * 4599.1 * * *
* Length Wtd. (m) * * * 5.00 * Wetted Per. (m) * * * * * 35.11 * * *
* Min Ch El (m) * * * 175.86 * Shear (N/m2) * * * * * 1.17 * * *
* Alpha * * * 1.00 * Stream Power (N/m s) * * * * * 0.81 * * *
* Frctn Loss (m) * * * 0.00 * Cum volume (1000 m3) * * * 0.13 * * * 1.28 * * * 0.17 *
* C & E Loss (m) * * * 0.00 * Cum SA (1000 m2) * * * 0.17 * * * 0.73 * * * 0.26 *
*****

```

CROSS SECTION OUTPUT Profile #Regional

Table with 6 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top width, Vel Total, Max Chl Dpth, Conv. Total, Length wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

CROSS SECTION

RIVER: Turkey Creek
REACH: Main

RS: 9.3

INPUT

Description:

Station Elevation Data table with columns: Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. Data points for stations 0, 47.673, 73.173, 225.65.

Manning's n values

Table with columns: Sta, n Val, Sta, n Val, Sta, n Val. Data points for stations 0, 47.673, 73.173.

Bank Sta: Left, Right, Lengths: Left Channel, Right, Coeff Contr., Expan. Data for stations 47.673, 73.173.

CROSS SECTION OUTPUT Profile #100 yr

Table with 6 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top width, Vel Total, Max Chl Dpth, Conv. Total, Length wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

CROSS SECTION OUTPUT Profile #Regional

Table with 6 columns: Parameter, Value, Element, Left OB, Channel, Right OB. Rows include E.G. Elev, Vel Head, W.S. Elev, Crit W.S., E.G. Slope, Q Total, Top width, Vel Total, Max Chl Dpth, Conv. Total, Length wtd., Min Ch El, Alpha, Frctn Loss, C & E Loss.

CROSS SECTION

RIVER: Turkey Creek
REACH: Main RS: 9.2

INPUT

Description:

Table with 10 columns: Station, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev. It lists elevation data for 28 stations.

Table with 6 columns: Manning's n, Sta, n Val, Sta, n Val, Sta, n Val. It lists Manning's n values for 3 stations.

Table with 7 columns: Bank Sta, Left, Right, Lengths, Left Channel, Right, Coeff Contr., Expan. It lists channel dimensions and coefficients.

CROSS SECTION OUTPUT Profile #100 yr

Table with 7 columns: Parameter, Value, Element, Left OB, Channel, Right OB. It lists various hydraulic parameters for the 100-year profile.

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION OUTPUT Profile #Regional

Table with 7 columns: Parameter, Value, Element, Left OB, Channel, Right OB. It lists various hydraulic parameters for the regional profile.

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION

RIVER: Turkey Creek
REACH: Main RS: 9.1

INPUT

Description:

Table with 3 columns: Station, Elev, Sta, num=, 29. It lists station and elevation data for 29 stations.

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	181.5	84.335	181.5	89.24	181	91.04	180.5	92.826	180
94.617	179.5	96.484	179	98.345	178.5	100.173	178	101.97	177.5
103.725	177	106.858	176.5	111.217	176.06	111.817	175.86	113.817	175.86
114.417	176.06	118.776	176.5	122.227	177	125.249	177.5	130.295	178
131.458	178.5	132.634	179	133.814	179.5	134.997	180	136.182	180.5
142.388	181	143.457	181.5	144.535	182	251.616	182		

Manning's n values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .03 106.858 .017 118.776 .03

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 106.858 118.776 0 0 .1 .3

CROSS SECTION OUTPUT Profile #100 yr
 * E.G. Elev (m) * 178.02 * Element * Left OB * Channel * Right OB *
 * Vel Head (m) * 0.12 * Wt. n-Val. * 0.030 * 0.017 * 0.030 *
 * W.S. Elev (m) * 177.90 * Reach Len. (m) * * * *
 * Crit w.s. (m) * 177.20 * Flow Area (m2) * 5.05 * 20.56 * 6.77 *
 * E.G. Slope (m/m) * 0.000375 * Area (m2) * 5.05 * 20.56 * 6.77 *
 * Q Total (m3/s) * 39.50 * Flow (m3/s) * 2.76 * 33.51 * 3.24 *
 * Top width (m) * 28.79 * Top width (m) * 6.34 * 11.92 * 10.54 *
 * Vel Total (m/s) * 1.22 * Avg. vel. (m/s) * 0.55 * 1.63 * 0.48 *
 * Max Chl Dpth (m) * 2.04 * Hydr. Depth (m) * 0.80 * 1.73 * 0.64 *
 * Conv. Total (m3/s) * 2038.9 * Conv. (m3/s) * 142.2 * 1729.7 * 167.0 *
 * Length wtd. (m) * * * Wetted Per. (m) * 6.50 * 12.03 * 10.63 *
 * Min Ch El (m) * 175.86 * Shear (N/m2) * 2.86 * 6.29 * 2.34 *
 * Alpha * 1.54 * Stream Power (N/m s) * 1.56 * 10.25 * 1.12 *
 * Frctn Loss (m) * * * Cum volume (1000 m3) * * * *
 * C & E Loss (m) * * * Cum SA (1000 m2) * * * *

CROSS SECTION OUTPUT Profile #Regional
 * E.G. Elev (m) * 178.51 * Element * Left OB * Channel * Right OB *
 * Vel Head (m) * 0.15 * Wt. n-Val. * 0.030 * 0.017 * 0.030 *
 * W.S. Elev (m) * 178.36 * Reach Len. (m) * * * *
 * Crit w.s. (m) * 177.53 * Flow Area (m2) * 8.32 * 26.00 * 12.13 *
 * E.G. Slope (m/m) * 0.000375 * Area (m2) * 8.32 * 26.00 * 12.13 *
 * Q Total (m3/s) * 62.60 * Flow (m3/s) * 5.41 * 49.53 * 7.66 *
 * Top width (m) * 32.27 * Top width (m) * 8.00 * 11.92 * 12.35 *
 * Vel Total (m/s) * 1.35 * Avg. vel. (m/s) * 0.65 * 1.90 * 0.63 *
 * Max Chl Dpth (m) * 2.50 * Hydr. Depth (m) * 1.04 * 2.18 * 0.98 *
 * Conv. Total (m3/s) * 3232.0 * Conv. (m3/s) * 279.3 * 2557.1 * 395.5 *
 * Length wtd. (m) * * * Wetted Per. (m) * 8.22 * 12.03 * 12.53 *
 * Min Ch El (m) * 175.86 * Shear (N/m2) * 3.72 * 7.95 * 3.56 *
 * Alpha * 1.63 * Stream Power (N/m s) * 2.42 * 15.15 * 2.25 *
 * Frctn Loss (m) * * * Cum volume (1000 m3) * * * *
 * C & E Loss (m) * * * Cum SA (1000 m2) * * * *

SUMMARY OF MANNING'S N VALUES

River: Turkey Creek

Reach	River Sta.	n1	n2	n3
Main	10	.03	.017*	.03*
Main	9.9	.03	.017*	.03*
Main	9.8	.03	.017*	.03*
Main	9.7	.02	.017*	.02*
*Main	9.65	*Bridge	*	*
Main	9.6	.02	.017*	.02*
Main	9.5	.02	.017*	.02*
*Main	9.45	*Bridge	*	*
Main	9.4	.02	.017*	.02*
Main	9.3	.02	.017*	.02*
Main	9.2	.03	.017*	.03*
Main	9.1	.03	.017*	.03*

SUMMARY OF REACH LENGTHS

River: Turkey Creek

```

*****
* Reach * River Sta. * Left * Channel * Right *
*****
*Main * 10 * 207.817* 210.778* 216.406*
*Main * 9.9 * 104.462* 100.685* 96.329*
*Main * 9.8 * 24.356* 24.248* 24.356*
*Main * 9.7 * 35.414* 35.414* 35.414*
*Main * 9.65 *Bridge * *
*Main * 9.6 * 8.508* 3* 8.362*
*Main * 9.5 * 39.545* 39.545* 39.545*
*Main * 9.45 *Bridge * *
*Main * 9.4 * 5.639* 5* 6.353*
*Main * 9.3 * 4.02* 3.772* 4.02*
*Main * 9.2 * 30.4* 31.81* 33.814*
*Main * 9.1 * 0* 0* 0*
*****

```

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS
River: Turkey Creek

```

*****
* Reach * River Sta. * Contr. * Expan. *
*****
*Main * 10 * .1* .3*
*Main * 9.9 * .1* .3*
*Main * 9.8 * .1* .3*
*Main * 9.7 * .3* .5*
*Main * 9.65 *Bridge * *
*Main * 9.6 * .3* .5*
*Main * 9.5 * .3* .5*
*Main * 9.45 *Bridge * *
*Main * 9.4 * .3* .5*
*Main * 9.3 * .1* .3*
*Main * 9.2 * .1* .3*
*Main * 9.1 * .1* .3*
*****

```

Appendix A.6

Alternative 3

Culvert Calculator Report

Basin Drain -All Alternatives - Reg Check

Solve For: Headwater Elevation

Culvert Summary

Allowable HW Elevation	181.40 m	Headwater Depth/Height	1.20
Computed Headwater Elevat	180.32 m	Discharge	8.1000 m ³ /s
Inlet Control HW Elev.	180.32 m	Tailwater Elevation	179.70 m
Outlet Control HW Elev.	180.32 m	Control Type	Inlet Control

Grades

Upstream Invert	178.50 m	Downstream Invert	178.20 m
Length	58.00 m	Constructed Slope	0.005172 m/m

Hydraulic Profile

Profile	CompositeS1S2	Depth, Downstream	1.05 m
Slope Type	Steep	Normal Depth	1.05 m
Flow Regime	N/A	Critical Depth	1.14 m
Velocity Downstream	3.62 m/s	Critical Slope	0.004178 m/m

Section

Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.13 m
Section Size	2130 x 1520 mm	Rise	1.52 m
Number Sections	1		

Outlet Control Properties

Outlet Control HW Elev.	180.32 m	Upstream Velocity Head	0.57 m
Ke	0.20	Entrance Loss	0.11 m

Inlet Control Properties

Inlet Control HW Elev.	180.32 m	Flow Control	Transition
Inlet Type	90° headwall w 45° bevels	Area Full	3.3 m ²
K	0.49500	HDS 5 Chart	10
M	0.66700	HDS 5 Scale	2
C	0.03140	Equation Form	2
Y	0.82000		

Titcombe Drain

Worksheet for Circular Channel

Project Description	
Worksheet	Titcome_prelimin
Flow Element	Circular Channel
Method	Manning's Formu
Solve For	Full Flow Diametr

Input Data	
Mannings Coeffici	0.013
Channel Slope	005000 m/m
Discharge	3.2000 m ³ /s

Results	
Depth	1.27 m
Diameter	1,269.0 mm
Flow Area	1.3 m ²
Wetted Perimet	4.30 m
Top Width	0.00 m
Critical Depth	0.97 m
Percent Full	100.0 %
Critical Slope	005735 m/m
Velocity	2.53 m/s
Velocity Head	0.33 m
Specific Energy	1.60 m
Froude Number	0.00
Maximum Disct	3.4423 m ³ /s
Discharge Full	3.2000 m ³ /s
Slope Full	005000 m/m
Flow Type	N/A

Notes: Discharge of 3.2 m³/s was taken from taking 50% of the 100 year flow of subcatchment 140 of turkey creek watershed.
 Drainage area D/S of Titcombe is approximately 274 ha, 55% of the entire subcatch #140. Drainage area therefore U/S Titcombe crossing is app.
 50% of the entire subcatch.
 From 1989 Maclaren report:
 Catch # 140
 DA = 496 Ha.
 100 yr existing = 6.4 m³/s
 100 yr efuture = 16.7 m³/s

Appendix B

Hydrologic Analysis Pre & Post Development Conditions

Appendix B.1

Pre-Development Condition

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients:

A: **2824.505**

B: **13.74**

C: **0.88**

**Rational Method Calculation
Existing Condition
Alternative 1A**

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration Tc (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m3/s)	Remarks
100	6.36	0.30	850	0.91	68.7	0.37	To Ttcombe Drain
101	2.53	0.30	733	0.82	73.9	0.16	To Basin Drain
102	5.60	0.30	833	0.26	143.5	0.68	Pump to Mangin Drain
103	2.60	0.30	747	0.83	73.2	0.16	To Turkey Creek
104	2.50	0.30	353	0.52	99.0	0.21	Pump to Lenon Drain
105	2.50	0.30	533	0.67	84.6	0.18	To Lenon Drain
106	5.60	0.30	767	0.85	72.0	0.34	Pump to Cahill Drain
107	3.10	0.30	881	0.92	67.9	0.18	To Wolfe Drain
108	3.96	0.30	834	0.89	69.6	0.23	Pump to Wolfe Drain
109	6.60	0.30	1737	1.45	48.8	0.27	To Wolfe Drain

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients:

A: **2824.505**

B: **13.74**

C: **0.88**

**Rational Method Calculation
Existing Condition
Alternative 1B**

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration Tc (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m ³ /s)	Remarks
100	6.36	0.30	885	0.93	67.5	0.36	To TTcombe Drain
101	2.70	0.30	468	0.62	89.0	0.20	To Basin Drain
102	5.38	0.30	670	0.78	76.3	0.34	Pump to Mangin Drain
103	4.50	0.30	847	0.90	69.0	0.26	Pump to Turkey Creek
104	2.74	0.30	451	0.61	90.2	0.21	Pump to Lenon Drain
105	7.21	0.30	1,016	1.02	63.3	0.38	Pump to Cahill Drain
106	6.17	0.30	1,498	1.31	52.6	0.27	Pump to Wolfe Drain
107	6.56	0.30	1737	1.45	48.8	0.27	To Wolfe Drain

Project: Detroit River International Crossing Study

Design Storm: 100-year

Rainfall Data: MTO District I Chatham

Rainfall Intensity Coefficients:

A: 2824.505

B: 13.74

C: 0.88

Rational Method Calculation

Existing Condition

Alternative 2A

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration Tc (hr)	Rainfall Intensity I (mm/h)	100 - year Peak Flow (m³/s)	Remarks
100	6.36	0.30	880	0.928	67.7	0.36	Drain to TTcombe Drain
101	1.69	0.30	483	0.626	88.3	0.13	Basin Drain
102	5.19	0.30	847	0.904	68.9	0.30	Pump to Mangin Drain
103	3.31	0.30	732	0.820	73.8	0.21	To Turkey Creek
104	4.93	0.30	556	0.696	82.4	0.34	Pump to Lennon Drain
105	2.61	0.30	529	0.667	84.8	0.19	To Cahill Drain
106	5.30	0.30	969	0.985	64.9	0.29	Pump to Cahill Drain
107	7.06	0.30	998	1.007	63.9	0.38	Pump to Wolfe Drain

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients:

A: **2824.505**

B: **13.74**

C: **0.88**

Rational Method Calculation

Existing Condition

Alternative 2B

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration Tc (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m ³ /s)	Remarks
100	6.36	0.30	880	0.93	67.7	0.36	Drain to TTcombe Drain
101	2.13	0.30	482	0.63	88.2	0.16	Basin Drain
102	6.54	0.30	884	0.93	67.5	0.37	Pump to Mangin Drain
103	7.21	0.30	1453	1.29	53.4	0.32	Pump to Turkey Creek
104	2.34	0.30	421	0.58	92.8	0.18	Pump to Lennon Drain
105	5.77	0.30	1377	1.24	54.8	0.27	Pump to Cahill Drain
106	9.32	0.30	1023	1.03	63.0	0.49	Pump to Wolfe Drain

Project: Detroit River International Crossing Study

Design Storm: 100-year

Rainfall Data: MTO District I Chatham

Rainfall Intensity Coefficients:

A: 2824.505

B: 13.74

C: 0.88

Rational Method Calculation

Existing Condition

Alternative 2B - Revised Proposed

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration Tc (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m³/s)	Remarks
100	6.36	0.30	810	0.88	70.2	0.37	To TTcombe Drain
101	8.67	0.30	1260	1.17	57.2	0.42	Pump To Basin Drain
102	6.22	0.30	1080	1.06	61.6	0.32	Pump to Lenon Drain
103	19.43	0.30	3240	2.22	35.0	0.57	Pump to Cahill Drain

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients: Inlet Time (t_i)= **5** min

A: **2824.505** Average Velocity = **1.5** m/s

B: **13.74**

C: **0.88**

Rational Method Calculation

Existing Conidition

Alternative 3

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration Tc (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m3/s)	Remarks
100	6.36	0.30	850	0.91	68.7	0.37	To TTcombe Drain
101	2.80	0.30	798	0.87	71.1	0.17	Pump to Basin Drain
102	0.34	0.30	342	0.50	101.7	0.03	Pump to Turkey Creek
103	0.34	0.30	342	0.50	101.7	0.03	Pump to Turkey Creek
104	0.14	0.30	140	0.29	138.1	0.02	Pump to Cahill Drain
105	0.19	0.30	194	0.35	124.6	0.02	Pump to Cahill Drain
106	0.17	0.30	174	0.33	129.1	0.02	Pump to Cahill Drain
107	0.19	0.30	191	0.35	125.3	0.02	Pump to Cahill Drain
108	2.16	0.30	774	0.85	72.1	0.13	Pump to Wolfe Drain
109	6.56	0.30	1737	1.45	48.8	0.27	To Wolfe Drain

Appendix B.2

Post Development Condition

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients: Inlet Time (t_i)= **5** min

Average Velocity = **1.5** m/s

A: **2824.505**

B: **13.74**

C: **0.88**

Rational Method Calculation

Proposed Conidition

Alternative 1A

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration $T_c = t_i + t_d$ (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m ³ /s)	Remarks
100	6.36	0.90	850	0.24	149.6	2.40	To TTcombe Drain
101	2.53	0.90	733	0.22	155.9	0.99	To Basin Drain
102	5.60	0.90	833	0.24	150.5	2.12	Pump to Mangin Drain
103	2.60	0.90	747	0.22	155.2	1.02	To Turkey Creek
104	2.50	0.90	353	0.15	181.2	1.14	Pump to Lenon Drain
105	2.50	0.90	533	0.18	168.2	1.06	To Lenon Drain
106	5.60	0.90	767	0.23	154.0	2.17	Pump to Cahill Drain
107	3.10	0.90	881	0.25	148.0	1.16	To Wolfe Drain
108	3.96	0.90	834	0.24	150.4	1.50	Pump to Wolfe Drain
109	6.60	0.90	1737	0.41	114.9	1.91	To Wolfe Drain

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients: Inlet Time (t_i) = **5** min

A: **2824.505** Average Velocity (v) = **1.5** m/s

B: **13.74**

C: **0.88**

Rational Method Calculation

Proposed Condition

Alternative 1B

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration $T_c = t_i + t_d$ (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m ³ /s)	Remarks
100	6.36	0.90	885	0.25	147.8	2.37	To TTcombe Drain
101	2.70	0.90	468	0.17	172.7	1.18	To Basin Drain
102	5.38	0.90	670	0.21	159.6	2.16	Pump to Mangin Drain
103	4.50	0.90	847	0.24	149.8	1.70	Pump to Turkey Creek
104	2.74	0.90	451	0.17	173.9	1.20	Pump to Lenon Drain
105	7.21	0.90	1,016	0.27	141.5	2.57	Pump to Cahill Drain
106	6.17	0.90	1,498	0.36	122.5	1.90	Pump to Wolfe Drain
107	6.56	0.90	1737	0.41	114.9	1.90	To Wolfe Drain

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients: Inlet Time (t_i)= **5** min

Average Velocity = **1.5** m/s

A: **2824.505**

B: **13.74**

C: **0.88**

Rational Method Calculation

Proposed Conidition

Alternative 2A

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration $T_c = t_i + t_d$ (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m ³ /s)	Remarks
100	6.36	0.90	880	0.25	148.1	2.37	Drain to TTcombe Drain
101	1.69	0.90	483	0.17	171.7	0.73	Basin Drain
102	5.19	0.90	847	0.24	149.8	1.96	Pump to Mangin Drain
103	3.31	0.90	732	0.22	156.0	1.30	To Turkey Creek
104	4.93	0.90	556	0.19	166.7	2.07	Pump to Lennon Drain
105	2.61	0.90	529	0.18	168.5	1.11	To Cahill Drain
106	5.30	0.90	969	0.26	143.7	1.92	Pump to Cahill Drain
107	7.06	0.90	998	0.27	142.3	2.53	Pump to Wolfe Drain

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients: Inlet Time (t_i)= **5** min

A: **2824.505** Average Velocity = **1.5** m/s

B: **13.74**

C: **0.88**

Rational Method Calculation

Proposed Condition

Alternative 2B

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration $T_c = t_i + t_d$ (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m^3/s)	Remarks
100	6.36	0.90	880	0.25	148.1	2.37	Drain to Ttcombe Drain
101	2.13	0.90	482	0.17	171.7	0.92	Basin Drain
102	6.54	0.90	884	0.25	147.9	2.44	Pump to Mangin Drain
103	7.21	0.90	1453	0.35	124.0	2.25	Pump to Turkey Creek
104	2.34	0.90	421	0.16	176.1	1.04	Pump to Lennon Drain
105	5.77	0.90	1377	0.34	126.7	1.84	Pump to Cahill Drain
106	9.32	0.90	1023	0.27	141.2	3.32	Pump to Wolfe Drain

STORM SEWER CALCULATIONS

FOR

Detroit Fiver International Crossing Study

Alternative 2B - Revised Profiles

Date: 11/16/06

File: 33015384

Designed by Kevin Chen

LOCATION	FROM MH	TO MH	CATCHMENT AREA NUMBERS	CATCHMENT CHARACTERISTICS			CONCENTRATION TIME (min)		CATCHMENT RUNOFF		PIPE CHARACTERISTICS						
				A (ha)	C	"AC" ACCUM.	INLET	IN PIPE	TOTAL	"I" (mm/hr)	"Q" (m ³ /s)	LENGTH (m)	DIAMETER (mm)	SLOPE (%)	CAPACITY (m ³ /s)	VELOCITY (m/s)	
To 2BR - P1	1	2	1	0.310	0.90	0.28	0.78	5.00	1.26	6.26	214.24	0.166	90.00	600	0.30	0.336	1.19
	2	3	2	0.320	0.90	0.29	0.57	6.26	1.26	7.52	202.31	0.318	90.00	600	0.30	0.336	1.19
	3	4	3	0.320	0.90	0.29	0.86	7.52	1.04	8.56	191.71	0.454	90.00	800	0.30	0.724	1.44
	4	5	4	0.580	0.90	0.52	1.38	8.56	1.04	9.60	183.81	0.701	90.00	800	0.30	0.724	1.44
	5	6	5	0.610	0.90	0.55	1.93	9.60	0.90	10.50	176.58	0.942	90.00	1000	0.30	1.313	1.67
	6	7	6	0.320	0.90	0.29	2.21	10.50	0.90	11.40	170.82	1.048	90.00	1000	0.30	1.313	1.67
	7	8	7	0.320	0.90	0.29	2.50	11.40	0.90	12.30	165.44	1.147	90.00	1000	0.30	1.313	1.67
	8	9	8	0.320	0.90	0.29	2.79	12.30	0.38	12.67	160.41	1.240	90.00	1000	1.70	3.126	3.98
	9	10	9	0.310	0.90	0.28	3.07	12.67	0.38	13.05	158.40	1.347	90.00	1000	1.70	3.126	3.98
	10	11	10	0.310	0.90	0.28	3.35	13.05	0.38	13.43	156.44	1.451	90.00	1000	1.70	3.126	3.98
	11	12	11	1.080	0.90	0.97	4.32	13.43	0.38	13.80	154.52	1.849	90.00	1000	1.70	3.126	3.98
	12	13	12	0.320	0.90	0.29	4.61	13.80	0.62	14.42	152.66	1.949	90.00	1200	0.50	2.757	2.44
	13	14	13	0.320	0.90	0.29	4.90	14.42	0.62	15.03	149.72	2.031	90.00	1200	0.50	2.757	2.44
	14	15	14	0.320	0.90	0.29	5.18	15.03	0.62	15.65	146.90	2.109	90.00	1200	0.50	2.757	2.44
	15	16	15	0.720	0.90	0.65	5.83	15.65	0.62	16.26	144.19	2.400	90.00	1200	0.50	2.757	2.44
	16	17	16	0.320	0.90	0.29	6.12	16.26	0.62	16.88	141.59	2.469	90.00	1200	0.50	2.757	2.44
	17	18	17	0.320	0.90	0.29	6.41	16.88	0.62	17.50	139.08	2.469	90.00	1200	0.50	2.757	2.44
	18	19	18	0.320	0.90	0.29	6.70	17.50	0.62	18.11	136.66	2.535	90.00	1200	0.50	2.757	2.44
	19	20	19	0.310	0.90	0.28	6.98	18.11	0.62	18.73	134.34	2.598	90.00	1200	0.50	2.757	2.44
	20	21	20	0.320	0.90	0.29	7.26	18.73	0.68	19.41	132.09	2.668	90.00	1500	0.30	3.872	2.19
	21	22	21	0.320	0.90	0.29	7.55	19.41	0.68	20.10	129.69	2.713	90.00	1500	0.30	3.872	2.19
	22	23	22	0.320	0.90	0.29	7.84	20.10	0.68	20.78	127.38	2.766	90.00	1500	0.30	3.872	2.19
	23	24	23	0.320	0.90	0.29	8.13	20.78	0.68	21.46	125.15	2.817	90.00	1500	0.30	3.872	2.19
	24	25	24	0.320	0.90	0.29	8.42	21.46	0.68	22.15	123.01	2.867	90.00	1500	0.30	3.872	2.19
	25	26	25	0.320	0.90	0.29	8.70	22.15	0.68	22.83	120.94	2.916	90.00	1500	0.30	3.872	2.19
	26	27	26	0.320	0.90	0.29	8.99	22.83	0.68	23.52	118.95	2.962	90.00	1500	0.30	3.872	2.19
	27	28	27	0.320	0.90	0.29	9.28	23.52	0.68	24.20	117.02	3.008	90.00	1500	0.30	3.872	2.19
	28	29	28	0.320	0.90	0.29	9.57	24.20	0.68	24.89	115.16	3.052	90.00	1500	0.30	3.872	2.19
	29	30	29	0.320	0.90	0.29	9.86	24.89	0.68	25.57	113.36	3.095	90.00	1500	0.30	3.872	2.19
	30	31	30	0.320	0.90	0.29	10.14	25.57	0.68	26.26	111.62	3.136	90.00	1500	0.30	3.872	2.19
	31	32	31	0.310	0.90	0.28	10.42	26.26	0.68	26.94	109.84	3.174	90.00	1500	0.30	3.872	2.19
	32	33	32	0.310	0.90	0.28	10.70	26.94	0.68	27.63	108.31	3.211	90.00	1500	0.30	3.872	2.19
	33	34	33	0.320	0.90	0.29	10.99	27.63	0.68	28.31	106.73	3.249	90.00	1500	0.30	3.872	2.19
	34	35	34	0.320	0.90	0.29	11.28	28.31	0.68	29.00	105.20	3.286	90.00	1500	0.30	3.872	2.19
	35	36	35	0.320	0.90	0.29	11.57	29.00	0.68	29.68	103.72	3.323	90.00	1500	0.30	3.872	2.19
	36	37	36	0.320	0.90	0.29	11.85	29.68	0.68	30.36	102.28	3.358	90.00	1500	0.30	3.872	2.19
			Sub Total	13.170									3240.00				
	50	49	50	0.490	0.90	0.11	0.44	5.00	1.26	6.26	214.24	0.262	90.00	600	0.30	0.336	1.19
	49	48	49	0.320	0.90	0.29	0.73	6.26	1.04	7.30	202.31	0.409	90.00	800	0.30	0.724	1.44
	48	47	48	0.320	0.90	0.29	1.02	7.30	1.04	8.34	193.47	0.545	90.00	800	0.30	0.724	1.44
	47	46	47	0.320	0.90	0.29	2.76	8.34	0.79	9.14	185.43	1.419	90.00	1200	0.30	2.135	1.89
	46	45	46	0.320	0.90	0.29	3.05	9.14	0.79	9.93	179.75	1.519	90.00	1200	0.30	2.135	1.89
	45	44	45	0.320	0.90	0.29	3.34	9.93	0.79	10.73	174.43	1.613	90.00	1200	0.30	2.135	1.89
	44	43	44	0.320	0.90	0.29	3.63	10.73	0.79	11.52	169.43	1.702	90.00	1200	0.30	2.135	1.89

S ORM SEWER CALCULATIONS

FOR

Detroit River International Crossing Study

Alternative 2B - Revised Profiles

Date: 11/16/06
File: 33015384

Designed by Kevin Chen

LOCATION	FROM MH	TO MH	CATCHMENT AREA NUMBERS	CATCHMENT CHARACTERISTICS			CONCENTRATION TIME (min)		CATCHMENT RUNOFF		PIPE CHARACTERISTICS						
				A (ft)	C	"AC"	ACCUM. "AC"	INLET	IN PIPE	TOTAL	"I" (mm/hr)	"Q" (m ³ /s)	LENGTH (m)	DIAMETER (mm)	SLOPE (%)	CAPACITY (m ³ /s)	VELOCITY (m/s)
	43	42	43	0.320	0.90	0.29	3.92	11.52	0.79	12.32	164.74	1.786	90.00	1200	0.30	2.135	1.89
	42	41	42	0.320	0.90	0.29	4.20	12.32	0.79	13.11	160.31	1.866	90.00	1200	0.30	2.135	1.89
	41	40	41	0.320	0.90	0.29	4.49	13.11	0.79	13.90	156.13	1.942	90.00	1200	0.30	2.135	1.89
	40	39	40	0.320	0.90	0.29	4.78	13.90	0.79	14.70	152.17	2.014	90.00	1200	0.30	2.135	1.89
	39	38	39	0.310	0.90	0.28	5.06	14.70	0.79	15.49	148.42	2.080	90.00	1200	0.30	2.135	1.89
	38	37	38	0.320	0.90	0.29	5.35	15.49	0.68	16.18	144.87	2.145	90.00	1500	0.30	3.872	2.19
			Sub Total	4.320													
	11a	12a	11a	0.140	0.90	0.13	0.13	5.00	0.84	5.84	214.24	0.075	90.00	450	1.00	0.285	1.79
	12a	13a	12a	0.180	0.90	0.16	0.29	5.84	0.84	6.67	206.16	0.164	90.00	450	1.00	0.285	1.79
	13a	47	13a	0.360	0.90	0.32	0.61	6.67	0.69	7.36	198.71	0.337	90.00	600	1.00	0.614	2.17
	14a	15a	14a	0.680	0.90	0.16	0.16	5.00	0.84	5.84	214.24	0.096	90.00	450	1.00	0.285	1.79
	15a	16a	15a	0.180	0.90	0.16	0.32	5.84	0.84	6.67	206.16	0.185	90.00	450	1.00	0.285	1.79
	16a	17a	16a	0.180	0.90	0.16	0.49	6.67	0.84	7.51	198.71	0.268	90.00	450	1.00	0.285	1.79
	17a	47	17a	0.400	0.90	0.36	0.85	7.51	0.69	8.20	191.81	0.449	90.00	600	1.00	0.614	2.17
	37	MHPOND		0.940	0.90	0.29	17.49	30.36	0.53	30.89	100.89	4.887	90.00	1500	0.50	4.998	2.83
			Drainage Area 19.43														
To 2BR - P2	51	52	51	0.320	0.90	0.29	0.29	5.00	1.18	6.18	214.24	0.171	90.00	450	0.50	0.202	1.27
	52	53	52	0.320	0.90	0.29	0.58	6.18	0.98	7.16	203.00	0.324	90.00	600	0.50	0.434	1.54
	53	54	53	0.310	0.90	0.28	0.86	7.16	0.81	7.97	194.63	0.461	90.00	800	0.50	0.935	1.86
	63	62	63	0.340	0.90	0.31	0.31	5.00	0.88	5.88	214.24	0.182	90.00	450	0.90	0.270	1.70
	62	61	62	0.320	0.90	0.29	0.59	5.88	0.73	6.61	205.74	0.339	90.00	600	0.90	0.583	2.06
	61	60	61	0.320	0.90	0.29	0.88	6.61	0.60	7.21	199.25	0.487	90.00	800	0.90	1.254	2.50
	60	59	60	0.320	0.90	0.29	1.17	7.21	0.74	7.95	194.21	0.629	90.00	800	0.60	1.024	2.04
	59	58	59	0.310	0.90	0.28	1.45	11.00	0.74	11.74	167.78	0.673	90.00	800	0.60	1.024	2.04
	58	57	58	0.320	0.90	0.29	1.74	11.74	0.74	12.47	163.51	0.787	90.00	800	0.60	1.024	2.04
	57	56	57	0.320	0.90	0.29	2.03	12.47	0.74	13.21	159.46	0.894	90.00	800	0.60	1.024	2.04
	56	55	56	0.320	0.90	0.29	2.31	13.21	0.74	13.94	155.62	0.997	90.00	800	0.60	1.024	2.04
	55	54	55	0.310	0.90	0.28	2.59	13.94	0.63	14.58	151.98	1.091	90.00	1000	0.60	1.857	2.36
	1a	2a	1a	0.180	0.90	0.16	0.16	5.00	0.84	5.84	214.24	0.096	1080.00	450	1.00	0.285	1.79
	2a	3a	2a	0.180	0.90	0.16	0.32	5.84	0.84	6.67	206.16	0.185	90.00	450	1.00	0.285	1.79
	3a	4a	3a	0.180	0.90	0.16	0.49	6.67	0.84	7.51	198.71	0.268	90.00	450	1.00	0.285	1.79
	4a	5a	4a	0.180	0.90	0.16	0.65	7.51	0.69	8.20	191.81	0.385	90.00	600	1.00	0.614	2.17
	5a	6a	5a	0.180	0.90	0.16	0.81	8.20	0.69	8.89	187.91	0.466	90.00	600	1.00	0.614	2.17
	6a	58	6a	0.200	0.90	0.18	0.99	8.89	0.69	9.58	184.24	0.552	90.00	600	1.00	0.614	2.17
	7a	8a	7a	0.180	0.90	0.16	0.16	5.00	0.84	5.84	214.24	0.096	90.00	450	1.00	0.285	1.79
	8a	9a	8a	0.180	0.90	0.16	0.32	5.84	0.84	6.67	206.16	0.185	90.00	450	1.00	0.285	1.79
	9a	10a	9a	0.180	0.90	0.16	0.49	6.67	0.84	7.51	198.71	0.268	90.00	450	1.00	0.285	1.79
	10a	58	10a	0.190	0.90	0.17	0.66	7.51	0.69	8.20	191.81	0.390	90.00	600	1.00	0.614	2.17

Date: 11/16/06
 File: 33015384

LOCATION	FROM MH	TO MH	CATCHMENT AREA NUMBERS	CATCHMENT		CATCHMENT		CONCENTRATION		CATCHMENT		PIPE CHARACTERISTICS					
				A (lin)	C	"AC"	"AC"	INLET	IN PIPE	TOTAL	"I" (min/hr)	"O" (m.v.s)	LENGTH (m)	DIAMETER (mm)	SLOPE (%)	CAPACITY (m ³ /s)	VELOCITY (m/s)
	54	MH POND	54	0.560	0.90	0.50	5.60	14.68	0.49	15.17	148.51	2.303	90.00	1000	1.00	2.398	3.05
				6.220							Peak Flow	2.303					
To 2BR - P3	64	65	64	0.320	0.90	0.29	0.29	5.00	0.59	5.59	214.24	0.171	90.00	450	2.00	0.403	2.54
	65	66	65	0.320	0.90	0.29	0.58	5.59	0.49	6.08	208.46	0.333	90.00	600	2.00	0.868	3.07
	66	67	66	0.320	0.90	0.29	0.86	6.08	0.40	6.48	203.93	0.488	90.00	800	2.00	1.870	3.72
	67	68	67	0.350	0.90	0.32	1.18	6.48	1.04	7.52	200.35	0.654	90.00	800	0.30	0.724	1.44
	68	69	68	0.280	0.90	0.25	1.43	7.52	0.90	8.42	191.69	0.760	90.00	1000	0.30	1.313	1.67
	69	70	69	1.090	0.90	0.98	2.41	8.42	0.90	9.32	184.85	1.235	90.00	1000	0.30	1.313	1.67
	70	71	70	0.310	0.90	0.28	2.69	9.32	0.79	10.11	178.51	1.331	90.00	1200	0.30	2.135	1.89
	71	72	71	0.890	0.90	0.80	3.49	10.11	0.79	10.91	173.26	1.676	90.00	1200	0.30	2.135	1.89
	72	73	72	0.330	0.90	0.30	3.79	10.91	0.79	11.70	168.34	1.767	90.00	1200	0.30	2.135	1.89
	73	74	73	0.310	0.90	0.28	4.07	11.70	0.79	12.50	163.70	1.845	90.00	1200	0.30	2.135	1.89
	74	75	74	0.320	0.90	0.29	4.36	12.50	0.79	13.29	159.33	1.923	90.00	1200	0.30	2.135	1.89
	75	76	75	0.310	0.90	0.28	4.64	13.29	0.79	14.09	155.21	1.993	90.00	1200	0.30	2.135	1.89
	76	77	76	0.310	0.90	0.28	4.91	14.09	0.79	14.88	151.30	2.059	90.00	1200	0.30	2.135	1.89
	77	78	77	0.310	0.90	0.28	5.19	14.88	0.79	15.67	147.60	2.123	90.00	1200	0.30	2.135	1.89
	78	79	78	0.310	0.90	0.28	5.19	14.88	0.79	15.67	147.60	2.123	1260.00				
	79	MH POND	79	0.460	0.90	0.41	7.80	15.67	0.49	16.16	144.10	3.115	90.00	1200	0.80	3.487	3.08
				8.670							Peak Flow	3.115					
To 2BR - P4	88	89	88	0.310	0.90	0.28	0.28	5.00	1.18	6.18	214.24	0.166	90.00	450	0.50	0.202	1.27
	89	90	89	0.320	0.90	0.29	0.57	6.18	0.59	6.78	203.00	0.319	90.00	450	2.00	0.403	2.54
	90	91	90	0.310	0.90	0.28	0.85	6.78	0.49	7.26	197.84	0.464	90.00	600	2.00	0.868	3.07
	91	92	91	0.330	0.90	0.30	1.14	7.26	0.49	7.75	193.79	0.614	90.00	600	2.00	0.868	3.07
	92	93	92	0.320	0.90	0.29	1.43	7.75	0.90	8.65	189.91	0.753	90.00	1000	0.30	1.313	1.67
	93	94	93	0.320	0.90	0.29	1.72	8.65	0.90	9.55	183.19	0.872	90.00	1000	0.30	1.313	1.67
	94	95	94	0.320	0.90	0.29	2.01	9.55	0.90	10.44	176.97	0.984	90.00	1000	0.30	1.313	1.67
	95	96	95	0.330	0.90	0.30	2.30	10.44	0.90	11.34	171.18	1.092	90.00	1000	0.30	1.313	1.67
	96	97	96	0.330	0.90	0.30	2.30	10.44	0.90	11.34	171.18	1.092	90.00	1000	0.30	1.313	1.67
	97	98	97	0.170	0.90	0.15	0.15	5.00	1.18	6.18	214.24	0.091	90.00	450	0.50	0.202	1.27
	98	99	98	0.320	0.90	0.29	0.41	6.18	0.99	7.16	203.00	0.248	90.00	600	0.50	0.434	1.54
	99	100	99	0.320	0.90	0.29	0.73	7.16	0.99	8.14	194.63	0.393	90.00	600	0.50	0.434	1.54
	100	101	100	0.320	0.90	0.29	1.02	8.14	0.81	9.12	186.65	0.527	90.00	800	0.50	0.935	1.86
	101	102	101	0.320	0.90	0.29	1.31	9.12	0.81	9.75	181.10	0.655	90.00	800	0.50	0.935	1.86
	102	103	102	0.320	0.90	0.29	1.59	9.75	0.81	10.56	175.62	0.775	90.00	800	0.50	0.935	1.86
	103	104	103	0.320	0.90	0.29	1.59	9.75	0.81	10.56	175.62	0.775	90.00	800	0.50	0.935	1.86
	104	105	104	0.320	0.90	0.29	1.59	9.75	0.81	10.56	175.62	0.775	90.00	800	0.50	0.935	1.86
	105	106	105	0.320	0.90	0.29	1.59	9.75	0.81	10.56	175.62	0.775	90.00	800	0.50	0.935	1.86

S ORM SEWER CALCULATIONS

FOR

Detroit River International Crossing Study

Alternative 2B - Revised Profiles

Date: 11/16/06
File : 33015384

Designed by Kevin Chen

LOCATION	FROM MH	TO MH	CATCHMENT AREA NUMBERS	CATCHMENT CHARACTERISTICS			CONCENTRATION TIME (min)		CATCHMENT RUNOFF		PIPE CHARACTERISTICS						
				A (ha)	C	"AC"	ACCUM. "AC"	INLET	IN PIPE	TOTAL	"I" (mm/hr)	"Q" (m ³ /s)	LENGTH (m)	DIAMETER (mm)	SLOPE (%)	CAPACITY (m ³ /s)	VELOCITY (m/s)
	99	98	99	0.320	0.90	0.29	1.88	10.56	0.40	10.96	170.48	0.888	90.00	800	2.00	1.870	3.72
	98	97	98	0.320	0.90	0.29	2.17	11.00	0.40	11.40	167.76	1.008	90.00	800	2.00	1.870	3.72
	97	96	97	0.310	0.90	0.28	2.45	11.40	0.90	12.30	165.41	1.122	90.00	1000	0.30	1.313	1.67
	96	MH POND	96	1.080	0.90	0.97	5.72	12.30	0.62	12.92	160.39	2.543	810.00	1200	0.50	2.757	2.44
				Drainage Area		6.360		Peak Flow		2.543							

DESIGN PARAMETERS:

$I = a/((t+b)^c)$
 100 Year Storm Parameters
 a = 2824.51
 b = 13.7400
 c = 0.8800
 manning's n = 0.0130
 Min. veloc 0.8000 m/s
 Max. veloc 6.0000 m/s

Project: **Detroit River International Crossing Study**

Design Storm: **100-year**

Rainfall Data: **MTO District I Chatham**

Rainfall Intensity Coefficients:

A: **2824.505**

B: **13.74**

C: **0.88**

Inlet Time (t_i)= **5** min

Average Velocity = **1.5** m/s

**Rational Method Calculation
Proposed Condition
Alternative 3**

Drainage ID	Drainage Area (ha)	Run-off Coefficient C	Travelled Distance d (m)	Time of Concentration $T_c = t_i + t_d$ (hr)	Rainfall Intensity I (mm/h)	100-year Peak Flow (m ³ /s)	Remarks
100	6.36	0.90	850	0.24	149.6	2.40	To Ttcombe Drain
101	2.80	0.90	798	0.23	152.4	1.08	Pump to Basin Drain
102	0.34	0.90	342	0.15	182.1	0.16	Pump to Turkey Creek
103	0.34	0.90	342	0.15	182.1	0.16	Pump to Turkey Creek
104	0.14	0.90	140	0.11	199.7	0.07	Pump to Cahill Drain
105	0.19	0.90	194	0.12	194.7	0.10	Pump to Cahill Drain
106	0.17	0.90	174	0.12	196.5	0.09	Pump to Cahill Drain
107	0.19	0.90	191	0.12	194.9	0.09	Pump to Cahill Drain
108	2.16	0.90	774	0.23	153.7	0.84	Pump to Wolfe Drain
109	6.56	0.90	1737	0.41	114.9	1.90	To Wolfe Drain

Appendix C

Stormwater Management Computations

Appendix C.1

Alternative 1A

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006

9:58 AM

	100	
Area =	6.36	ha
"C" =	0.9	
AC =	5.724	
Tc =	14.4	min
Time Increment =	10.0	min
Release Rate =	367.1	l/s
Max.Storage =	2376	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

100					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
14.4	149.6	2380.56	2063.1	318.1	1745.0
24.4	114.5	1822.33	2672.7	538.4	2134.3
34.4	93.3	1485.01	3069.0	758.7	2310.4
44.4	79.1	1257.93	3354.5	978.9	2375.6
54.4	68.8	1094.07	3574.0	1199.2	2374.8
64.4	61.0	969.93	3750.4	1419.4	2331.0
74.4	54.8	872.45	3897.0	1639.7	2257.3
84.4	49.9	793.76	4021.7	1859.9	2161.8
94.4	45.8	728.82	4130.0	2080.2	2049.8
104.4	42.4	674.27	4225.4	2300.5	1925.0
114.4	39.5	627.76	4310.6	2520.7	1789.9
124.4	36.9	587.60	4387.4	2741.0	1646.5
134.4	34.7	552.56	4457.3	2961.2	1496.1
144.4	32.8	521.70	4521.4	3181.5	1339.9
154.4	31.1	494.31	4580.6	3401.7	1178.8
164.4	29.5	469.81	4635.4	3622.0	1013.5
174.4	28.1	447.77	4686.6	3842.2	844.4
184.4	26.9	427.82	4734.6	4062.5	672.1
194.4	25.7	409.69	4779.7	4282.8	496.9
204.4	24.7	393.12	4822.2	4503.0	319.2
214.4	23.7	377.92	4862.5	4723.3	139.3
224.4	22.9	363.92	4900.8	4943.5	-42.8
234.4	22.1	350.98	4937.2	5163.8	-226.6
244.4	21.3	338.99	4971.9	5384.0	-412.2

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006 9:58 AM

	101	
Area =	2.53	ha
"C" =	0.9	
AC =	2.277	
Tc =	13.1	min
Time Increment =	10.0	min
Release Rate =	156.9	l/s
Max.Storage =	915	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

101					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m ³)	Released Volume (m ³)	Storage Volume (m ³)
13.1	155.9	987.16	778.5	123.8	654.8
23.1	118.1	747.36	1037.8	217.9	819.9
33.1	95.6	605.12	1203.4	312.1	891.3
43.1	80.6	510.45	1321.4	406.3	915.1 <<<<
53.1	69.9	442.65	1411.5	500.4	911.0
63.1	61.9	391.57	1483.5	594.6	888.9
73.1	55.5	351.63	1543.2	688.8	854.4
83.1	50.5	319.48	1593.8	783.0	810.8
93.1	46.3	293.03	1637.6	877.1	760.5
103.1	42.8	270.85	1676.2	971.3	704.9
113.1	39.8	251.97	1710.5	1065.5	645.1
123.1	37.2	235.70	1741.5	1159.6	581.9
133.1	35.0	221.52	1769.6	1253.8	515.9
143.1	33.0	209.05	1795.4	1348.0	447.5
153.1	31.3	197.98	1819.2	1442.1	377.1
163.1	29.7	188.10	1841.2	1536.3	304.9
173.1	28.3	179.21	1861.8	1630.5	231.3
183.1	27.0	171.18	1881.0	1724.6	156.4
193.1	25.9	163.87	1899.1	1818.8	80.3
203.1	24.8	157.21	1916.1	1913.0	3.2
213.1	23.9	151.09	1932.3	2007.1	-74.9
223.1	23.0	145.46	1947.6	2101.3	-153.7
233.1	22.2	140.27	1962.1	2195.5	-233.3
243.1	21.4	135.45	1976.0	2289.6	-313.6

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006 9:58 AM

Area =	102	
"C" =	5.60	ha
AC =	0.9	
Tc =	5.04	
Time Increment =	14.3	min
Release Rate =	10.0	min
Max.Storage =	675.0	l/s
	1363	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

102					
Time	Rainfall	Storm	Runoff	Released	Storage
(min)	Intensity	Runoff	Volume	Volume	Volume
	(mm/hr)	(l/s)	(m ³)	(m ³)	(m ³)
14.3	150.5	2108.53	1803.5	577.4	1226.1
24.3	115.0	1611.58	2345.4	982.4	1363.0
34.3	93.6	1312.08	2696.8	1387.4	1309.3
44.3	79.3	1110.79	2949.5	1792.4	1157.1
54.3	68.9	965.68	3143.6	2197.5	946.2
64.3	61.1	855.85	3299.6	2602.5	697.1
74.3	54.9	769.65	3429.0	3007.5	421.5
84.3	50.0	700.09	3539.2	3412.5	126.7
94.3	45.9	642.72	3634.8	3817.6	-182.8
104.3	42.4	594.53	3719.0	4222.6	-503.6
114.3	39.5	553.46	3794.2	4627.6	-833.4
124.3	37.0	518.01	3861.9	5032.6	-1170.7
134.3	34.8	487.08	3923.6	5437.6	-1514.1
144.3	32.8	459.84	3980.1	5842.7	-1862.6
154.3	31.1	435.67	4032.3	6247.7	-2215.4
164.3	29.6	414.05	4080.6	6652.7	-2572.1
174.3	28.2	394.61	4125.8	7057.7	-2931.9
184.3	26.9	377.02	4168.0	7462.7	-3294.7
194.3	25.8	361.02	4207.8	7867.8	-3660.0
204.3	24.7	346.40	4245.3	8272.8	-4027.5
214.3	23.8	333.00	4280.8	8677.8	-4397.0
224.3	22.9	320.65	4314.5	9082.8	-4768.3
234.3	22.1	309.25	4346.6	9487.9	-5141.3
244.3	21.3	298.68	4377.2	9892.9	-5515.7

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006

9:58 AM

	103	
Area =	2.60	ha
"C" =	0.9	
AC =	2.34	
Tc =	13.3	min
Time Increment =	10.0	min
Release Rate =	159.9	l/s
Max.Storage =	944	m ³

Controlled Condition

100 Year - Post Dev't.	
a=	2825
b=	13.74
c=	0.880

Constant Inflows

	l/s

103					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m ³)	Released Volume (m ³)	Storage Volume (m ³)
13.3	155.2	1009.34	805.5	127.6	677.8
23.3	117.6	765.20	1069.7	223.6	846.2
33.3	95.3	620.06	1238.9	319.5	919.3
43.3	80.4	523.32	1359.6	415.5	944.1
53.3	69.8	453.97	1451.8	511.4	940.4
63.3	61.7	401.69	1525.6	607.4	918.2
73.3	55.5	360.79	1586.7	703.3	883.4
83.3	50.4	327.86	1638.6	799.3	839.3
93.3	46.2	300.75	1683.6	895.3	788.3
103.3	42.7	278.02	1723.1	991.2	731.9
113.3	39.8	258.66	1758.4	1087.2	671.2
123.3	37.2	241.98	1790.2	1183.1	607.0
133.3	35.0	227.44	1819.0	1279.1	540.0
143.3	33.0	214.64	1845.5	1375.0	470.5
153.3	31.3	203.29	1869.9	1471.0	398.9
163.3	29.7	193.15	1892.5	1566.9	325.6
173.3	28.3	184.04	1913.6	1662.9	250.7
183.3	27.0	175.79	1933.3	1758.9	174.5
193.3	25.9	168.30	1951.9	1854.8	97.1
203.3	24.8	161.45	1969.4	1950.8	18.7
213.3	23.9	155.18	1986.0	2046.7	-60.7
223.3	23.0	149.40	2001.7	2142.7	-141.0
233.3	22.1	144.07	2016.7	2238.6	-222.0
243.3	21.4	139.12	2030.9	2334.6	-303.6

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006

9:58 AM

	104	
Area =	2.50	ha
"C" =	0.9	
AC =	2.25	
Tc =	8.9	min
Time Increment =	10.0	min
Release Rate =	207.9	l/s
Max.Storage =	775	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

104					
Time	Rainfall	Storm	Runoff	Released	Storage
(min)	Intensity	Runoff	Volume	Volume	Volume
	(mm/hr)	(l/s)	(m ³)	(m ³)	(m ³)
8.9	181.2	1133.71	606.9	111.3	495.6
18.9	131.4	821.88	933.1	236.1	697.0
28.9	103.9	649.73	1127.5	360.8	766.7
38.9	86.3	539.82	1260.7	485.6	775.1
48.9	74.1	463.24	1359.8	610.3	749.4
58.9	65.0	406.65	1437.6	735.1	702.6
68.9	58.0	363.03	1501.2	859.8	641.4
78.9	52.5	328.32	1554.7	984.6	570.1
88.9	48.0	300.00	1600.6	1109.3	491.3
98.9	44.2	276.44	1640.8	1234.1	406.7
108.9	41.0	256.51	1676.4	1358.9	317.5
118.9	38.3	239.42	1708.3	1483.6	224.7
128.9	35.9	224.58	1737.2	1608.4	128.9
138.9	33.8	211.59	1763.6	1733.1	30.5
148.9	32.0	200.10	1787.9	1857.9	-69.9
158.9	30.4	189.86	1810.4	1982.6	-172.2
168.9	28.9	180.68	1831.3	2107.4	-276.1
178.9	27.6	172.40	1850.8	2232.1	-381.3
188.9	26.4	164.90	1869.1	2356.9	-487.7
198.9	25.3	158.05	1886.4	2481.6	-595.2
208.9	24.3	151.79	1902.7	2606.4	-703.7
218.9	23.3	146.03	1918.2	2731.1	-813.0
228.9	22.5	140.72	1932.9	2855.9	-923.0
238.9	21.7	135.81	1946.9	2980.7	-1033.8

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006 9:58 AM

	105	
Area =	2.50	ha
"C" =	0.9	
AC =	2.25	
Tc =	10.9	min
Time Increment =	10.0	min
Release Rate =	177.6	l/s
Max.Storage =	847	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

105					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
10.9	168.2	1052.40	689.7	116.4	573.3
20.9	124.7	780.00	979.2	222.9	756.3
30.9	99.8	624.05	1157.8	329.4	828.4
40.9	83.5	522.40	1282.7	436.0	846.7
50.9	72.0	450.61	1376.8	542.5	834.3
60.9	63.5	397.05	1451.3	649.0	802.3
70.9	56.8	355.47	1512.6	755.6	757.1
80.9	51.5	322.21	1564.4	862.1	702.3
90.9	47.2	294.95	1609.1	968.6	640.4
100.9	43.5	272.19	1648.2	1075.2	573.1
110.9	40.4	252.88	1683.0	1181.7	501.3
120.9	37.8	236.28	1714.3	1288.2	426.1
130.9	35.5	221.85	1742.7	1394.8	347.9
140.9	33.4	209.18	1768.7	1501.3	267.3
150.9	31.6	197.96	1792.6	1607.8	184.7
160.9	30.0	187.95	1814.7	1714.4	100.3
170.9	28.6	178.96	1835.3	1820.9	14.4
180.9	27.3	170.84	1854.6	1927.4	-72.9
190.9	26.1	163.48	1872.7	2034.0	-161.3
200.9	25.1	156.76	1889.7	2140.5	-250.8
210.9	24.1	150.60	1905.9	2247.0	-341.2
220.9	23.2	144.94	1921.2	2353.6	-432.4
230.9	22.3	139.71	1935.7	2460.1	-524.4
240.9	21.6	134.87	1949.6	2566.6	-617.0

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006 9:58 AM

	106	
Area =	5.60	ha
"C" =	0.9	
AC =	5.04	
Tc =	13.5	min
Time Increment =	10.0	min
Release Rate =	338.5	l/s
Max.Storage =	2049	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

106					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m ³)	Released Volume (m ³)	Storage Volume (m ³)
13.5	154.0	2158.36	1751.1	274.6	1476.5
23.5	117.0	1639.46	2313.8	477.7	1836.1
33.5	94.9	1329.98	2675.0	680.8	1994.2
43.5	80.2	1123.29	2933.3	883.9	2049.4
53.5	69.6	974.94	3130.9	1087.0	2043.8
63.5	61.6	862.99	3289.1	1290.1	1999.0
73.5	55.3	775.34	3420.3	1493.2	1927.0
83.5	50.3	704.74	3531.7	1696.3	1835.3
93.5	46.1	646.58	3628.2	1899.4	1728.8
103.5	42.7	597.80	3713.2	2102.5	1610.6
113.5	39.7	556.27	3788.9	2305.6	1483.3
123.5	37.1	520.44	3857.2	2508.7	1348.4
133.5	34.9	489.21	3919.2	2711.8	1207.4
143.5	33.0	461.73	3976.1	2914.9	1061.2
153.5	31.2	437.35	4028.6	3118.0	910.5
163.5	29.7	415.56	4077.2	3321.1	756.1
173.5	28.3	395.97	4122.6	3524.2	598.3
183.5	27.0	378.25	4165.0	3727.3	437.7
193.5	25.8	362.14	4205.0	3930.4	274.5
203.5	24.8	347.43	4242.6	4133.5	109.1
213.5	23.8	333.94	4278.3	4336.6	-58.4
223.5	22.9	321.53	4312.1	4539.7	-227.6
233.5	22.1	310.06	4344.3	4742.8	-398.6
243.5	21.4	299.42	4375.0	4945.9	-571.0

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006

9:58 AM

	107	
Area =	3.10	ba
"C" =	0.9	
AC =	2.79	
Tc =	14.8	min
Time Increment =	10.0	min
Release Rate =	176.9	l/s
Max.Storage =	1164	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows	
	l/s

107					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m ³)	Released Volume (m ³)	Storage Volume (m ³)
14.8	148.0	1148.00	1018.7	157.0	861.7
24.8	113.6	881.25	1310.7	263.1	1047.6
34.8	92.7	719.30	1501.4	369.2	1132.2
44.8	78.6	609.97	1639.2	475.3	1163.8
54.8	68.5	530.91	1745.3	581.5	1163.8
64.8	60.7	470.94	1830.7	687.6	1143.1
74.8	54.6	423.80	1901.7	793.7	1108.0
84.8	49.7	385.71	1962.2	899.9	1062.3
94.8	45.7	354.25	2014.7	1006.0	1008.7
104.8	42.3	327.81	2061.1	1112.1	948.9
114.8	39.4	305.26	2102.4	1218.3	884.2
124.8	36.8	285.78	2139.7	1324.4	815.4
134.8	34.7	268.78	2173.7	1430.5	743.2
144.8	32.7	253.80	2204.9	1536.7	668.2
154.8	31.0	240.50	2233.6	1642.8	590.8
164.8	29.5	228.61	2260.3	1748.9	511.4
174.8	28.1	217.90	2285.2	1855.1	430.1
184.8	26.8	208.21	2308.5	1961.2	347.3
194.8	25.7	199.40	2330.5	2067.3	263.1
204.8	24.7	191.35	2351.2	2173.4	177.7
214.8	23.7	183.96	2370.8	2279.6	91.2
224.8	22.8	177.16	2389.4	2385.7	3.6
234.8	22.0	170.87	2407.1	2491.8	-84.8
244.8	21.3	165.04	2424.0	2598.0	-174.0

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Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006 9:58 AM

	108	
Area =	3.96	ha
"C" =	0.9	
AC =	3.564	
Tc =	14.3	min
Time Increment =	10.0	min
Release Rate =	231.4	l/s
Max.Storage =	1471	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

108					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
14.3	150.4	1490.51	1275.9	198.1	1077.8
24.3	115.0	1139.33	1658.9	336.9	1321.9
34.3	93.6	927.64	1907.2	475.8	1431.5
44.3	79.3	785.35	2085.9	614.6	1471.3
54.3	68.9	682.78	2223.1	753.4	1469.7
64.3	61.1	605.13	2333.4	892.3	1441.1
74.3	54.9	544.19	2424.9	1031.1	1393.8
84.3	50.0	495.02	2502.8	1169.9	1332.9
94.3	45.9	454.45	2570.4	1308.8	1261.6
104.3	42.4	420.38	2629.9	1447.6	1182.3
114.3	39.5	391.35	2683.1	1586.5	1096.6
124.3	37.0	366.28	2731.0	1725.3	1005.7
134.3	34.8	344.41	2774.6	1864.1	910.4
144.3	32.8	325.16	2814.5	2003.0	811.6
154.3	31.1	308.06	2851.4	2141.8	709.6
164.3	29.6	292.78	2885.6	2280.7	605.0
174.3	28.2	279.03	2917.5	2419.5	498.1
184.3	26.9	266.59	2947.4	2558.3	389.1
194.3	25.8	255.28	2975.5	2697.2	278.4
204.3	24.7	244.95	3002.1	2836.0	166.1
214.3	23.8	235.47	3027.2	2974.8	52.3
224.3	22.9	226.74	3051.0	3113.7	-62.7
234.3	22.1	218.67	3073.7	3252.5	-178.8
244.3	21.3	211.20	3095.3	3391.4	-296.0

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1A

November 14, 2006

9:58 AM

	109	
Area =	6.60	ha
"C" =	0.9	
AC =	5.94	
Tc =	24.3	min
Time Increment =	10.0	min
Release Rate =	270.3	l/s
Max.Storage =	2847	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

109					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
24.3	114.9	1897.41	2766.4	394.1	2372.3
34.3	93.6	1545.12	3179.9	556.3	2623.6
44.3	79.2	1308.26	3477.4	718.5	2758.9
54.3	68.9	1137.47	3705.9	880.6	2825.3
64.3	61.1	1008.17	3889.5	1042.8	2846.7
74.3	54.9	906.68	4042.0	1205.0	2837.0
84.3	49.9	824.78	4171.7	1367.2	2804.6
94.3	45.9	757.21	4284.3	1529.3	2755.0
104.3	42.4	700.47	4383.5	1691.5	2692.0
114.3	39.5	652.09	4472.1	1853.7	2618.3
124.3	37.0	610.34	4551.9	2015.9	2536.0
134.3	34.8	573.90	4624.5	2178.1	2446.5
144.3	32.8	541.82	4691.1	2340.2	2350.9
154.3	31.1	513.35	4752.6	2502.4	2250.1
164.3	29.5	487.89	4809.6	2664.6	2145.0
174.3	28.2	464.98	4862.8	2826.8	2036.0
184.3	26.9	444.25	4912.6	2989.0	1923.6
194.3	25.8	425.41	4959.4	3151.1	1808.3
204.3	24.7	408.19	5003.6	3313.3	1690.3
214.3	23.8	392.40	5045.4	3475.5	1569.9
224.3	22.9	377.85	5085.1	3637.7	1447.5
234.3	22.1	364.41	5122.9	3799.9	1323.1
244.3	21.3	351.96	5159.0	3962.0	1196.9
254.3	20.6	340.38	5193.4	4124.2	1069.2

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Stormwater Management Pond Area Requirement Calculation Sheet Alternative 1A

Stormwater Management Facility No.	1A-P1	1A-P2	1A-P3	1A-P4	1A-P5	1A-P6	1A-P7	1A-P8	1A-P9	1A-P9
Drainage ID	100	101	102	103	104	105	106	107	108	109
Drainage Area (ha)	6.4	2.5	5.6	2.6	2.5	2.5	5.6	3.1	4.0	6.6
Imperviousness of Drainage Area	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Runoff Volume ¹ (25mm Storm) (mm)	25.00	25.00	25.00	25.00	25.00	25.00	25.00	25.00	25.00	25.00
Permanent Pool Volume Required ² (m ³)	1,336	531	1,176	546	525	525	1,176	651	832	1,386
Extended Detention Volume ³ (m ³)	254	101	224	104	100	100	224	124	158	264
Erosion Control Volume ⁴ , 25mm Storm (m ³)	1,590	633	1,400	650	625	625	1,400	775	990	1,650
Total Extended Detention Vol. Req ¹⁵ (m ³)	1,590	633	1,400	650	625	625	1,400	775	990	1,650
Assumed Permanent Pool Depth	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Assumed Quantity Storage Depth	1.80	1.80	1.80	1.80	1.80	1.80	1.80	1.80	1.80	1.80
Designed Slope	V:1 H:	5:00	5:00	5:00	5:00	5:00	5:00	5:00	5:00	5:00
Trial and Error Method Assuming square lot										
Assumed Bottom width	25.00	15.25	22.25	12.00	11.50	11.50	22.25	14.00	17.25	25.00
Volume	1668.75	860.72	1411.97	654.75	625.88	625.88	1411.97	777.75	1003.22	1668.75
Pond Bottom Area Requirement (m ²)	625	233	495	144	132	132	495	196	298	625
Pond Bottom width requirement assuming a square lot	25.00	15.25	22.25	12.00	11.50	11.50	22.25	14.00	17.25	25.00
Pond Area at Normal Water Level	1,600	915	1,388	729	702	702	1,388	841	1,040	1,600
Pond Area Requirement (m ²)	3,364	2,328	3,053	2,025	1,980	1,980	3,053	2,209	2,525	3,364
Pond Area Requirement with 10 m buffer (m ²)	6,084	4,658	5,663	4,225	4,160	4,160	5,663	4,489	4,935	6,084
Total Quantity Control Volume @ 1.80m depth from NWL =	4,468	2,919	3,996	2,479	2,414	2,414	3,996	2,745	3,209	4,468
Total Quantity and Quantity Control Volume =	6,136	3,780	5,408	3,133	3,040	3,040	5,408	3,523	4,212	6,136
Approximate Volume of Excavation =	17,779	12,960	16,318	11,578	11,375	11,375	16,318	12,415	13,866	17,779

Notes:

- ¹ Based on Imperviousness for the site (25mm * Imperviousness)
- ² Calculated using Table 3.2 of the MOE 2003 SWM Planning and Design Manual, 85% Imperviousness (250m³/ha - 40m³/ha)
- ³ Based on 40m³/ha
- ⁴ Area x Runoff Volume
- ⁵ Greater of Extended Detention or Erosion Control
- ⁶ Average release over 24 hours.

Appendix C.2

Alternative 1B

Modified Rational Method

Project Name : **Detroit River International Crossing** November 14, 2006 10:30 AM

Stormwater Management Study Alternative 1B

	100			
Area =	6.36	ha		
"C" =	0.9			
AC =	5.724			
Tc =	14.8	min		
Time Increment =	10.0	min		
Release Rate =	360.5	l/s		
Max.Storage =	2395	m ³		

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

100					
Time	Rainfall Intensity	Storm Runoff	Runoff Volume	Released Volume	Storage Volume
(min)	(mm/hr)	(l/s)	(m3)	(m ³)	(m ³)
14.8	147.8	2352.02	2093.3	320.9	1772.4
24.8	113.5	1806.15	2691.2	537.2	2154.0
34.8	92.7	1474.54	3081.8	753.5	2328.3
44.8	78.6	1250.58	3364.1	969.8	2394.2
54.8	68.4	1088.60	3581.5	1186.2	2395.4
64.8	60.7	965.71	3756.6	1402.5	2354.1
74.8	54.6	869.08	3902.2	1618.8	2283.4
84.8	49.7	791.00	4026.2	1835.1	2191.1
94.8	45.7	726.52	4133.9	2051.4	2082.5
104.8	42.3	672.32	4228.9	2267.7	1961.2
114.8	39.3	626.09	4313.7	2484.1	1829.7
124.8	36.8	586.15	4390.3	2700.4	1689.9
134.8	34.6	551.29	4459.9	2916.7	1543.2
144.8	32.7	520.58	4523.8	3133.0	1390.8
154.8	31.0	493.30	4582.8	3349.3	1233.4
164.8	29.5	468.91	4637.5	3565.7	1071.8
174.8	28.1	446.96	4688.6	3782.0	906.6
184.8	26.8	427.09	4736.4	3998.3	738.1
194.8	25.7	409.01	4781.4	4214.6	566.8
204.8	24.7	392.50	4823.8	4430.9	392.9
214.8	23.7	377.35	4864.0	4647.3	216.8
224.8	22.8	363.40	4902.2	4863.6	38.6
234.8	22.0	350.50	4938.5	5079.9	-141.4
244.8	21.3	338.54	4973.2	5296.2	-323.0

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing** November 14, 2006 10:30 AM

Stormwater Management Study Alternative 1B

	101	
Area =	2.70	ha
"C" =	0.9	
AC =	2.43	
Tc =	10.2	min
Time Increment =	10.0	min
Release Rate =	201.9	l/s
Max.Storage =	890	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

101					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
10.2	172.7	1166.71	714.0	123.6	590.5
20.2	127.0	858.16	1040.1	244.7	795.4
30.2	101.2	683.72	1238.9	365.8	873.1
40.2	84.5	570.84	1376.9	486.9	889.9
50.2	72.8	491.49	1480.4	608.1	872.3
60.2	64.0	432.49	1562.2	729.2	833.0
70.2	57.3	386.81	1629.3	850.3	778.9
80.2	51.9	350.34	1685.8	971.5	714.4
90.2	47.4	320.50	1734.5	1092.6	641.9
100.2	43.8	295.61	1777.2	1213.7	563.5
110.2	40.6	274.52	1815.1	1334.9	480.2
120.2	38.0	256.40	1849.1	1456.0	393.1
130.2	35.6	240.65	1880.0	1577.1	302.9
140.2	33.6	226.84	1908.2	1698.2	209.9
150.2	31.8	214.62	1934.2	1819.4	114.8
160.2	30.2	203.72	1958.2	1940.5	17.7
170.2	28.7	193.94	1980.6	2061.6	-81.1
180.2	27.4	185.12	2001.5	2182.8	-181.3
190.2	26.2	177.11	2021.1	2303.9	-282.8
200.2	25.1	169.80	2039.6	2425.0	-385.4
210.2	24.1	163.11	2057.1	2546.2	-489.0
220.2	23.2	156.96	2073.7	2667.3	-593.6
230.2	22.4	151.28	2089.5	2788.4	-698.9
240.2	21.6	146.03	2104.5	2909.6	-805.0

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing** November 14, 2006 10:30 AM
Stormwater Management Study
Alternative 1B

	102	
Area =	5.38	ha
"C" =	0.9	
AC =	4.842	
Tc =	12.4	min
Time Increment =	10.0	min
Release Rate =	344.6	l/s
Max.Storage =	1917	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

102					
Time	Rainfall	Storm	Runoff	Released	Storage
(min)	Intensity (mm/hr)	Runoff (l/s)	Volume (m ³)	Volume (m ³)	Volume (m ³)
12.4	159.6	2148.49	1604.2	257.3	1346.9
22.4	120.1	1616.27	2176.6	464.1	1712.5
32.4	96.9	1303.93	2538.3	670.9	1867.5
42.4	81.5	1097.36	2794.6	877.6	1917.0
52.4	70.6	950.05	2989.5	1084.4	1905.1
62.4	62.4	839.40	3145.0	1291.2	1853.8
72.4	55.9	753.07	3273.3	1498.0	1775.4
82.4	50.8	683.72	3382.1	1704.7	1677.4
92.4	46.6	626.73	3476.2	1911.5	1564.7
102.4	43.0	579.00	3558.9	2118.3	1440.7
112.4	40.0	538.43	3632.6	2325.1	1307.5
122.4	37.4	503.48	3698.9	2531.8	1167.1
132.4	35.1	473.04	3759.1	2738.6	1020.5
142.4	33.2	446.28	3814.2	2945.4	868.9
152.4	31.4	422.56	3865.1	3152.1	712.9
162.4	29.8	401.38	3912.2	3358.9	553.2
172.4	28.4	382.35	3956.1	3565.7	390.4
182.4	27.1	365.15	3997.1	3772.5	224.7
192.4	26.0	349.52	4035.7	3979.2	56.5
202.4	24.9	335.25	4072.1	4186.0	-113.9
212.4	23.9	322.17	4106.6	4392.8	-286.2
222.4	23.0	310.14	4139.3	4599.6	-460.3
232.4	22.2	299.02	4170.4	4806.3	-636.0
242.4	21.4	288.73	4200.0	5013.1	-813.1

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1B

November 14, 2006

10:30 AM

	103	
Area =	4.50	ha
"C" =	0.9	
AC =	4.05	
Tc =	14.4	min
Time Increment =	10.0	min
Release Rate =	260.9	l/s
Max.Storage =	1678	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

103					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
14.4	149.8	1686.11	1457.9	225.6	1232.3
24.4	114.6	1290.37	1890.0	382.2	1507.8
34.4	93.4	1051.35	2170.7	538.7	1632.0
44.4	79.1	890.49	2372.9	695.2	1677.6
54.4	68.8	774.44	2528.3	851.8	1676.5
64.4	61.0	686.53	2653.2	1008.3	1644.9
74.4	54.8	617.51	2757.0	1164.9	1592.1
84.4	49.9	561.79	2845.3	1321.4	1523.8
94.4	45.8	515.82	2921.9	1478.0	1443.9
104.4	42.4	477.20	2989.5	1634.5	1354.9
114.4	39.5	444.27	3049.8	1791.1	1258.7
124.4	36.9	415.84	3104.1	1947.6	1156.5
134.4	34.7	391.04	3153.6	2104.2	1049.4
144.4	32.8	369.20	3199.0	2260.7	938.2
154.4	31.1	349.81	3240.8	2417.3	823.5
164.4	29.5	332.47	3279.7	2573.8	705.8
174.4	28.1	316.87	3315.9	2730.4	585.5
184.4	26.9	302.75	3349.8	2886.9	462.9
194.4	25.7	289.91	3381.7	3043.5	338.3
204.4	24.7	278.19	3411.9	3200.0	211.8
214.4	23.8	267.43	3440.4	3356.6	83.8
224.4	22.9	257.52	3467.4	3513.1	-45.7
234.4	22.1	248.37	3493.2	3669.7	-176.5
244.4	21.3	239.88	3517.8	3826.2	-308.5

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1B

November 14, 2006

10:30 AM

	104	
Area =	2.74	ha
"C" =	0.9	
AC =	2.466	
Tc =	10.0	min
Time Increment =	10.0	min
Release Rate =	207.7	l/s
Max.Storage =	896	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

104					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
10.0	173.9	1192.28	716.2	124.8	591.4
20.0	127.7	875.16	1050.8	249.4	801.4
30.0	101.6	696.48	1254.1	374.0	880.1
40.0	84.8	581.08	1395.0	498.6	896.4
50.0	72.9	500.07	1500.5	623.2	877.3
60.0	64.2	439.89	1583.9	747.9	836.0
70.0	57.4	393.32	1652.2	872.5	779.7
80.0	52.0	356.16	1709.8	997.1	712.7
90.0	47.5	325.77	1759.4	1121.7	637.6
100.0	43.8	300.43	1802.8	1246.3	556.4
110.0	40.7	278.96	1841.3	1371.0	470.3
120.0	38.0	260.52	1875.9	1495.6	380.3
130.0	35.7	244.50	1907.3	1620.2	287.1
140.0	33.6	230.45	1935.9	1744.8	191.1
150.0	31.8	218.02	1962.3	1869.4	92.9
160.0	30.2	206.94	1986.8	1994.1	-7.3
170.0	28.7	197.00	2009.5	2118.7	-109.2
180.0	27.4	188.02	2030.7	2243.3	-212.6
190.0	26.2	179.88	2050.7	2367.9	-317.2
200.0	25.2	172.45	2069.5	2492.5	-423.0
210.0	24.2	165.65	2087.3	2617.2	-529.9
220.0	23.3	159.40	2104.1	2741.8	-637.7
230.0	22.4	153.63	2120.2	2866.4	-746.2
240.0	21.6	148.29	2135.4	2991.0	-855.6

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing** November 14, 2006 10:30 AM
Stormwater Management Study
Alternative 1B

	105	
Area =	7.21	ha
"C" =	0.9	
AC =	6.489	
Tc =	16.3	min
Time Increment =	10.0	min
Release Rate =	383.5	l/s
Max. Storage =	2796	m ³

Controlled Condition	
100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows	
	l/s

105					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
16.3	141.5	2552.29	2494.4	374.8	2119.6
26.3	109.9	1981.87	3126.1	605.0	2521.1
36.3	90.3	1628.74	3546.3	835.1	2711.2
46.3	76.9	1387.42	3853.3	1065.2	2788.1
56.3	67.2	1211.49	4091.6	1295.3	2796.3
66.3	59.7	1077.23	4284.5	1525.5	2759.0
76.3	53.8	971.20	4445.5	1755.6	2689.9
86.3	49.1	885.23	4583.1	1985.7	2597.4
96.3	45.1	814.03	4702.9	2215.8	2487.1
106.3	41.8	754.04	4808.8	2446.0	2362.8
116.3	39.0	702.76	4903.4	2676.1	2227.3
126.3	36.5	658.41	4989.0	2906.2	2082.8
136.3	34.3	619.63	5066.9	3136.3	1930.6
146.3	32.5	585.42	5138.5	3366.4	1772.0
156.3	30.8	555.02	5204.6	3596.6	1608.0
166.3	29.3	527.79	5266.0	3826.7	1439.3
176.3	27.9	503.27	5323.3	4056.8	1266.5
186.3	26.7	481.06	5377.0	4286.9	1090.1
196.3	25.5	460.85	5427.6	4517.1	910.5
206.3	24.5	442.37	5475.3	4747.2	728.1
216.3	23.6	425.40	5520.5	4977.3	543.2
226.3	22.7	409.76	5563.5	5207.4	356.1
236.3	21.9	395.31	5604.4	5437.6	166.8
246.3	21.2	381.90	5643.4	5667.7	-24.3

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Modified Rational Method

Project Name : **Detroit River International Crossing**

November 14, 2006

10:30 AM

Stormwater Management Study

Alternative 1B

	106	
Area =	6.17	ha
"C" =	0.9	
AC =	5.553	
Tc =	21.6	min
Time Increment =	10.0	min
Release Rate =	272.5	l/s
Max.Storage =	2586	m ³

Controlled Condition

100 Year - Post Dev't.	
a=	2825
b=	13.74
c=	0.880

Constant Inflows

	l/s

106					
Time	Rainfall Intensity	Storm Runoff	Runoff Volume	Released Volume	Storage Volume
(min)	(mm/hr)	(l/s)	(m3)	(m ³)	(m ³)
21.6	122.5	1890.43	2455.0	353.8	2101.2
31.6	98.4	1518.58	2883.3	517.3	2365.9
41.6	82.6	1274.48	3184.5	680.8	2503.7
51.6	71.3	1101.28	3412.5	844.3	2568.2
61.6	62.9	971.64	3593.8	1007.8	2586.0
71.6	56.4	870.77	3743.1	1171.3	2571.9
81.6	51.2	789.90	3869.5	1334.7	2534.7
91.6	46.9	723.55	3978.6	1498.2	2480.4
101.6	43.3	668.08	4074.4	1661.7	2412.7
111.6	40.2	620.96	4159.6	1825.2	2334.4
121.6	37.6	580.41	4236.2	1988.7	2247.5
131.6	35.3	545.13	4305.8	2152.2	2153.6
141.6	33.3	514.13	4369.5	2315.6	2053.8
151.6	31.5	486.68	4428.1	2479.1	1949.0
161.6	29.9	462.17	4482.4	2642.6	1839.8
171.6	28.5	440.16	4533.1	2806.1	1727.0
181.6	27.2	420.27	4580.4	2969.6	1610.8
191.6	26.1	402.21	4624.9	3133.1	1491.8
201.6	25.0	385.73	4666.8	3296.5	1370.3
211.6	24.0	370.63	4706.5	3460.0	1246.5
221.6	23.1	356.74	4744.2	3623.5	1120.6
231.6	22.3	343.91	4779.9	3787.0	993.0
241.6	21.5	332.03	4814.1	3950.5	863.6
251.6	20.8	321.00	4846.7	4114.0	732.7

<<<<

Modified Rational Method

Project Name : **Detroit River International Crossing**
Stormwater Management Study
Alternative 1B

November 14, 2006

10:30 AM

	107	
Area =	6.56	ha
"C" =	0.9	
AC =	5.904	
Tc =	24.3	min
Time Increment =	10.0	min
Release Rate =	268.7	l/s
Max.Storage =	2829	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

107					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
24.3	114.9	1885.91	2749.7	391.7	2358.0
34.3	93.6	1535.76	3160.6	552.9	2607.7
44.3	79.2	1300.33	3456.3	714.1	2742.2
54.3	68.9	1130.58	3683.4	875.3	2808.1
64.3	61.1	1002.06	3866.0	1036.5	2829.5
74.3	54.9	901.19	4017.5	1197.7	2819.8
84.3	49.9	819.78	4146.5	1358.9	2787.6
94.3	45.9	752.63	4258.4	1520.1	2738.3
104.3	42.4	696.22	4357.0	1681.2	2675.7
114.3	39.5	648.14	4444.9	1842.4	2602.5
124.3	37.0	606.64	4524.3	2003.6	2520.7
134.3	34.8	570.43	4596.5	2164.8	2431.7
144.3	32.8	538.54	4662.7	2326.0	2336.7
154.3	31.1	510.24	4723.8	2487.2	2236.5
164.3	29.5	484.93	4780.4	2648.4	2132.0
174.3	28.2	462.16	4833.3	2809.6	2023.7
184.3	26.9	441.56	4882.8	2970.8	1912.0
194.3	25.8	422.83	4929.3	3132.0	1797.3
204.3	24.7	405.72	4973.3	3293.2	1680.1
214.3	23.8	390.02	5014.8	3454.4	1560.5
224.3	22.9	375.56	5054.3	3615.6	1438.7
234.3	22.1	362.20	5091.9	3776.8	1315.1
244.3	21.3	349.82	5127.7	3938.0	1189.8
254.3	20.6	338.31	5162.0	4099.1	1062.8

<<<<

Stormwater Management Pond Area Requirement Calculation Sheet Alternative 1B

Stormwater Management Facility No.	1B-P1	1B-P2	1B-P3	1B-P4	1B-P5	1B-P6	1B-P7	1B-P8
Drainage ID	100	101	102	103	104	105	106	107
Drainage Area (ha)	6.4	2.7	5.4	4.5	2.7	7.2	6.2	6.6
Imperviousness of Drainage Area	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Runoff Volume ¹ (25mm Storm) (mm)	25.00	25.00	25.00	25.00	25.00	25.00	25.00	25.00
Permanent Pool Volume Required ² (m ³)	1,336	567	1,130	945	575	1,514	1,296	1,378
Extended Detention Volume ³ (m ³)	254	108	215	180	110	288	247	262
Erosion Control Volume ⁴ , 25mm Storm (m ³)	1,590	675	1,345	1,125	685	1,803	1,543	1,640
Total Extended Detention Vol. Req ⁵ (m ³)	1,590	675	1,345	1,125	685	1,803	1,543	1,640
Assumed Permanent Pool Depth	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Assumed Quantity Storage Depth	1.80	1.80	1.80	1.80	1.80	1.80	1.80	1.80
Designed Slope	V:1 H:	5:00	5:00	5:00	5:00	5:00	5:00	5:00
Trial and Error Method Assuming square lot								
Assumed Bottom width	25.00	12.75	21.50	19.00	12.75	26.50	23.75	25.00
Volume	1668.75	699.47	1345.88	1137.75	699.47	1818.38	1549.22	1668.75
Pond Bottom Area Requirement (m ²)	625	163	462	361	163	702	564	625
Pond Bottom width requirement assuming a square lot	25.00	12.75	21.50	19.00	12.75	26.50	23.75	25.00
Pond Area at Normal Water Level	1,600	770	1,332	1,156	770	1,722	1,502	1,600
Pond Area Requirement (m ²)	3,364	2,093	2,970	2,704	2,093	3,540	3,221	3,364
Pond Area Requirement with 10 m buffer (m ²)	6,084	4,323	5,550	5,184	4,323	6,320	5,891	6,084
Total Quantity Control Volume @ 1.80m depth from NWL =	4,468	2,577	3,872	3,474	2,577	4,736	4,250	4,468
Total Quality and Quantity Control Volume =	6,136	3,276	5,218	4,612	3,276	6,555	5,799	6,136
Approximate Volume of Excavation =	17,779	11,887	15,933	14,694	11,887	18,610	17,105	17,779

Notes:

- ¹ Based on Imperviousness for the site (25mm * Imperviousness)
- ² Calculated using Table 3.2 Of the MOE 2003 SWM Planning and Design Manual, 85% Imperviousness (250m³/ha - 40m³/ha)
- ³ Based on 40m³/ha
- ⁴ Area x Runoff Volume
- ⁵ Greater of Extended Detention or Erosion Control
- ⁶ Average release over 24 hours.

Appendix C.3

Alternative 2A

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:41 AM
Stormwater Management Study Alternative 2A

Project No. : 33015384

	100	
Area =	6.36	ha
"C" =	0.9	
AC=	5.724	
Tc =	14.8	min
Time Increment =	2.0	min
Release Rate =	0.4	l/s
Max.Storage =	3688	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
14.8	148.1	2356.05	2089.0	0.3	2088.7
16.8	139.5	2219.63	2234.4	0.4	2234.1
18.8	131.9	2099.04	2364.9	0.4	2364.5
20.8	125.2	1991.63	2482.9	0.4	2482.5
22.8	119.1	1895.32	2590.3	0.5	2589.8
24.8	113.6	1808.44	2688.6	0.5	2688.0
26.8	108.7	1729.65	2779.0	0.6	2778.4
28.8	104.2	1657.85	2862.6	0.6	2861.9
30.8	100.1	1592.13	2940.1	0.7	2939.5
32.8	96.3	1531.73	3012.4	0.7	3011.7
34.8	92.8	1476.03	3080.0	0.8	3079.2
36.8	89.5	1424.48	3143.3	0.8	3142.6
38.8	86.5	1376.63	3203.0	0.8	3202.1
40.8	83.7	1332.09	3259.2	0.9	3258.3
42.8	81.1	1290.52	3312.3	0.9	3311.4
44.8	78.7	1251.62	3362.7	1.0	3361.7
46.8	76.4	1215.15	3410.5	1.0	3409.5
48.8	74.2	1180.87	3456.0	1.1	3455.0
50.8	72.2	1148.60	3499.4	1.1	3498.3
52.8	70.3	1118.15	3540.8	1.1	3539.7
54.8	68.5	1089.38	3580.4	1.2	3579.2
56.8	66.7	1062.15	3618.4	1.2	3617.1
58.8	65.1	1036.32	3654.8	1.3	3653.5
60.8	63.6	1011.81	3689.7	1.3	3688.4

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:42 AM
Stormwater Management Study Alternative 2A

Project No. : 33015384

	101	
Area =	1.69	ha
"C" =	0.9	
AC=	1.521	
Tc =	10.4	min
Time Increment =	2.0	min
Release Rate =	0.1	l/s
Max.Storage =	959	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
10.4	171.7	725.83	451.5	0.1	451.4
12.4	160.0	676.67	502.1	0.1	502.0
14.4	150.0	634.11	546.6	0.1	546.5
16.4	141.2	596.89	586.1	0.1	586.0
18.4	133.4	564.04	621.6	0.1	621.4
20.4	126.5	534.83	653.6	0.2	653.4
22.4	120.3	508.67	682.6	0.2	682.5
24.4	114.7	485.10	709.2	0.2	709.0
26.4	109.7	463.75	733.7	0.2	733.4
28.4	105.1	444.31	756.2	0.2	756.0
30.4	100.9	426.53	777.1	0.2	776.9
32.4	97.0	410.21	796.6	0.3	796.4
34.4	93.5	395.16	814.8	0.3	814.6
36.4	90.2	381.25	831.9	0.3	831.6
38.4	87.1	368.34	847.9	0.3	847.6
40.4	84.3	356.33	863.0	0.3	862.7
42.4	81.6	345.13	877.3	0.3	877.0
44.4	79.1	334.66	890.9	0.3	890.5
46.4	76.8	324.84	903.7	0.4	903.3
48.4	74.6	315.61	915.9	0.4	915.5
50.4	72.6	306.93	927.5	0.4	927.2
52.4	70.7	298.74	938.7	0.4	938.2
54.4	68.8	291.01	949.3	0.4	948.9
56.4	67.1	283.69	959.4	0.4	959.0

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:43 AM
Stormwater Management Study Alternative 2A

Project No. : 33015384

	102	
Area =	5.19	ha
"C" =	0.9	
AC=	4.671	
Tc =	14.4	min
Time Increment =	2.0	min
Release Rate =	0.3	l/s
Max.Storage =	3005	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
14.4	149.8	1944.65	1681.5	0.3	1681.2
16.4	141.0	1830.67	1802.6	0.3	1802.3
18.4	133.2	1730.07	1911.2	0.3	1910.8
20.4	126.3	1640.59	2009.2	0.4	2008.8
22.4	120.2	1560.45	2098.3	0.4	2097.9
24.4	114.6	1488.23	2179.8	0.4	2179.3
26.4	109.6	1422.80	2254.7	0.5	2254.2
28.4	105.0	1363.22	2323.8	0.5	2323.3
30.4	100.8	1308.72	2388.0	0.5	2387.4
32.4	96.9	1258.68	2447.7	0.6	2447.1
34.4	93.4	1212.56	2503.5	0.6	2502.9
36.4	90.1	1169.90	2555.8	0.7	2555.2
38.4	87.0	1130.33	2605.0	0.7	2604.3
40.4	84.2	1093.51	2651.4	0.7	2650.7
42.4	81.6	1059.16	2695.2	0.8	2694.4
44.4	79.1	1027.04	2736.7	0.8	2735.9
46.4	76.8	996.93	2776.1	0.8	2775.3
48.4	74.6	968.64	2813.6	0.9	2812.7
50.4	72.5	942.01	2849.3	0.9	2848.4
52.4	70.6	916.90	2883.4	0.9	2882.4
54.4	68.8	893.18	2915.9	1.0	2915.0
56.4	67.1	870.74	2947.2	1.0	2946.1
58.4	65.4	849.46	2977.1	1.1	2976.0
60.4	63.9	829.27	3005.8	1.1	3004.7

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:43 AM
Stormwater Management Study Alternative 2A

Project No. : 33015384

	103	
Area =	3.31	ha
"C" =	0.9	
AC=	2.979	
Tc =	13.1	min
Time Increment =	2.0	min
Release Rate =	0.2	l/s
Max.Storage =	1905	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
13.1	156.0	1291.98	1018.1	0.2	1017.9
15.1	146.5	1212.89	1101.3	0.2	1101.1
17.1	138.1	1143.47	1175.5	0.2	1175.3
19.1	130.7	1082.02	1242.2	0.2	1241.9
21.1	124.0	1027.22	1302.5	0.3	1302.2
23.1	118.1	978.03	1357.5	0.3	1357.2
25.1	112.7	933.61	1407.9	0.3	1407.6
27.1	107.9	893.29	1454.3	0.3	1453.9
29.1	103.4	856.51	1497.2	0.4	1496.8
31.1	99.4	822.83	1537.0	0.4	1536.6
33.1	95.6	791.85	1574.2	0.4	1573.8
35.1	92.2	763.26	1609.0	0.4	1608.5
37.1	89.0	736.79	1641.6	0.5	1641.1
39.1	86.0	712.21	1672.3	0.5	1671.8
41.1	83.2	689.32	1701.2	0.5	1700.7
43.1	80.7	667.94	1728.6	0.5	1728.1
45.1	78.2	647.93	1754.6	0.6	1754.0
47.1	76.0	629.16	1779.3	0.6	1778.7
49.1	73.8	611.52	1802.7	0.6	1802.1
51.1	71.8	594.89	1825.1	0.6	1824.5
53.1	69.9	579.21	1846.5	0.7	1845.8
55.1	68.1	564.38	1867.0	0.7	1866.3
57.1	66.5	550.34	1886.6	0.7	1885.9
59.1	64.8	537.03	1905.4	0.7	1904.6

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:43 AM
Stormwater Management Study Alternative 2A

Project No. : 33015384

	104	
Area =	4.93	ha
"C" =	0.9	
AC=	4.437	
Tc =	11.2	min
Time Increment =	2.0	min
Release Rate =	0.3	l/s
Max.Storage =	2809	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
11.2	166.7	2056.59	1379.3	0.2	1379.1
13.2	155.8	1921.51	1519.3	0.3	1519.0
15.2	146.3	1804.06	1642.9	0.3	1642.6
17.2	137.9	1700.96	1753.1	0.4	1752.8
19.2	130.5	1609.67	1852.2	0.4	1851.8
21.2	123.9	1528.25	1941.9	0.4	1941.5
23.2	118.0	1455.16	2023.6	0.5	2023.2
25.2	112.6	1389.14	2098.5	0.5	2098.0
27.2	107.8	1329.21	2167.5	0.6	2166.9
29.2	103.3	1274.55	2231.3	0.6	2230.7
31.2	99.3	1224.47	2290.6	0.6	2289.9
33.2	95.5	1178.42	2345.8	0.7	2345.2
35.2	92.1	1135.92	2397.5	0.7	2396.8
37.2	88.9	1096.56	2446.1	0.8	2445.3
39.2	85.9	1060.00	2491.7	0.8	2490.9
41.2	83.2	1025.96	2534.8	0.8	2534.0
43.2	80.6	994.16	2575.5	0.9	2574.7
45.2	78.2	964.41	2614.2	0.9	2613.3
47.2	75.9	936.49	2650.9	1.0	2649.9
49.2	73.8	910.24	2685.8	1.0	2684.8
51.2	71.8	885.52	2719.1	1.0	2718.1
53.2	69.9	862.18	2750.9	1.1	2749.9
55.2	68.1	840.13	2781.4	1.1	2780.3
57.2	66.4	819.24	2810.5	1.2	2809.4

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:44 AM
Stormwater Management Study Alternative 2A
Project No. : 33015384

	105	
Area =	2.61	ha
"C" =	0.9	
AC =	2.349	
Tc =	10.9	min
Time Increment =	2.0	min
Release Rate =	0.2	l/s
Max.Storage =	1485	m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
10.9	168.5	1100.45	718.2	0.1	718.1
12.9	157.3	1027.35	793.8	0.1	793.7
14.9	147.6	963.90	860.4	0.2	860.3
16.9	139.1	908.27	919.8	0.2	919.6
18.9	131.6	859.07	973.0	0.2	972.8
20.9	124.8	815.24	1021.2	0.2	1021.0
22.9	118.8	775.93	1065.1	0.3	1064.8
24.9	113.4	740.45	1105.2	0.3	1105.0
26.9	108.5	708.27	1142.2	0.3	1141.9
28.9	104.0	678.94	1176.4	0.3	1176.0
30.9	99.9	652.08	1208.1	0.4	1207.7
32.9	96.1	627.40	1237.7	0.4	1237.3
34.9	92.6	604.63	1265.3	0.4	1264.9
36.9	89.4	583.56	1291.2	0.4	1290.8
38.9	86.4	563.99	1315.6	0.4	1315.2
40.9	83.6	545.78	1338.6	0.5	1338.1
42.9	81.0	528.78	1360.4	0.5	1359.9
44.9	78.5	512.87	1381.0	0.5	1380.5
46.9	76.3	497.95	1400.6	0.5	1400.0
48.9	74.1	483.92	1419.2	0.6	1418.6
50.9	72.1	470.72	1436.9	0.6	1436.4
52.9	70.2	458.26	1453.9	0.6	1453.3
54.9	68.4	446.48	1470.1	0.6	1469.5
56.9	66.7	435.34	1485.7	0.6	1485.0

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:44 AM
Stormwater Management Study Alternative 2A

Project No. : 33015384

	106	
Area =	5.3	ha
"C" =	0.9	
AC=	4.77	
Tc =	15.8	min
Time Increment =	2.0	min
Release Rate =	0.3	l/s
Max.Storage =	3088	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
15.8	143.7	1905.35	1802.5	0.3	1802.2
17.8	135.6	1798.50	1917.2	0.3	1916.9
19.8	128.5	1703.69	2020.6	0.3	2020.2
21.8	122.1	1618.95	2114.3	0.4	2114.0
23.8	116.3	1542.73	2199.9	0.4	2199.5
25.8	111.1	1473.79	2278.5	0.4	2278.0
27.8	106.4	1411.11	2350.9	0.5	2350.4
29.8	102.1	1353.87	2418.0	0.5	2417.5
31.8	98.1	1301.37	2480.4	0.6	2479.9
33.8	94.5	1253.03	2538.6	0.6	2538.1
35.8	91.1	1208.37	2593.2	0.6	2592.5
37.8	88.0	1166.99	2644.4	0.7	2643.7
39.8	85.1	1128.51	2692.6	0.7	2691.9
41.8	82.4	1092.65	2738.2	0.7	2737.5
43.8	79.9	1059.14	2781.3	0.8	2780.5
45.8	77.5	1027.75	2822.2	0.8	2821.4
47.8	75.3	998.28	2861.1	0.8	2860.3
49.8	73.2	970.56	2898.1	0.9	2897.2
51.8	71.2	944.44	2933.4	0.9	2932.5
53.8	69.4	919.77	2967.2	0.9	2966.3
55.8	67.6	896.44	2999.5	1.0	2998.5
57.8	65.9	874.34	3030.5	1.0	3029.5
59.8	64.4	853.37	3060.2	1.0	3059.1
61.8	62.9	833.45	3088.8	1.1	3087.7

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:44 AM
Stormwater Management Study Alternative 2A

Project No. : 33015384

	107	
Area =	7.06	ha
"C" =	0.9	
AC=	6.354	
Tc =	16.1	min
Time Increment =	2.0	min
Release Rate =	0.4	l/s
Max.Storage =	4119	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
16.1	142.3	2513.93	2426.8	0.4	2426.4
18.1	134.4	2374.39	2577.0	0.4	2576.6
20.1	127.4	2250.41	2712.5	0.5	2712.0
22.1	121.1	2139.48	2835.5	0.5	2835.0
24.1	115.5	2039.62	2947.9	0.5	2947.4
26.1	110.3	1949.22	3051.2	0.6	3050.6
28.1	105.7	1866.96	3146.5	0.6	3145.8
30.1	101.4	1791.78	3234.8	0.7	3234.1
32.1	97.5	1722.79	3316.9	0.7	3316.2
34.1	93.9	1659.23	3393.7	0.8	3392.9
36.1	90.6	1600.48	3465.6	0.8	3464.8
38.1	87.5	1546.01	3533.1	0.9	3532.3
40.1	84.7	1495.34	3596.8	0.9	3595.9
42.1	82.0	1448.10	3656.9	1.0	3656.0
44.1	79.5	1403.93	3713.9	1.0	3712.9
46.1	77.1	1362.55	3767.9	1.1	3766.9
48.1	74.9	1323.69	3819.3	1.1	3818.2
50.1	72.9	1287.12	3868.2	1.1	3867.1
52.1	70.9	1252.64	3914.9	1.2	3913.7
54.1	69.1	1220.08	3959.6	1.2	3958.3
56.1	67.3	1189.28	4002.3	1.3	4001.0
58.1	65.7	1160.09	4043.3	1.3	4042.0
60.1	64.1	1132.39	4082.6	1.4	4081.3
62.1	62.6	1106.06	4120.5	1.4	4119.0

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Stormwater Management Pond Area Requirement Calculation Sheet Alternative 2A

Stormwater Management Facility No.	2A-P1	2A-P2	2A-P3	2A-P4	2A-P5	2A-P6	2A-P7	2A-P8
Drainage ID	100	101	102	103	104	105	106	107
Drainage Area (ha)	6.4	1.7	5.2	3.3	4.9	2.6	5.3	7.1
Imperviousness of Drainage Area	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Runoff Volume (25mm Storm) (mm)	25.00	25.00	25.00	25.00	25.00	25.00	25.00	25.00
Permanent Pool Volume Required ² (m ³)	1,336	357	1,092	693	1,035	548	1,113	1,483
Extended Detention Volume ³ (m ³)	254	68	208	132	197	104	212	282
Erosion Control Volume ⁴ , 25mm Storm (m ³)	1,590	425	1,300	825	1,233	653	1,325	1,765
Total Extended Detention Vol. Req ⁵ (m ³)	1,590	425	1,300	825	1,233	653	1,325	1,765
Assumed Permanent Pool Depth	1.50	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Assumed Quantity Storage Depth	1.80	1.80	1.80	1.80	1.80	1.80	1.80	1.80
Designed Slope	V:1 H:	5.00	5.00	5.00	5.00	5.00	5.00	5.00
Trial and Error Method Assuming square lot								
Assumed Bottom width	25.00	10.00	22.00	15.00	18.00	8.00	24.00	26.00
Volume	1668.75	543.75	1389.75	843.75	1059.75	444.75	1572.75	1767.75
Pond Bottom Area Requirement (m ²)	625	100	484	225	324	64	576	676
Pond Bottom width requirement assuming a square lot	25.00	10.00	22.00	15.00	18.00	8.00	24.00	26.00
Pond Area at Normal Water Level	1,600	625	1,369	900	1,089	529	1,521	1,681
Pond Area Requirement (m ²)	3,364	1,849	3,025	2,304	2,601	1,681	3,249	3,481
Pond Area Requirement with 10 m buffer (m ²)	6,084	3,969	5,625	4,624	5,041	3,721	5,929	6,241
Total Quantity Control Volume @ 1.80m depth from NWL =	4,488	2,277	3,955	2,884	3,321	1,989	4,293	4,646
Total Quantity and Quantity Control Volume =	6,136	2,770	5,344	3,727	4,381	2,434	5,866	6,414
Approximate Volume of Excavation =	17,779	10,783	16,189	12,850	14,217	10,030	17,238	18,330

Notes:

- ¹ Based on Imperviousness for the site (25mm * Imperviousness)
- ² Calculated using Table 3.2 Of the MOE 2003 SWM Planning and Design Manual, 85% Imperviousness (250m³/ha - 40m³/ha)
- ³ Based on 40m³/ha
- ⁴ Area x Runoff Volume
- ⁵ Greater of Extended Detention or Erosion Control
- ⁶ Average release over 24 hours.

Appendix C.4

Alternative 2B

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:46 AM
Stormwater Management Study Alternative 2B

Project No. : 33015384

	100	
Area =	6.36	ha
"C" =	0.9	
AC=	5.724	
Tc =	14.8	min
Time Increment =	2.0	min
Release Rate =	0.4	l/s
Max.Storage =	3688	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
14.8	148.1	2356.05	2089.0	0.3	2088.7
16.8	139.5	2219.63	2234.4	0.4	2234.1
18.8	131.9	2099.04	2364.9	0.4	2364.5
20.8	125.2	1991.63	2482.9	0.4	2482.5
22.8	119.1	1895.32	2590.3	0.5	2589.8
24.8	113.6	1808.44	2688.6	0.5	2688.0
26.8	108.7	1729.65	2779.0	0.6	2778.4
28.8	104.2	1657.85	2862.6	0.6	2861.9
30.8	100.1	1592.13	2940.1	0.7	2939.5
32.8	96.3	1531.73	3012.4	0.7	3011.7
34.8	92.8	1476.03	3080.0	0.8	3079.2
36.8	89.5	1424.48	3143.3	0.8	3142.6
38.8	86.5	1376.63	3203.0	0.8	3202.1
40.8	83.7	1332.09	3259.2	0.9	3258.3
42.8	81.1	1290.52	3312.3	0.9	3311.4
44.8	78.7	1251.62	3362.7	1.0	3361.7
46.8	76.4	1215.15	3410.5	1.0	3409.5
48.8	74.2	1180.87	3456.0	1.1	3455.0
50.8	72.2	1148.60	3499.4	1.1	3498.3
52.8	70.3	1118.15	3540.8	1.1	3539.7
54.8	68.5	1089.38	3580.4	1.2	3579.2
56.8	66.7	1062.15	3618.4	1.2	3617.1
58.8	65.1	1036.32	3654.8	1.3	3653.5
60.8	63.6	1011.81	3689.7	1.3	3688.4

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:47 AM
Stormwater Management Study Alternative 2B

Project No. : 33015384

	101	
Area =	2.13	ha
"C" =	0.9	
AC=	1.917	
Tc =	10.4	min
Time Increment =	2.0	min
Release Rate =	0.2	l/s
Max.Storage =	1209	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
10.4	171.7	915.18	568.6	0.1	568.5
12.4	160.1	853.16	632.5	0.1	632.4
14.4	150.0	799.48	688.6	0.1	688.5
16.4	141.2	752.54	738.5	0.2	738.3
18.4	133.4	711.11	783.2	0.2	783.0
20.4	126.5	674.27	823.5	0.2	823.3
22.4	120.3	641.28	860.2	0.2	860.0
24.4	114.8	611.56	893.7	0.2	893.5
26.4	109.7	584.63	924.5	0.3	924.2
28.4	105.1	560.12	953.0	0.3	952.7
30.4	100.9	537.70	979.3	0.3	979.0
32.4	97.0	517.12	1003.9	0.3	1003.6
34.4	93.5	498.15	1026.8	0.3	1026.5
36.4	90.2	480.60	1048.4	0.3	1048.0
38.4	87.1	464.33	1068.6	0.4	1068.2
40.4	84.3	449.19	1087.6	0.4	1087.2
42.4	81.6	435.06	1105.6	0.4	1105.2
44.4	79.2	421.86	1122.7	0.4	1122.3
46.4	76.8	409.48	1138.9	0.4	1138.4
48.4	74.7	397.85	1154.3	0.5	1153.8
50.4	72.6	386.90	1169.0	0.5	1168.5
52.4	70.7	376.58	1183.0	0.5	1182.5
54.4	68.8	366.83	1196.4	0.5	1195.8
56.4	67.1	357.60	1209.2	0.5	1208.6

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:47 AM
Stormwater Management Study Alternative 2B

Project No. : 33015384

	102	
Area =	6.54	ha
"C" =	0.9	
AC =	5.886	
Tc =	14.8	min
Time Increment =	2.0	min
Release Rate =	0.4	l/s
Max.Storage =	3794	m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
14.8	147.9	2419.42	2151.7	0.3	2151.3
16.8	139.3	2279.53	2300.8	0.4	2300.4
18.8	131.8	2155.85	2434.7	0.4	2434.3
20.8	125.0	2045.68	2555.7	0.5	2555.3
22.8	119.0	1946.88	2665.9	0.5	2665.4
24.8	113.5	1857.74	2766.8	0.6	2766.2
26.8	108.6	1776.89	2859.6	0.6	2859.0
28.8	104.1	1703.20	2945.4	0.6	2944.8
30.8	100.0	1635.75	3025.0	0.7	3024.4
32.8	96.2	1573.76	3099.3	0.7	3098.5
34.8	92.7	1516.58	3168.6	0.8	3167.9
36.8	89.4	1463.66	3233.7	0.8	3232.9
38.8	86.4	1414.54	3294.9	0.9	3294.1
40.8	83.7	1368.81	3352.7	0.9	3351.8
42.8	81.0	1326.12	3407.3	1.0	3406.3
44.8	78.6	1286.19	3459.0	1.0	3458.0
46.8	76.3	1248.73	3508.1	1.0	3507.1
48.8	74.2	1213.54	3554.9	1.1	3553.8
50.8	72.1	1180.39	3599.4	1.1	3598.3
52.8	70.2	1149.12	3642.0	1.2	3640.8
54.8	68.4	1119.57	3682.7	1.2	3681.4
56.8	66.7	1091.60	3721.6	1.3	3720.4
58.8	65.1	1065.08	3759.0	1.3	3757.7
60.8	63.6	1039.90	3794.9	1.4	3793.6

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Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:47 AM
Stormwater Management Study Alternative 2B

Project No. : 33015384

	103	
Area =	7.21	ha
"C" =	0.9	
AC =	6.489	
Tc =	21.1	min
Time Increment =	2.0	min
Release Rate =	0.3	l/s
Max.Storage =	4298	m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
21.1	124.0	2236.91	2837.9	0.4	2837.5
23.1	118.1	2129.82	2957.6	0.4	2957.2
25.1	112.7	2033.12	3067.3	0.5	3066.8
27.1	107.8	1945.33	3168.3	0.5	3167.8
29.1	103.4	1865.27	3261.7	0.6	3261.2
31.1	99.3	1791.93	3348.5	0.6	3347.9
33.1	95.6	1724.49	3429.4	0.6	3428.8
35.1	92.1	1662.24	3505.1	0.7	3504.4
37.1	89.0	1604.61	3576.1	0.7	3575.4
39.1	86.0	1551.09	3643.0	0.8	3642.2
41.1	83.2	1501.24	3706.1	0.8	3705.3
43.1	80.6	1454.69	3765.7	0.8	3764.9
45.1	78.2	1411.12	3822.3	0.9	3821.4
47.1	76.0	1370.25	3876.0	0.9	3875.1
49.1	73.8	1331.82	3927.1	0.9	3926.2
51.1	71.8	1295.63	3975.9	1.0	3974.9
53.1	69.9	1261.48	4022.4	1.0	4021.4
55.1	68.1	1229.19	4067.0	1.1	4065.9
57.1	66.4	1198.62	4109.7	1.1	4108.6
59.1	64.8	1169.62	4150.6	1.1	4149.5
61.1	63.3	1142.09	4189.9	1.2	4188.8
63.1	61.9	1115.90	4227.8	1.2	4226.6
65.1	60.5	1090.97	4264.2	1.3	4263.0
67.1	59.2	1067.19	4299.4	1.3	4298.1

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Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:47 AM
Stormwater Management Study Alternative 2B

Project No. : 33015384

	104	
Area =	2.34	ha
"C" =	0.9	
AC=	2.106	
Tc =	9.7	min
Time Increment =	2.0	min
Release Rate =	0.2	l/s
Max.Storage =	1323	m3

One Hundred Year	
a=	2824.505
b=	13.74
c=	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
9.7	176.1	1030.97	598.6	0.1	598.5
11.7	163.8	959.23	672.1	0.1	672.0
13.7	153.3	897.38	736.5	0.1	736.3
15.7	144.1	843.47	793.4	0.2	793.3
17.7	136.0	796.03	844.3	0.2	844.1
19.7	128.8	753.95	890.2	0.2	890.0
21.7	122.4	716.36	931.7	0.2	931.5
23.7	116.6	682.55	969.7	0.3	969.4
25.7	111.4	651.98	1004.5	0.3	1004.2
27.7	106.6	624.20	1036.6	0.3	1036.3
29.7	102.3	598.82	1066.3	0.3	1066.0
31.7	98.3	575.55	1093.9	0.3	1093.6
33.7	94.6	554.14	1119.7	0.4	1119.4
35.7	91.3	534.35	1143.9	0.4	1143.5
37.7	88.1	516.02	1166.5	0.4	1166.1
39.7	85.2	498.98	1187.9	0.4	1187.5
41.7	82.5	483.10	1208.1	0.5	1207.6
43.7	80.0	468.26	1227.1	0.5	1226.7
45.7	77.6	454.36	1245.2	0.5	1244.8
47.7	75.4	441.31	1262.4	0.5	1261.9
49.7	73.3	429.04	1278.8	0.5	1278.3
51.7	71.3	417.48	1294.5	0.6	1293.9
53.7	69.4	406.56	1309.4	0.6	1308.8
55.7	67.7	396.23	1323.7	0.6	1323.1

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Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:47 AM
Stormwater Management Study Alternative 2B

Project No. : 33015384

	105	
Area =	5.77	ha
"C" =	0.9	
AC =	5.193	
Tc =	20.3	min
Time Increment =	2.0	min
Release Rate =	0.3	l/s
Max.Storage =	3428	m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
20.3	126.7	1829.17	2227.9	0.3	2227.6
22.3	120.5	1739.54	2327.5	0.4	2327.1
24.3	114.9	1658.80	2418.5	0.4	2418.1
26.3	109.8	1585.66	2502.2	0.4	2501.7
28.3	105.2	1519.09	2579.4	0.5	2578.9
30.3	101.0	1458.21	2651.0	0.5	2650.5
32.3	97.1	1402.32	2717.7	0.5	2717.2
34.3	93.6	1350.81	2780.0	0.6	2779.4
36.3	90.3	1303.19	2838.3	0.6	2837.7
38.3	87.2	1259.01	2893.2	0.6	2892.6
40.3	84.4	1217.91	2944.9	0.7	2944.3
42.3	81.7	1179.58	2993.8	0.7	2993.1
44.3	79.2	1143.73	3040.0	0.7	3039.3
46.3	76.9	1110.14	3084.0	0.8	3083.2
48.3	74.7	1078.58	3125.7	0.8	3125.0
50.3	72.7	1048.89	3165.5	0.8	3164.7
52.3	70.7	1020.88	3203.5	0.8	3202.7
54.3	68.9	994.43	3239.8	0.9	3239.0
56.3	67.1	969.40	3274.6	0.9	3273.7
58.3	65.5	945.67	3308.0	0.9	3307.0
60.3	63.9	923.16	3340.0	1.0	3339.0
62.3	62.5	901.75	3370.8	1.0	3369.7
64.3	61.1	881.39	3400.4	1.0	3399.3
66.3	59.7	861.98	3428.9	1.1	3427.9

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Modified Rational Method

Project Name : Detroit River International Crossing November 16, 2006 9:48 AM
Stormwater Management Study Alternative 2B

Project No. : 33015384

	106	
Area =	9.32	ha
"C" =	0.9	
AC =	8.388	
Tc =	16.4	min
Time Increment =	2.0	min
Release Rate =	0.5	l/s
Max.Storage =	5444	m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
16.4	141.2	3291.72	3232.5	0.5	3232.0
18.4	133.4	3110.58	3427.9	0.5	3427.3
20.4	126.5	2949.49	3604.3	0.6	3603.7
22.4	120.3	2805.23	3764.6	0.7	3764.0
24.4	114.7	2675.25	3911.2	0.7	3910.5
26.4	109.7	2557.50	4046.0	0.8	4045.2
28.4	105.1	2450.29	4170.4	0.8	4169.6
30.4	100.9	2352.24	4285.8	0.9	4284.9
32.4	97.0	2262.21	4393.2	1.0	4392.3
34.4	93.5	2179.24	4493.6	1.0	4492.6
36.4	90.2	2102.51	4587.7	1.1	4586.6
38.4	87.1	2031.33	4676.1	1.1	4675.0
40.4	84.3	1965.10	4759.5	1.2	4758.3
42.4	81.6	1903.32	4838.3	1.2	4837.0
44.4	79.1	1845.55	4912.9	1.3	4911.6
46.4	76.8	1791.40	4983.7	1.4	4982.3
48.4	74.6	1740.54	5051.0	1.4	5049.6
50.4	72.6	1692.66	5115.2	1.5	5113.7
52.4	70.7	1647.52	5176.5	1.5	5175.0
54.4	68.8	1604.87	5235.1	1.6	5233.5
56.4	67.1	1564.51	5291.2	1.7	5289.5
58.4	65.5	1526.26	5344.9	1.7	5343.2
60.4	63.9	1489.95	5396.6	1.8	5394.8
62.4	62.4	1455.44	5446.2	1.8	5444.4

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Stormwater Management Pond Area Requirement Calculation Sheet Alternative 2B

Stormwater Management Facility No.	2B-P1	2B-P2	2B-P3	2B-P4	2B-P5	2B-P6	2B-P7
Drainage ID	100	101	102	103	104	105	106
Drainage Area (ha)	6.4	2.1	6.5	7.2	2.3	5.8	9.3
Imperviousness of Drainage Area	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Runoff Volume (25mm Storm) (mm)	25.00	25.00	25.00	25.00	25.00	25.00	25.00
Permanent Pool Volume Required ² (m ³)	1,336	447	1,373	1,514	491	1,212	1,957
Extended Detention Volume ³ (m ³)	254	85	262	288	94	231	373
Erosion Control Volume ⁴ , 25mm Storm (m ³)	1,590	533	1,635	1,803	585	1,443	2,330
Total Extended Detention Vol. Req't ⁵ (m ³)	1,590	533	1,635	1,803	585	1,443	2,330
Assumed Permanent Pool Depth	1.50	1.50	1.50	1.50	1.50	1.50	1.50
Assumed Quantity Storage Depth	1.80	1.80	1.80	1.80	1.80	1.80	1.80
Designed Slope	V:1 H: 5.00	5.00	5.00	5.00	5.00	5.00	5.00
Trial and Error Method Assuming square lot							
Assumed Bottom width	25.00	10.00	25.00	29.00	12.00	23.00	32.00
Volume	1668.75	543.75	1668.75	2082.75	654.75	1479.75	2424.75
Pond Bottom Area Requirement (m ²)	625	100	625	841	144	529	1,024
Pond Bottom width requirement assuming a square lot	25.00	10.00	25.00	29.00	12.00	23.00	32.00
Pond Area at Normal Water Level	1,600	625	1,600	1,936	729	1,444	2,209
Pond Area Requirement (m ²)	3,364	1,849	3,364	3,844	2,025	3,136	4,225
Pond Area Requirement with 10 m buffer (m ²)	6,084	3,959	6,084	6,724	4,225	5,776	7,225
Total Quantity Control Volume @ 1.80m depth from NWL =	4,468	2,227	4,468	5,202	2,479	4,122	5,791
Total Quantity and Quantity Control Volume =	6,136	2,770	6,136	7,285	3,133	5,602	8,215
Approximate Volume of Excavation =	17,779	10,783	17,779	20,047	11,578	16,708	21,860

Notes:

- ¹ Based on Imperviousness for the site (25mm * Imperviousness)
- ² Calculated using Table 3.2 Of the MOE 2003 SWM Planning and Design Manual, 85% Imperviousness (250m³/ha - 40m³/ha)
- ³ Based on 40m³/ha
- ⁴ Area x Runoff Volume
- ⁵ Greater of Extended Detention or Erosion Control
- ⁶ Average release over 24 hours.

Appendix C.5

Alternative 2B – Revised

Modified Rational Method

Project Name : Detroit River International Crossing November 17, 2006 3:49 PM
Stormwater Management Study Alternative 2B Revised Profile
Project No. : 33015384

2BR - P1	
Area =	19.43 ha
"C" =	0.9
AC =	17.487
Tc =	30.9 min
Time Increment =	2.0 min
Release Rate =	0.6 l/s
Max.Storage =	12001 m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.880

Constant Inflows

Rooftop 1	0.0 l/s
Rooftop 2	0.0 l/s
External Area	0.0 l/s
	l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
30.9	99.8	4853.23	8995.0	1.1	8993.9
32.9	96.1	4669.57	9214.9	1.1	9213.8
34.9	92.6	4500.15	9420.6	1.2	9419.4
36.9	89.3	4343.34	9613.5	1.3	9612.3
38.9	86.3	4197.76	9795.0	1.3	9793.7
40.9	83.6	4062.22	9966.2	1.4	9964.8
42.9	81.0	3935.70	10128.1	1.5	10126.7
44.9	78.5	3817.31	10281.5	1.5	10280.0
46.9	76.2	3706.27	10427.2	1.6	10425.6
48.9	74.1	3601.92	10565.9	1.7	10564.2
50.9	72.1	3503.65	10698.0	1.7	10696.3
52.9	70.2	3410.93	10824.3	1.8	10822.4
54.9	68.4	3323.30	10945.0	1.9	10943.1
56.9	66.7	3240.35	11060.6	1.9	11058.7
58.9	65.0	3161.70	11171.5	2.0	11169.5
60.9	63.5	3087.01	11278.1	2.1	11276.0
62.9	62.0	3016.00	11380.6	2.2	11378.4
64.9	60.6	2948.39	11479.3	2.2	11477.0
66.9	59.3	2883.93	11574.4	2.3	11572.1
68.9	58.1	2822.42	11666.2	2.4	11663.8
70.9	56.8	2763.64	11754.9	2.4	11752.4
72.9	55.7	2707.41	11840.6	2.5	11838.1
74.9	54.6	2653.58	11923.6	2.6	11921.0
76.9	53.5	2601.97	12004.0	2.6	12001.3

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 17, 2006 3:46 PM
Stormwater Management Study Alternative 2B Revised Profile

Project No. : 33015384

2BR - P2	
Area =	6.22 ha
"C" =	0.9
AC =	5.598
Tc =	15.2 min
Time Increment =	2.0 min
Release Rate =	0.3 l/s
Max.Storage =	3614 m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.380

Constant Inflows

Rooftop 1	0.0 l/s
Rooftop 2	0.0 l/s
External Area	0.0 l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
15.2	146.3	2276.66	2072.2	0.3	2071.9
17.2	137.9	2146.51	2211.3	0.3	2211.0
19.2	130.5	2031.29	2336.4	0.4	2336.0
21.2	123.9	1928.52	2449.6	0.4	2449.2
23.2	118.0	1836.26	2552.8	0.4	2552.3
25.2	112.6	1752.94	2647.3	0.5	2646.8
27.2	107.8	1677.30	2734.3	0.5	2733.8
29.2	103.3	1608.31	2814.9	0.6	2814.3
31.2	99.3	1545.11	2889.7	0.6	2889.1
33.2	95.5	1486.98	2959.4	0.6	2958.8
35.2	92.1	1433.34	3024.6	0.7	3024.0
37.2	88.9	1383.67	3085.9	0.7	3085.2
39.2	85.9	1337.54	3143.5	0.8	3142.7
41.2	83.2	1294.57	3197.9	0.8	3197.1
43.2	80.6	1254.45	3249.3	0.8	3248.5
45.2	78.2	1216.90	3298.0	0.9	3297.2
47.2	75.9	1181.66	3344.3	0.9	3343.4
49.2	73.8	1148.54	3388.4	0.9	3387.5
51.2	71.8	1117.34	3430.5	1.0	3429.5
53.2	69.9	1087.90	3470.6	1.0	3469.6
55.2	68.1	1060.06	3509.0	1.1	3508.0
57.2	66.4	1033.71	3545.8	1.1	3544.7
59.2	64.8	1008.71	3581.1	1.1	3580.0
61.2	63.3	984.97	3615.1	1.2	3613.9

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 17, 2006 3:47 PM
Stormwater Management Study Alternative 2B Revised Profile

Project No. : 33015384

2BR - P3	
Area =	8.67 ha
"C" =	0.9
AC =	7.803
Tc =	16.2 min
Time Increment =	2.0 min
Release Rate =	0.4 l/s
Max.Storage =	5060 m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.880

Constant Inflows

Rooftop 1	0.0	l/s
Rooftop 2	0.0	l/s
External Area	0.0	l/s
		l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
16.2	142.0	3080.76	2987.1	0.4	2986.7
18.2	134.2	2910.13	3170.9	0.5	3170.4
20.2	127.2	2758.50	3336.7	0.5	3336.2
22.2	120.9	2622.80	3487.3	0.6	3486.7
24.2	115.3	2500.61	3624.9	0.6	3624.3
26.2	110.2	2389.97	3751.3	0.7	3750.6
28.2	105.5	2289.29	3868.0	0.7	3867.3
30.2	101.3	2197.25	3976.1	0.8	3975.4
32.2	97.4	2112.78	4076.8	0.8	4076.0
34.2	93.8	2034.95	4170.8	0.9	4170.0
36.2	90.5	1963.00	4258.9	0.9	4258.0
38.2	87.4	1896.28	4341.7	1.0	4340.8
40.2	84.6	1834.22	4419.7	1.0	4418.7
42.2	81.9	1776.34	4493.4	1.1	4492.4
44.2	79.4	1722.23	4563.2	1.1	4562.1
46.2	77.1	1671.53	4629.5	1.2	4628.3
48.2	74.9	1623.91	4692.4	1.2	4691.2
50.2	72.8	1579.09	4752.4	1.3	4751.2
52.2	70.8	1536.84	4809.7	1.3	4808.4
54.2	69.0	1496.94	4864.4	1.4	4863.1
56.2	67.3	1459.18	4916.9	1.4	4915.4
58.2	65.6	1423.40	4967.1	1.5	4965.6
60.2	64.1	1389.45	5015.3	1.5	5013.8
62.2	62.6	1357.18	5061.7	1.6	5060.2

<<<<

Modified Rational Method

Project Name : Detroit River International Crossing November 17, 2006 4:09 PM
Stormwater Management Study Alternative 2B Revised Profile
Project No. : 33015384

2BR - P4	
Area =	6.36 ha
"C" =	0.9
AC =	5.724
Tc =	12.9 min
Time Increment =	2.0 min
Release Rate =	0.4 l/s
Max.Storage =	3656 m3

One Hundred Year	
a =	2824.505
b =	13.74
c =	0.880

Constant Inflows

Rooftop 1	0.0 l/s
Rooftop 2	0.0 l/s
External Area	0.0 l/s
	l/s

Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m3)	Storage Volume (m3)
12.9	157.1	2499.94	1938.0	0.3	1937.7
14.9	147.4	2345.76	2099.9	0.3	2099.6
16.9	138.9	2210.56	2244.2	0.4	2243.8
18.9	131.4	2090.99	2373.7	0.4	2373.3
20.9	124.7	1984.44	2490.9	0.5	2490.4
22.9	118.7	1888.85	2597.5	0.5	2597.0
24.9	113.3	1802.59	2695.2	0.6	2694.7
26.9	108.4	1724.33	2785.1	0.6	2784.5
28.9	103.9	1652.98	2868.3	0.6	2867.6
30.9	99.8	1587.66	2945.4	0.7	2944.7
32.9	96.0	1527.62	3017.4	0.7	3016.6
34.9	92.5	1472.23	3084.6	0.8	3083.8
36.9	89.3	1420.96	3147.7	0.8	3146.9
38.9	86.3	1373.36	3207.1	0.9	3206.2
40.9	83.5	1329.04	3263.1	0.9	3262.1
42.9	80.9	1287.67	3316.0	1.0	3315.0
44.9	78.5	1248.95	3366.2	1.0	3365.2
46.9	76.2	1212.64	3413.8	1.0	3412.8
48.9	74.1	1178.52	3459.2	1.1	3458.1
50.9	72.0	1146.38	3502.4	1.1	3501.3
52.9	70.1	1116.05	3543.7	1.2	3542.5
54.9	68.3	1087.40	3583.2	1.2	3582.0
56.9	66.6	1060.26	3621.0	1.3	3619.7
58.9	65.0	1034.54	3657.3	1.3	3656.0

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Stormwater Management Pond Area Requirement Calculation Sheet Alternative 2B-Revised Profile

Stormwater Management Facility No.	2BR - P1	2BR - P2	2BR - P3	2BR - P4
Drainage Area ID	103	102	101	100
Drainage Area (ha)	19.4	6.2	8.7	6.4
Imperviousness of Drainage Area	1.00	1.00	1.00	1.00
Runoff Volume ¹ (25mm Storm) (mm)	25.00	25.00	25.00	25.00
Permanent Pool Volume Required ² (m ³)	4,080	1,306	1,821	1,336
Extended Detention Volume ³ (m ³)	777	249	347	254
Erosion Control Volume ⁴ , 25mm Storm (m ³)	4,858	1,555	2,168	1,590
Total Extended Detention Vol. Req't ⁵ (m ³)	4,858	1,555	2,168	1,590
Assumed Permanent Pool Depth	1.50	1.50	1.50	1.50
Assumed Quantity Storage Depth	1.80	1.80	1.80	1.80
Designed Slope	V:1 H:1	5.00	5.00	5.00
Trial and Error Method Assuming square lot				
Assumed Bottom width	49.00	24.00	30.00	29.00
Volume	4872.75	1572.75	2193.75	2082.75
Pond Bottom Area Requirement (m ²)	2,401	576	900	841
Pond Bottom width requirement assuming a square lot	49.00	24.00	30.00	29.00
Pond Area at Normal Water Level	4,096	1,521	2,025	1,936
Pond Area Requirement (m ²)	6,724	3,249	3,969	3,844
Pond Area Requirement with 10 m buffer (m ²)	10,404	5,929	6,889	6,724
Total Quantity Control Volume @ 1.80m depth from NWL =	9,738	4,293	5,395	5,202
Total Quantity and Quantity Control Volume =	14,611	5,866	7,588	7,285
Approximate Volume of Excavation =	33,933	17,238	20,641	20,047

Notes:

- ¹ Based on Imperviousness for the site (25mm * Imperviousness)
- ² Calculated using Table 3.2 Of the MOE 2003 SWM Planning and Design Manual, 85% Imperviousness (25C)
- ³ Based on 40m³/ha
- ⁴ Area x Runoff Volume
- ⁵ Greater of Extended Detention or Erosion Control
- ⁶ Average release over 24 hours.

Appendix C.6

Alternative 3 – Tunnel

Modified Rational Method

Project Name : **Detroit River International Crossing Stormwater Management Study** November 14, 2006 11:19 AM
Alternative 3

	100	
Area =	6.36	ha
"C" =	0.9	
AC =	5.724	
Tc =	14.4	min
Time Increment =	10.0	min
Release Rate =	367.0	l/s
Max.Storage =	2376	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

100					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m ³)	Released Volume (m ³)	Storage Volume (m ³)
14.4	149.6	2380.56	2063.1	318.1	1745.0
24.4	114.5	1822.33	2672.7	538.3	2134.4
34.4	93.3	1485.01	3069.0	758.6	2310.5
44.4	79.1	1257.93	3354.5	978.8	2375.7 <<<<
54.4	68.8	1094.07	3574.0	1199.0	2374.9
64.4	61.0	969.93	3750.4	1419.2	2331.2
74.4	54.8	872.45	3897.0	1639.5	2257.5
84.4	49.9	793.76	4021.7	1859.7	2162.0
94.4	45.8	728.82	4130.0	2079.9	2050.1
104.4	42.4	674.27	4225.4	2300.1	1925.3
114.4	39.5	627.76	4310.6	2520.4	1790.2
124.4	36.9	587.60	4387.4	2740.6	1646.8
134.4	34.7	552.56	4457.3	2960.8	1496.5
144.4	32.8	521.70	4521.4	3181.0	1340.4
154.4	31.1	494.31	4580.6	3401.3	1179.3
164.4	29.5	469.81	4635.4	3621.5	1014.0
174.4	28.1	447.77	4686.6	3841.7	844.9
184.4	26.9	427.82	4734.6	4061.9	672.6
194.4	25.7	409.69	4779.7	4282.2	497.5
204.4	24.7	393.12	4822.2	4502.4	319.8
214.4	23.7	377.92	4862.5	4722.6	139.9
224.4	22.9	363.92	4900.8	4942.8	-42.1
234.4	22.1	350.98	4937.2	5163.1	-225.9
244.4	21.3	338.99	4971.9	5383.3	-411.4

Modified Rational Method

Project Name : **Detroit River International Crossing Stormwater Management Study** November 14, 2006 11:19 AM
Alternative 3

	101	
Area =	2.80	ha
"C" =	0.9	
AC =	2.52	
Tc =	13.9	min
Time Increment =	10.0	min
Release Rate =	167.3	l/s
Max.Storage =	1030	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

101					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m3)	Released Volume (m ³)	Storage Volume (m ³)
13.9	152.4	1067.32	888.0	139.2	748.8
23.9	116.1	813.12	1164.4	239.6	924.8
33.9	94.3	660.75	1342.7	340.0	1002.7
43.9	79.7	558.69	1470.5	440.3	1030.1
53.9	69.3	485.29	1568.4	540.7	1027.7
63.9	61.4	429.81	1647.0	641.1	1005.9
73.9	55.1	386.33	1712.2	741.5	970.7
83.9	50.1	351.27	1767.6	841.9	925.8
93.9	46.0	322.38	1815.6	942.2	873.4
103.9	42.6	298.13	1858.0	1042.6	815.3
113.9	39.6	277.47	1895.7	1143.0	752.7
123.9	37.1	259.65	1929.7	1243.4	686.3
133.9	34.8	244.10	1960.6	1343.8	616.9
143.9	32.9	230.42	1989.0	1444.1	544.9
153.9	31.2	218.28	2015.1	1544.5	470.6
163.9	29.6	207.43	2039.4	1644.9	394.5
173.9	28.2	197.66	2062.0	1745.3	316.8
183.9	27.0	188.83	2083.2	1845.7	237.6
193.9	25.8	180.81	2103.1	1946.0	157.1
203.9	24.8	173.47	2121.9	2046.4	75.5
213.9	23.8	166.75	2139.7	2146.8	-7.1
223.9	22.9	160.56	2156.6	2247.2	-90.6
233.9	22.1	154.84	2172.7	2347.6	-174.9
243.9	21.3	149.54	2188.0	2447.9	-259.9

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Modified Rational Method

Project Name : **Detroit River International Crossing Stormwater Management Study**
 Alternative 3

November 14, 2006 11:19 AM

Area =	108	ha
"C" =	2.16	
AC =	0.9	
Tc =	1.944	min
Time Increment =	13.6	min
Release Rate =	10.0	l/s
Max.Storage =	130.6	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

l/s

108					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m ³)	Released Volume (m ³)	Storage Volume (m ³)
13.6	153.7	830.43	677.6	106.6	571.0
23.6	116.8	631.20	893.8	185.0	708.8
33.6	94.8	512.25	1032.7	263.4	769.3
43.6	80.1	432.75	1132.1	341.7	790.3
53.6	69.5	375.67	1208.1	420.1	788.0
63.6	61.5	332.57	1269.1	498.5	770.6
73.6	55.3	298.82	1319.6	576.9	742.7
83.6	50.3	271.64	1362.5	655.3	707.3
93.6	46.1	249.24	1399.7	733.6	666.1
103.6	42.6	230.45	1432.5	812.0	620.4
113.6	39.7	214.44	1461.7	890.4	571.3
123.6	37.1	200.64	1488.0	968.8	519.2
133.6	34.9	188.61	1511.9	1047.1	464.7
143.6	32.9	178.02	1533.8	1125.5	408.3
153.6	31.2	168.62	1554.0	1203.9	350.1
163.6	29.6	160.23	1572.8	1282.3	290.5
173.6	28.3	152.68	1590.3	1360.7	229.6
183.6	27.0	145.85	1606.6	1439.0	167.6
193.6	25.8	139.64	1622.0	1517.4	104.6
203.6	24.8	133.97	1636.6	1595.8	40.7
213.6	23.8	128.77	1650.3	1674.2	-23.9
223.6	22.9	123.98	1663.3	1752.6	-89.2
233.6	22.1	119.56	1675.7	1830.9	-155.2
243.6	21.4	115.46	1687.6	1909.3	-221.7

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Modified Rational Method

Project Name : **Detroit River International Crossing Stormwater Management Study Alternative 3** November 14, 2006 11:19 AM

	109	
Area =	6.56	ha
"C" =	0.9	
AC =	5.904	
Tc =	24.3	min
Time Increment =	10.0	min
Release Rate =	268.7	l/s
Max.Storage =	2829	m ³

Controlled Condition

100 Year - Post Dev't.	
a =	2825
b =	13.74
c =	0.880

Constant Inflows

	l/s

109					
Time (min)	Rainfall Intensity (mm/hr)	Storm Runoff (l/s)	Runoff Volume (m ³)	Released Volume (m ³)	Storage Volume (m ³)
24.3	114.9	1885.91	2749.7	391.8	2357.9
34.3	93.6	1535.76	3160.6	553.0	2607.6
44.3	79.2	1300.33	3456.3	714.3	2742.0
54.3	68.9	1130.58	3683.4	875.5	2807.9
64.3	61.1	1002.06	3866.0	1036.7	2829.2
74.3	54.9	901.19	4017.5	1197.9	2819.6
84.3	49.9	819.78	4146.5	1359.2	2787.3
94.3	45.9	752.63	4258.4	1520.4	2738.0
104.3	42.4	696.22	4357.0	1681.6	2675.3
114.3	39.5	648.14	4444.9	1842.9	2602.1
124.3	37.0	606.64	4524.3	2004.1	2520.2
134.3	34.8	570.43	4596.5	2165.3	2431.2
144.3	32.8	538.54	4662.7	2326.6	2336.1
154.3	31.1	510.24	4723.8	2487.8	2236.0
164.3	29.5	484.93	4780.4	2649.0	2131.4
174.3	28.2	462.16	4833.3	2810.2	2023.0
184.3	26.9	441.56	4882.8	2971.5	1911.3
194.3	25.8	422.83	4929.3	3132.7	1796.6
204.3	24.7	405.72	4973.3	3293.9	1679.3
214.3	23.8	390.02	5014.8	3455.2	1559.7
224.3	22.9	375.56	5054.3	3616.4	1437.9
234.3	22.1	362.20	5091.9	3777.6	1314.3
244.3	21.3	349.82	5127.7	3938.9	1188.9
254.3	20.6	338.31	5162.0	4100.1	1061.9

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Stormwater Management Pond Area Requirement Calculation Sheet OPTION 3

Stormwater Management Facility No.	3-P1	3-P2	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	3-P3	3-P3
Drainage ID	100	101	102	103	104	105	106	107	108	109											
Drainage Area (ha)	6.4	2.8	0.3	0.3	0.1	0.2	0.2	0.2	2.2	6.6											
Imperviousness of Drainage Area	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00											
Runoff Volume ¹ (25mm Storm) (mm)	25.00	25.00	25.00	25.00	25.00	25.00	25.00	25.00	25.00	25.00											
Permanent Pool Volume Required ² (m ³)	1,336	588	71	71	29	40	36	40	454	1,378											
Extended Detention Volume ³ (m ³)	254	112	14	14	6	8	7	8	86	262											
Erosion Control Volume ⁴ , 25mm Storm (m ³)	1,590	700	85	85	35	48	43	48	540	1,640											
Total Extended Detention Vol. Req ⁵ (m ³)	1,590	700	85	85	35	48	43	48	540	1,640											
Assumed Permanent Pool Depth	1.50	1.50	N/A	N/A	N/A	N/A	N/A	N/A	1.50	1.50											
Assumed Quantity Storage Depth	1.80	1.80	N/A	N/A	N/A	N/A	N/A	N/A	1.80	1.80											
Designed Slope	V:1 H:1	5:00	N/A	N/A	N/A	N/A	N/A	N/A	5:00	5:00											
Trial and Error Method Assuming square lot																					
Assumed Bottom width	25.00	14.00	N/A	N/A	N/A	N/A	N/A	N/A	11.00	25.00											
Volume	1668.75	777.75	N/A	N/A	N/A	N/A	N/A	N/A	597.75	1668.75											
Pond Bottom Area Requirement (m ²)	625	196	N/A	N/A	N/A	N/A	N/A	N/A	121	625											
Pond Bottom width requirement assuming a square lot	25.00	14.00	N/A	N/A	N/A	N/A	N/A	N/A	11.00	25.00											
Pond Area at Normal Water Level	1,600	841	N/A	N/A	N/A	N/A	N/A	N/A	676	1,600											
Pond Area Requirement (m ²)	3,364	2,209	N/A	N/A	N/A	N/A	N/A	N/A	1,936	3,364											
Pond Area Requirement with 10 m buffer (m ²)	6,084	4,489	N/A	N/A	N/A	N/A	N/A	N/A	4,096	6,084											
Total Quantity Control Volume @ 1.80m depth from NWL =	4,468	2,745	N/A	N/A	N/A	N/A	N/A	N/A	2,351	4,468											
Total Quality and Quantity Control Volume =	6,136	3,523	N/A	N/A	N/A	N/A	N/A	N/A	2,949	6,136											
Approximate Volume of Excavation =	17,779	12,415	N/A	N/A	N/A	N/A	N/A	N/A	11,175	17,779											

Notes:

- ¹ Based on Imperviousness for the site (25mm * Imperviousness)
- ² Calculated using Table 3.2 Of the MOE 2003 SWM Planning and Design Manual, 85% Imperviousness (250m³/ha - 40m³/ha)
- ³ Based on 40m³/ha
- ⁴ Area x Runoff Volume
- ⁵ Greater of Extended Detention or Erosion Control
- ⁶ Average release over 24 hours.
- * Drainage Area 108 and 109 Drain to same pond 3-P3